# First Southern Baptist Church of Bryant 604 S REYNOLDS ROAD, BRYANT, AR 72022 DRAINAGE REPORT

FOR
City of Bryant, Saline County, AR

September 2024

Owner & Developer: Peter Cunningham.

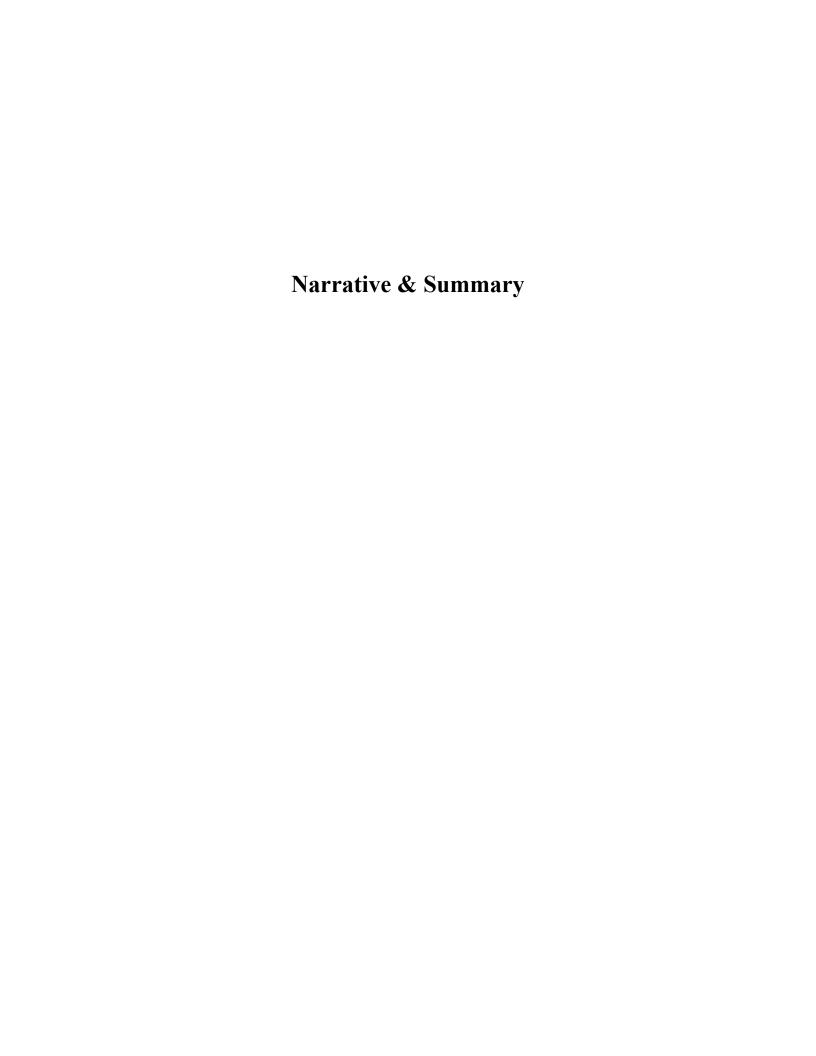
By:



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### ITEM DESCRIPTION

- 1. Narrative & Summary
- 2. Hydrograph Report



#### PROJECT TITLE

First Southern Baptist Church of Bryant

#### PROJECT PROPERTY OWNER

Peter Cunningham

#### PROJECT LOCATION

604 S Reynolds Road, Bryant, AR

#### PROJECT DESCRIPTION

The proposed development is on South Reynolds Road, Bryant, AR. Total development site area is 7.58 acres.

#### **DRAINAGE ANALYSIS**

On Site Drainage- Rational method was used to determine the existing and proposed flows from proposed site. There will be one retention pond to detain water from this development. Detailed drainage calculations considering the future expected development have been conducted to determine the required detention pond and culvert dimensions. Summary of the calculations are below:

#### **Retention Pond**

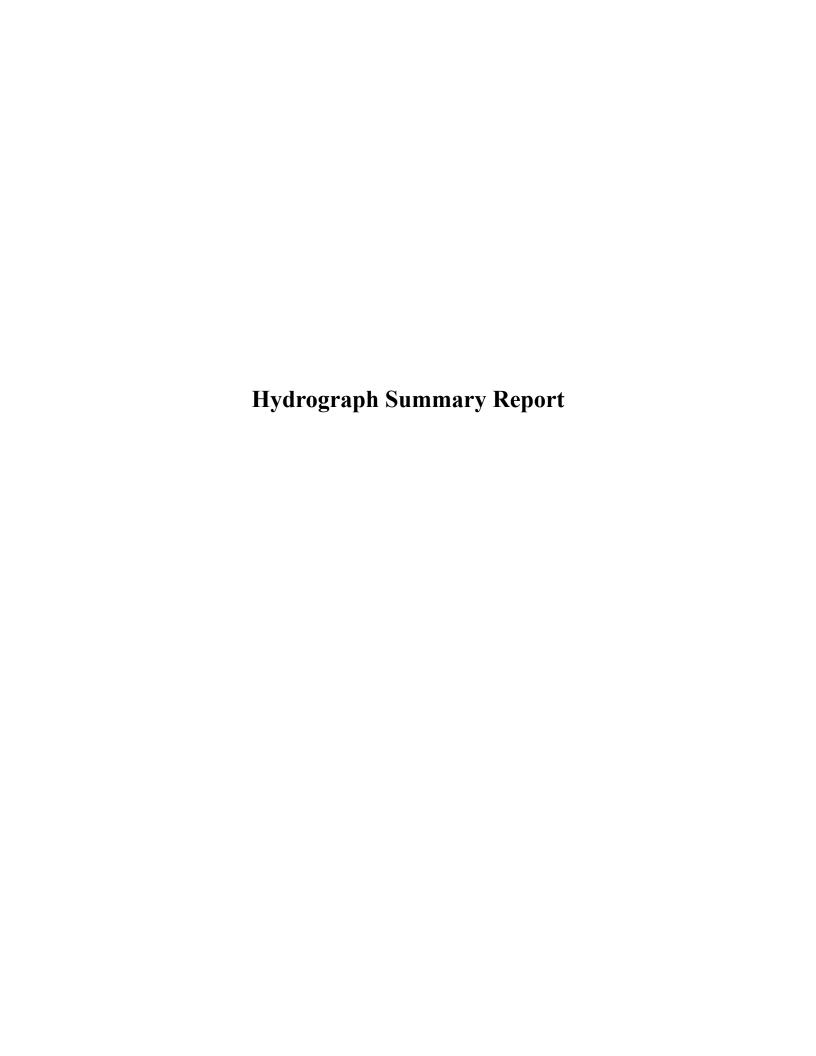
- Pond is situated on the north-east side of the property.
- Pre-development area 7.36 acres.
- Post-development area 7.34 acres.
- Pre-development runoff cumulative coefficient 0.65.
- Post-development runoff cumulative coefficient 0.72
- Pond has a bottom area of 16,570 sqft with bottom elevation of 393.4'.
- A 5' wide spillway with a 3' wide weir outlet structure.

#### Peak flows for Pre and post development phase of onsite area have been tabulated below-

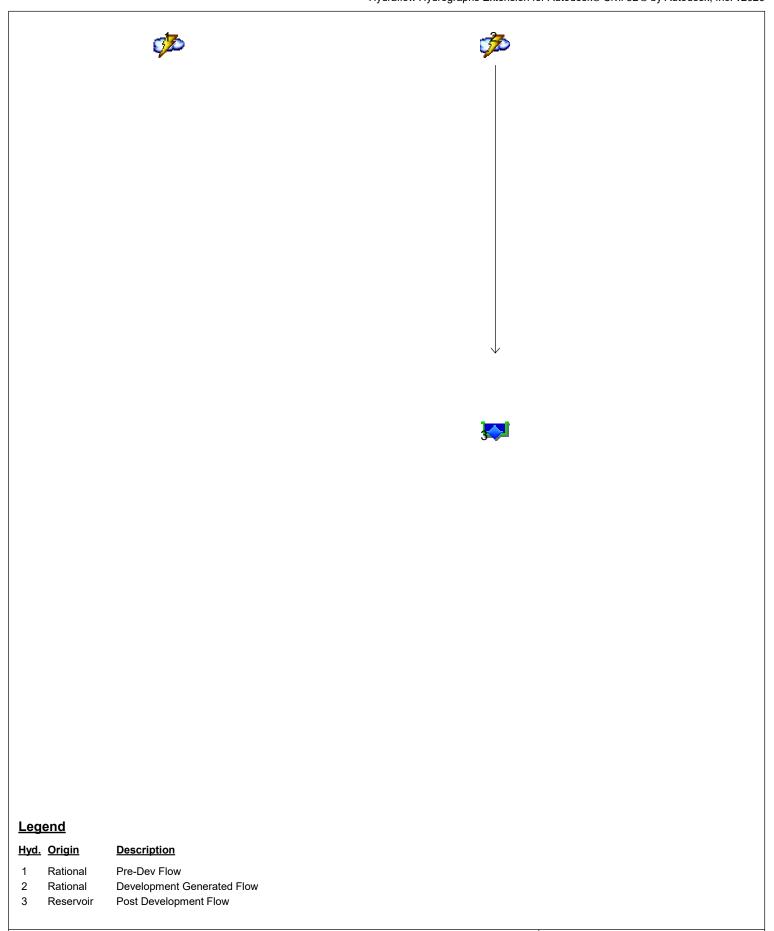
Period of	Pre-development	Post-dev. Without	Post-dev. With detention
time		detention	
	Peak Flow (cfs)	Peak Flow (cfs)	Peak Flow (cfs)
2-Year	18.69	22.67	5.733
5-Year	20.65	25.15	6.587
10-Year	24.35	29.23	8.068
25-Year	27.93	33.44	9.693
50-Year	31.84	38.07	11.94
100-Year	33.86	40.40	13.17

### **CONCLUSION**

From the onsite drainage calculation, it is seen that there is decrease in flow for all storm events due to the proposed retention pond.



## **Watershed Model Schematic**



Project: K:\Land Projects 2004\Commercial\2024\24-0260 FSCB Expansion and Remodel\\@ihill@daiya@e\DE/T20V5ION-CONTOUR 8

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

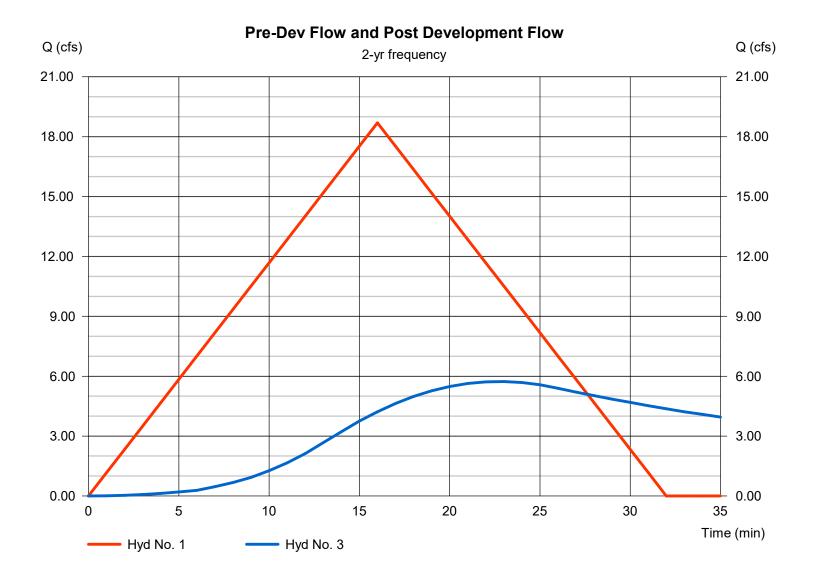
Hyd. No. 1

Pre-Dev Flow

Hydrograph type = Rational Peak discharge = 18.69 cfs Time to peak = 16 min Hyd. Volume = 17,943 cuft Hyd. No. 3

Post Development Flow

Hydrograph type = Reservoir
Peak discharge = 5.73 cfs
Time to peak = 23 min
Hyd. Volume = 17,672 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

Hyd. No. 1

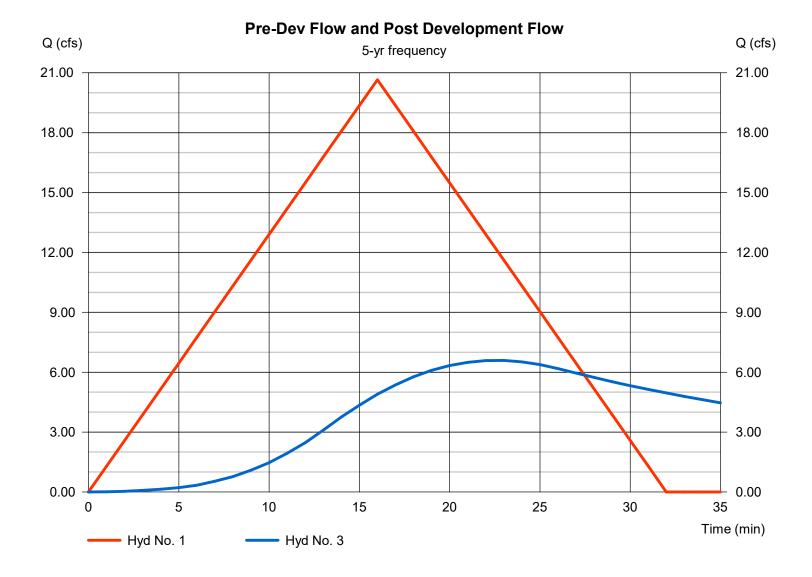
Pre-Dev Flow

Hydrograph type = Rational
Peak discharge = 20.65 cfs
Time to peak = 16 min
Hyd. Volume = 19,826 cuft

Hyd. No. 3

Post Development Flow

Hydrograph type = Reservoir
Peak discharge = 6.59 cfs
Time to peak = 23 min
Hyd. Volume = 19,608 cuft



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Hyd. No. 1

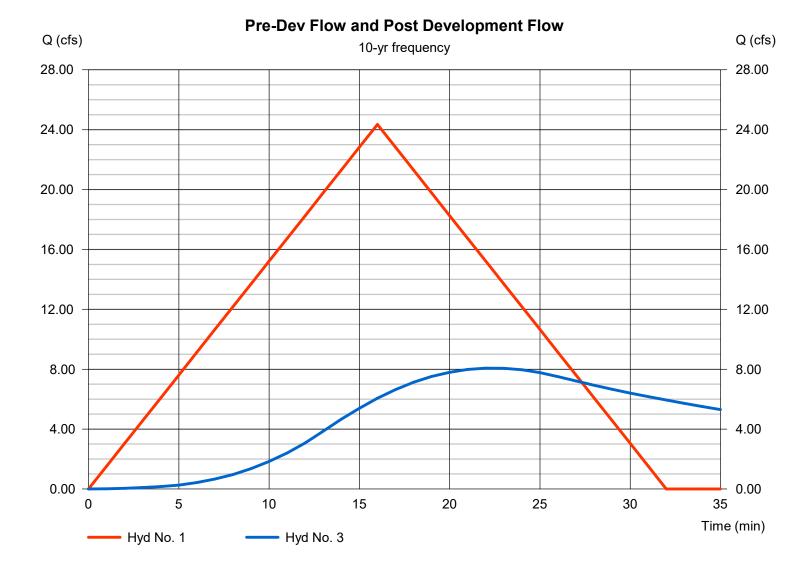
Pre-Dev Flow

Hydrograph type = Rational Peak discharge = 24.35 cfs Time to peak = 16 min Hyd. Volume = 23,373 cuft

### Hyd. No. 3

Post Development Flow

Hydrograph type = Reservoir
Peak discharge = 8.07 cfs
Time to peak = 22 min
Hyd. Volume = 22,791 cuft



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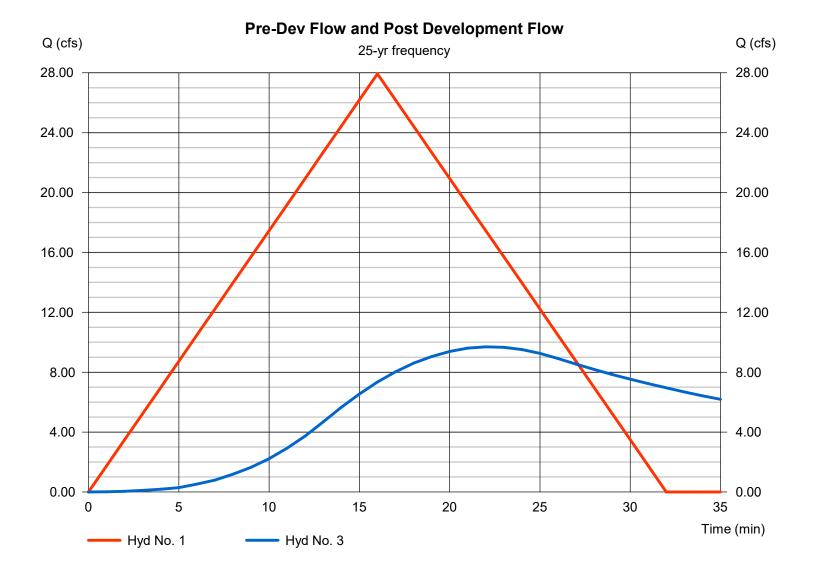
Hyd. No. 1

Pre-Dev Flow

Hydrograph type = Rational Peak discharge = 27.93 cfs Time to peak = 16 min Hyd. Volume = 26,812 cuft Hyd. No. 3

Post Development Flow

Hydrograph type = Reservoir
Peak discharge = 9.69 cfs
Time to peak = 22 min
Hyd. Volume = 26,080 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

Hyd. No. 1

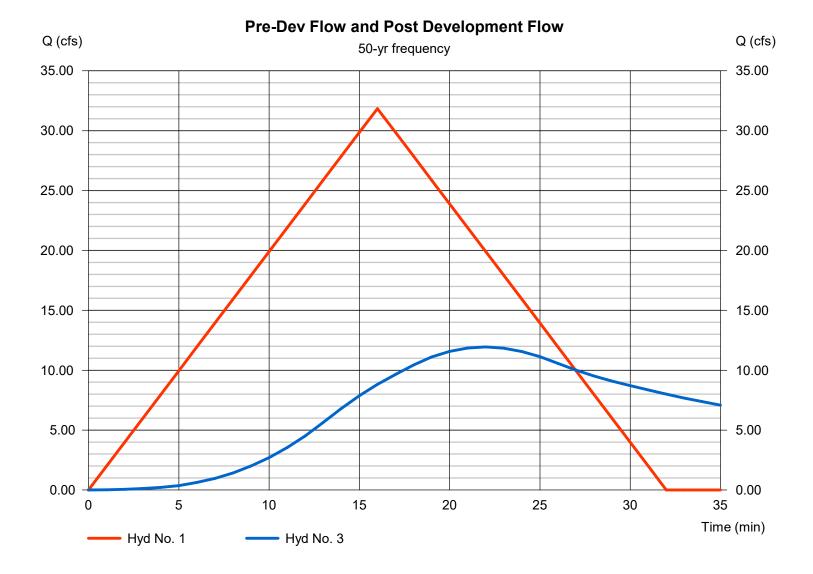
Pre-Dev Flow

Hydrograph type = Rational Peak discharge = 31.84 cfs Time to peak = 16 min Hyd. Volume = 30,570 cuft

### Hyd. No. 3

Post Development Flow

Hydrograph type = Reservoir
Peak discharge = 11.94 cfs
Time to peak = 22 min
Hyd. Volume = 29,692 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

Hyd. No. 1

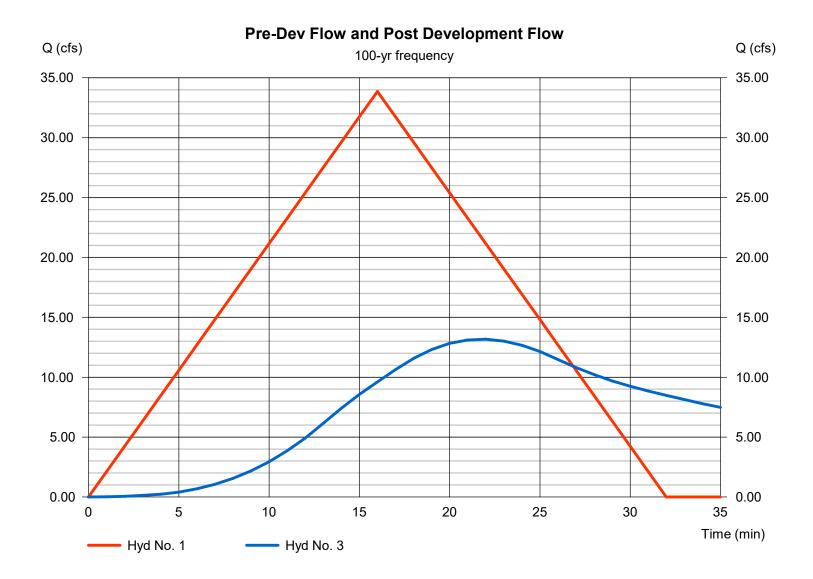
Pre-Dev Flow

Hydrograph type = Rational
Peak discharge = 33.86 cfs
Time to peak = 16 min
Hyd. Volume = 32,504 cuft

Hyd. No. 3

Post Development Flow

Hydrograph type = Reservoir
Peak discharge = 13.17 cfs
Time to peak = 22 min
Hyd. Volume = 31,502 cuft



## **Pond Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

Thursday, 08 / 7 / 2025

#### Pond No. 1 - Retention Pond

#### **Pond Data**

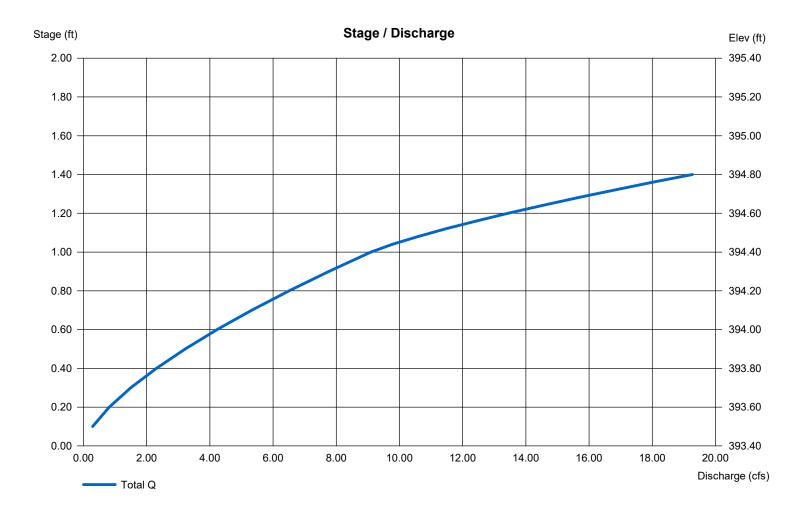
Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 393.40 ft

#### Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	393.40	16,570	0	0
1.00	394.40	21,182	18,827	18,827
1.40	394.80	23,045	8,842	27,669

Culvert / Ori	fice Structur	es		Weir Structu	Weir Structures					
	[A]	[B]	[C]	[PrfRsr]			[A]	[B]	[C]	[D]
Rise (in)	Inactive	Inactive	Inactive	Inactive	Crest Len (ft)	=	3.00	5.00	0.00	0.00
Span (in)	= 8.00	8.00	0.00	0.00	Crest El. (ft)	=	393.40	394.40	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	=	3.03	3.33	3.33	3.33
Invert El. (ft)	= 393.40	393.40	0.00	0.00	Weir Type	=	Rect	Rect		
Length (ft)	= 25.00	25.00	0.00	0.00	Multi-Stage	=	No	No	No	No
Slope (%)	= 0.52	0.52	0.00	n/a	_					
N-Value	= .013	.013	.013	n/a						
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	=	0.000 (by	Contour)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	=	0.00	,		

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	18.69	1	16	17,943				Pre-Dev Flow
2	Rational	22.67	1	13	17,679				Development Generated Flow
2 3	Reservoir	22.67 5.733	1 1	13 23	17,679 17,672	2	394.13	13,831	Post Development Flow
	ND 8-7-2025				Datama	Period: 2 Y		Therese	08 / 7 / 2025

yd. o.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	20.65	1	16	19,826				Pre-Dev Flow
2	Rational	25.15	1	13	19,614				Development Generated Flow
3	Reservoir	6.587	1	23	19,608	2	394.21	15,185	Post Development Flow
-O	ND 8-7-2025	.gpw	-	•	Return I	Period: 5 Ye	ear	Thursday,	08 / 7 / 2025

lyd. Io.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	24.35	1	16	23,373				Pre-Dev Flow
2	Rational	29.23	1	13	22,797				Development Generated Flow
3	Reservoir	8.068	1	22	22,791	2	394.32	17,379	Post Development Flow
-0	ND 8-7-2025	.gpw			Return I	Period: 10 `	 Year	Thursday.	08 / 7 / 2025

łyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	27.93	1	16	26,812				Pre-Dev Flow
2	Rational	33.44	1	13	26,086				Development Generated Flow
2 3	Reservoir	33.44 9.693	1 1	13 22	26,086 26,080	2	394.44	19,606	Development Generated Flow Post Development Flow
	ND 8-7-2025.								

łyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	31.84	1	16	30,570				Pre-Dev Flow
2	Rational	38.07	1	13	29,698				Development Generated Flow
2 3	Rational	38.07	1 1	13 22	29,698 29,692	2	394.54	21,917	Development Generated Flow Post Development Flow
	ND 8-7-2025.					Period: 50 N			08 / 7 / 2025

yd. Hydrograp o. type (origin)	h Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1 Rational	33.86	1	16	32,504				Pre-Dev Flow
2 Rational	40.40	1	13	31,509				Development Generated Flow
2 Rational 3 Reservoir	40.40	1 1	13 22	31,509 31,502	2	394.59	23,012	Development Generated Flow Post Development Flow