ARKANSAS STORAGE CENTER BRYANT, AR DRAINAGE REPORT

FOR
City of Bryant, Saline County, AR

October 2023

Owner & Developer: STUART FINLEY Address: P.O Box 10, Bryant, AR. 72089

By:



PROJECT TITLE

I-30 SELF STORAGE

PROJECT PROPERTY OWNER

STUART FINLEY

PROJECT LOCATION

25300 I-30 North, Bryant, AR

PROJECT DESCRIPTION

The proposed self-storage facility development is located on High-way I-30 in the city of Bryant, Arkansas. The total development area is 24.31 acres.

DRAINAGE ANALYSIS

On Site Drainage- Rational method was used to determine the existing and proposed flows from proposed site. Detailed drainage calculations considering the future expected development have been conducted. Summary of the calculations are below:

- Pre-development area: 28.91 acres.
- Post-development area: 28.91 acres.
- Pre-development runoff coefficient: 0.47.
- Post-development runoff coefficient: 0.88.
- Time of Concentration for Pre-development Area: 16.05 min
- Time of Concentration for Post-development Area: 8.03 min
- Pond has a bottom area of 1.67 acres with bottom elevation of 349.00'
- One 18" RCP with 0.5% slope is proposed for outflow culvert.

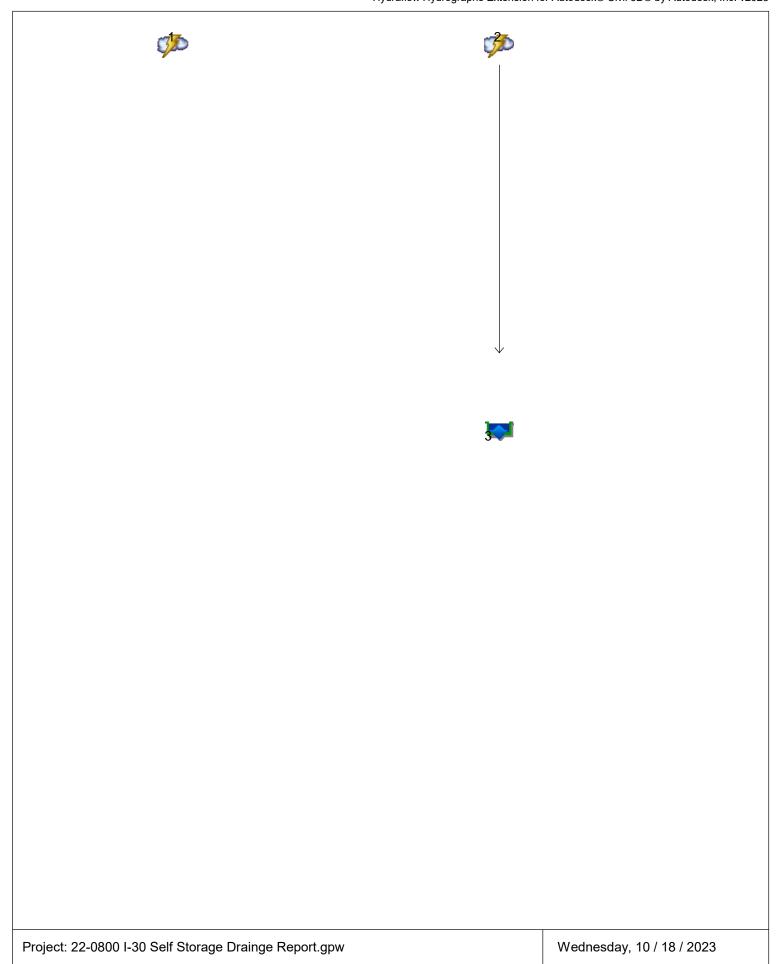
Peak flows for Pre and post development phase of onsite area have been tabulated below-

		Post-Development	Post-Development
	Pre-Development	without Detention	with Detention
	Peak Flow (cfs)	Peak Flow (cfs)	Peak Flow (cfs)
2-Year	53.08	131.14	2.99
5-Year	58.66	147.91	3.498
10-Year	69.15	166.14	4.020
25-Year	79.33	189.21	4.600
50-Year	90.45	213.91	5.051
100-Year	96.16	226.82	5.157
TOC	16.05 min	8.03 min	

CONCLUSION

The onsite drainage calculation for pre and post condition has been provided.

Watershed Model Schematic



lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	53.08	1	16	50,961				Pre-development
2	Rational	131.14	1	8	62,945				Post-Development
2 3	Reservoir	131.14 2.990	1 1	8 16	62,945 57,823	2	349.84	61,739	Post-Development Pond

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Wednesday, 10 / 18 / 2023

Hyd. No. 1

Pre-development

Hydrograph type = Rational Peak discharge = 53.08 cfsStorm frequency = 2 yrsTime to peak = 16 min Time interval = 1 min Hyd. volume = 50,961 cuftDrainage area Runoff coeff. = 28.910 ac= 0.47Tc by User Intensity = 3.907 in/hr= 16.00 min



Hydraflow Hydrographs Extension for Autodesk® Civil							The state of the orange of the state of the		
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	58.66	1	16	56,310				Pre-development
2	Rational	147.91	1	8	70,997				Post-Development
3	Reservoir	3.498	1	16	65,800	2	349.95	69,554	Pond
22-	0800 I-30 Sel	 f Storage	Drainge	Report.g	pwReturn F	Period: 5 Ye	ear	Wednesda	y, 10 / 18 / 2023

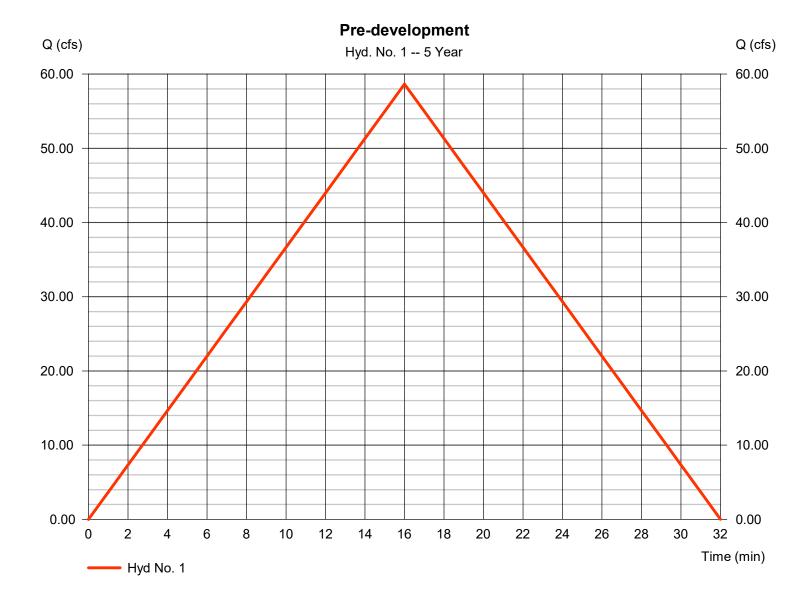
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Wednesday, 10 / 18 / 2023

Hyd. No. 1

Pre-development

Hydrograph type = Rational Peak discharge = 58.66 cfsStorm frequency = 5 yrsTime to peak = 16 min Time interval = 1 min Hyd. volume = 56,310 cuftDrainage area Runoff coeff. = 28.910 ac= 0.47Tc by User Intensity = 4.317 in/hr= 16.00 min



Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	69.15	1	16	66,385				Pre-development
2	Rational	166.14	1	8	79,748				Post-Development
2 3	Reservoir	166.14	1 1	8 16	79,748 74,479	2	350.06	78,053	Post-Development Pond
22-									

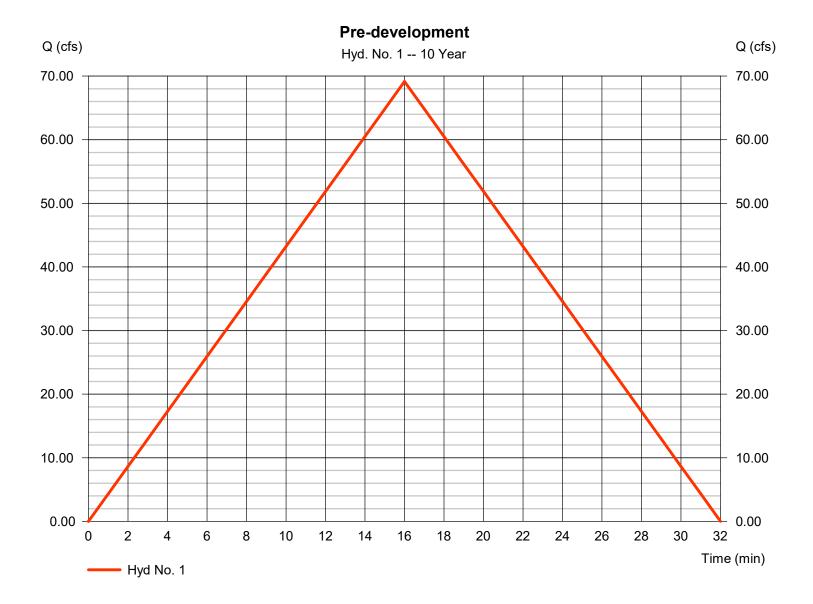
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Wednesday, 10 / 18 / 2023

Hyd. No. 1

Pre-development

Hydrograph type = Rational Peak discharge = 69.15 cfsStorm frequency Time to peak = 10 yrs= 16 min Time interval = 1 min Hyd. volume = 66,385 cuft Drainage area Runoff coeff. = 28.910 ac = 0.47Tc by User = 16.00 min Intensity = 5.089 in/hr



lyd. Io.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	79.33	1	16	76,152				Pre-development
2	Rational	189.21	1	8	90,822				Post-Development
2 3	Reservoir	189.21 4.600	1 1	8 16	90,822	2	350.21	88,823	Post-Development Pond

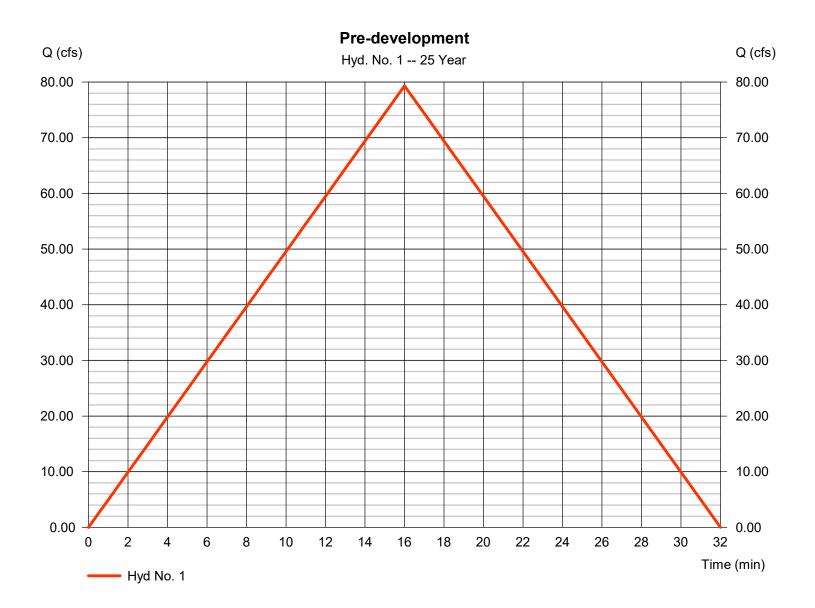
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Wednesday, 10 / 18 / 2023

Hyd. No. 1

Pre-development

Hydrograph type = Rational Peak discharge = 79.33 cfsStorm frequency = 25 yrsTime to peak = 16 min Time interval = 1 min Hyd. volume = 76,152 cuft Drainage area Runoff coeff. = 28.910 ac = 0.47Tc by User = 16.00 min Intensity = 5.838 in/hr



lyd. Io.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	90.45	1	16	86,827				Pre-development
2	Rational	213.91	1	8	102,677				Post-Development
2 3	Reservoir	213.91 5.051	1 1	8 16	102,677 97,246	2	350.36	100,388	Post-Development Pond

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

= Bryant 50.IDF

Wednesday, 10 / 18 / 2023

Hyd. No. 1

Pre-development

Hydrograph type = Rational Peak discharge $= 90.45 \, \text{cfs}$ Storm frequency Time to peak = 50 yrs= 16 min Time interval = 1 min Hyd. volume = 86,827 cuft Drainage area Runoff coeff. = 28.910 ac = 0.47Tc by User = 16.00 min Intensity = 6.656 in/hr**IDF** Curve Asc/Rec limb fact = 1/1

Pre-development Q (cfs) Q (cfs) Hyd. No. 1 -- 50 Year 100.00 100.00 90.00 90.00 80.00 80.00 70.00 70.00 60.00 60.00 50.00 50.00 40.00 40.00 30.00 30.00 20.00 20.00 10.00 10.00 0.00 0.00 2 6 8 10 12 14 16 18 20 22 24 26 28 30 32 Time (min) Hyd No. 1

łyd. ło.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	96.16	1	16	92,318				Pre-development
2	Rational	226.82	1	8	108,874				Post-Development
2 3	Reservoir	226.82 5.157	1 1	8 16	108,874	2	350.44	106,461	Post-Development Pond

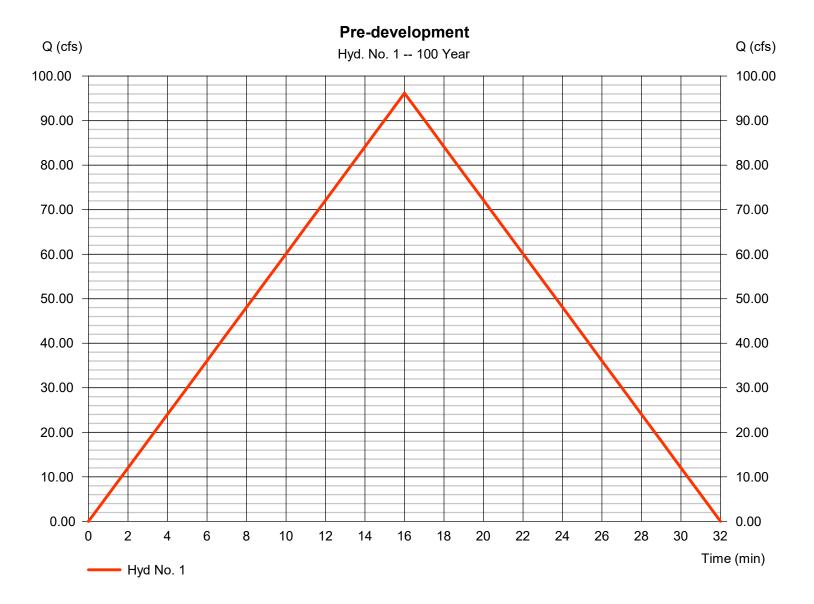
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Wednesday, 10 / 18 / 2023

Hyd. No. 1

Pre-development

= 96.16 cfsHydrograph type = Rational Peak discharge Storm frequency = 100 yrsTime to peak = 16 min Time interval = 1 min Hyd. volume = 92,318 cuft Drainage area Runoff coeff. = 28.910 ac = 0.47Tc by User Intensity = 7.077 in/hr= 16.00 min



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

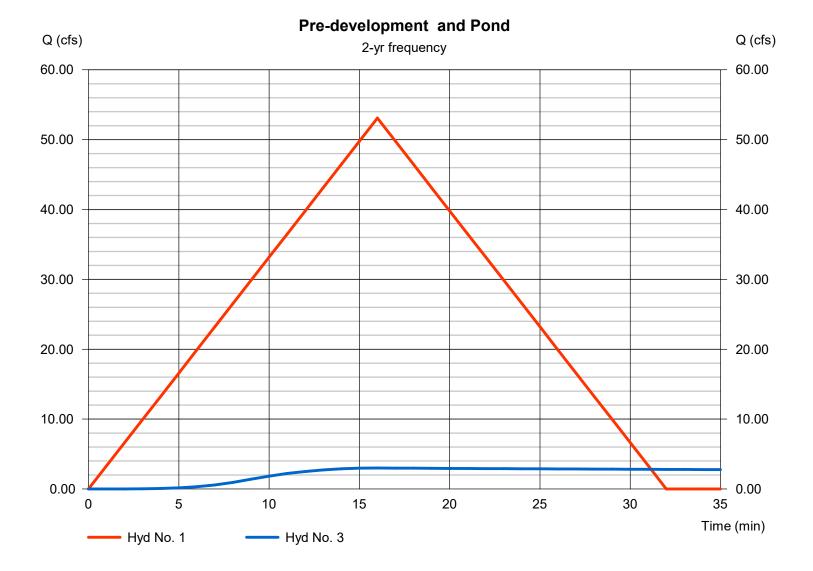
Pre-development

Hydrograph type = Rational
Peak discharge = 53.08 cfs
Time to peak = 16 min
Hyd. Volume = 50,961 cuft

Hyd. No. 3

Pond

Hydrograph type = Reservoir
Peak discharge = 2.99 cfs
Time to peak = 16 min
Hyd. Volume = 57,823 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

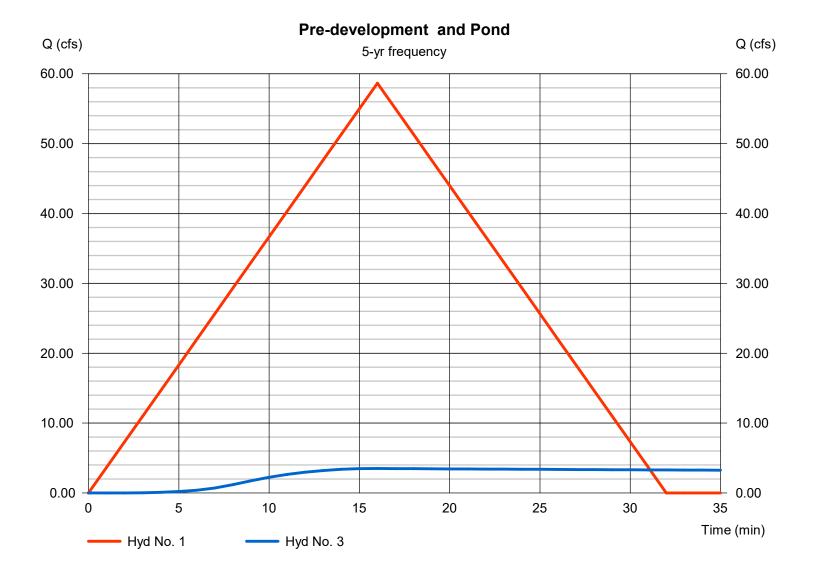
Pre-development

Hydrograph type = Rational
Peak discharge = 58.66 cfs
Time to peak = 16 min
Hyd. Volume = 56,310 cuft

Hyd. No. 3

Pond

Hydrograph type = Reservoir
Peak discharge = 3.50 cfs
Time to peak = 16 min
Hyd. Volume = 65,800 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

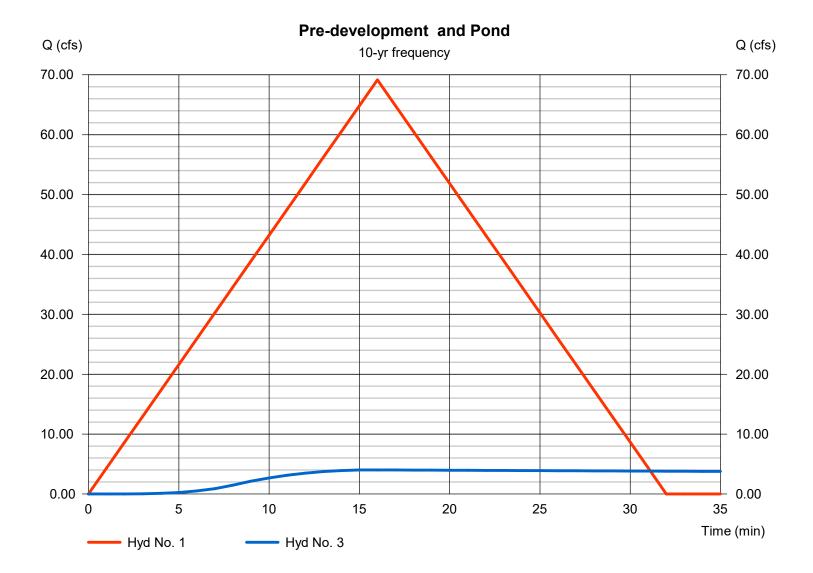
Pre-development

Hydrograph type = Rational
Peak discharge = 69.15 cfs
Time to peak = 16 min
Hyd. Volume = 66,385 cuft

Hyd. No. 3

Pond

Hydrograph type = Reservoir
Peak discharge = 4.02 cfs
Time to peak = 16 min
Hyd. Volume = 74,479 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

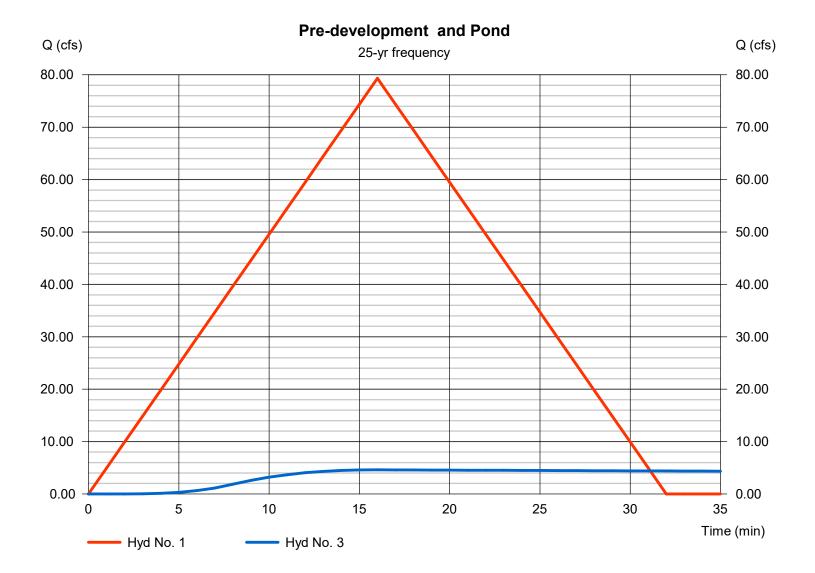
Pre-development

Hydrograph type = Rational
Peak discharge = 79.33 cfs
Time to peak = 16 min
Hyd. Volume = 76,152 cuft

Hyd. No. 3

Pond

Hydrograph type = Reservoir
Peak discharge = 4.60 cfs
Time to peak = 16 min
Hyd. Volume = 85,472 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

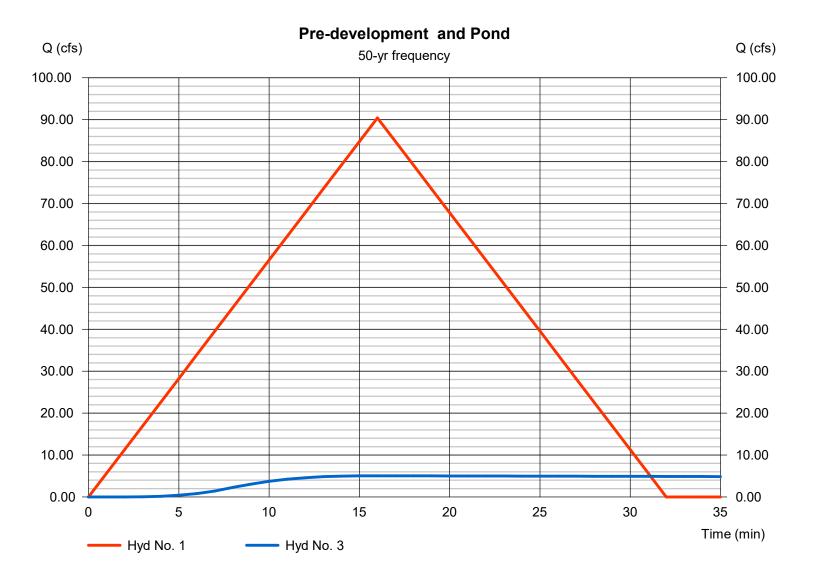
Pre-development

Hydrograph type = Rational
Peak discharge = 90.45 cfs
Time to peak = 16 min
Hyd. Volume = 86,827 cuft

Hyd. No. 3

Pond

Hydrograph type = Reservoir
Peak discharge = 5.05 cfs
Time to peak = 16 min
Hyd. Volume = 97,246 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

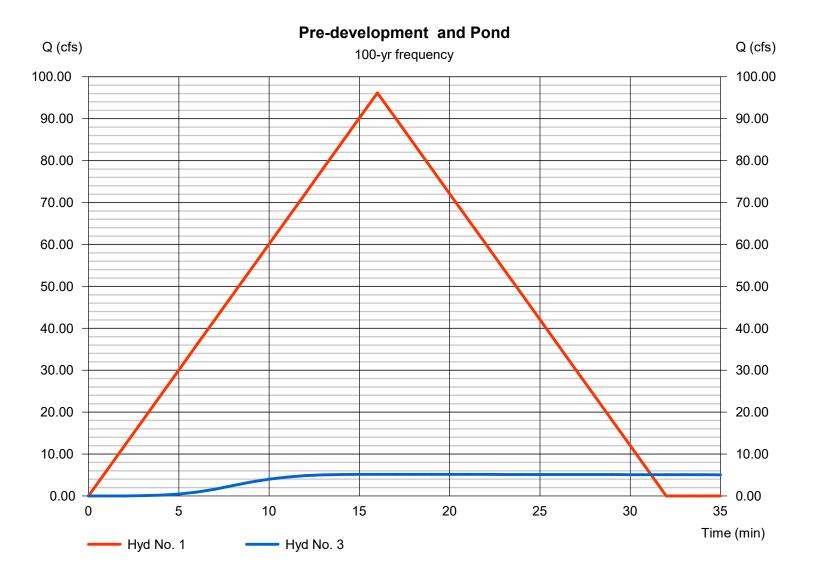
Hyd. No. 1

Pre-development

Hydrograph type = Rational Peak discharge = 96.16 cfs Time to peak = 16 min Hyd. Volume = 92,318 cuft Hyd. No. 3

Pond

Hydrograph type = Reservoir
Peak discharge = 5.16 cfs
Time to peak = 16 min
Hyd. Volume = 103,403 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

Thursday, 10 / 26 / 2023

Pond No. 1 - <New Pond>

Pond Data

Trapezoid -Bottom L x W = 412.0 x 175.0 ft, Side slope = 2.00:1, Bottom elev. = 349.00 ft, Depth = 5.00 ft

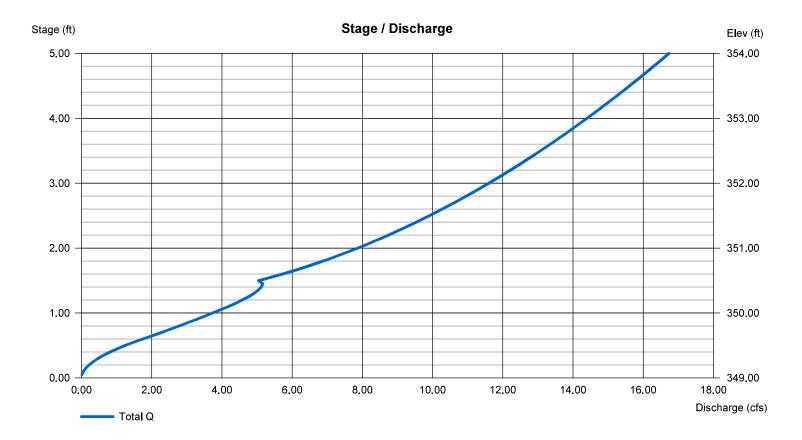
Stage / Storage Table

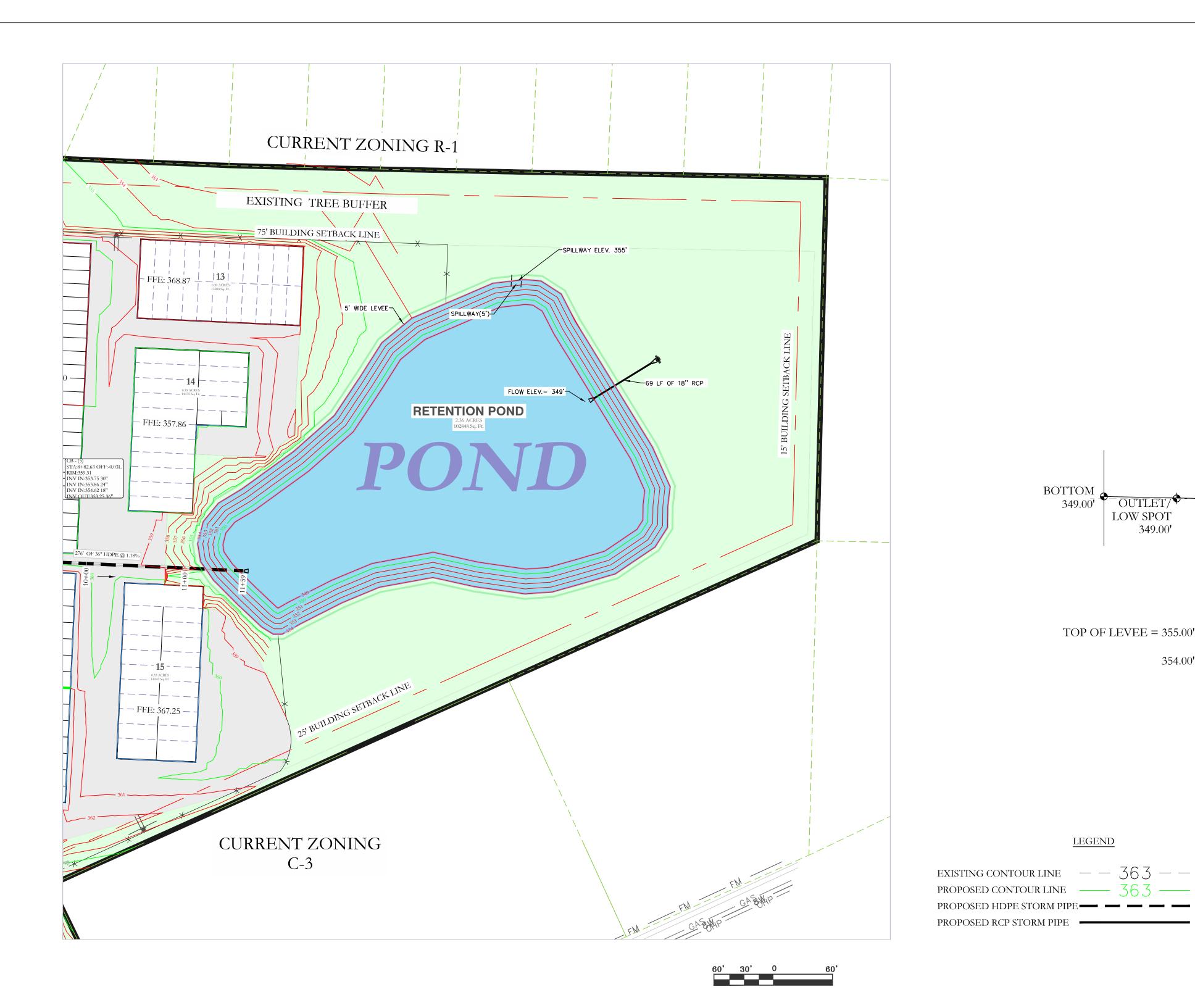
Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	349.00	72,100	0	0
0.50	349.50	73,278	36,344	36,344
1.00	350.00	74,464	36,935	73,279
1.50	350.50	75,658	37,530	110,810
2.00	351.00	76,860	38,129	148,939
2.50	351.50	78,070	38,732	187,671
3.00	352.00	79,288	39,339	227,010
3.50	352.50	80,514	39,950	266,960
4.00	353.00	81,748	40,565	307,525
4.50	353.50	82,990	41,184	348,710
5.00	354.00	84,240	41,807	390,517

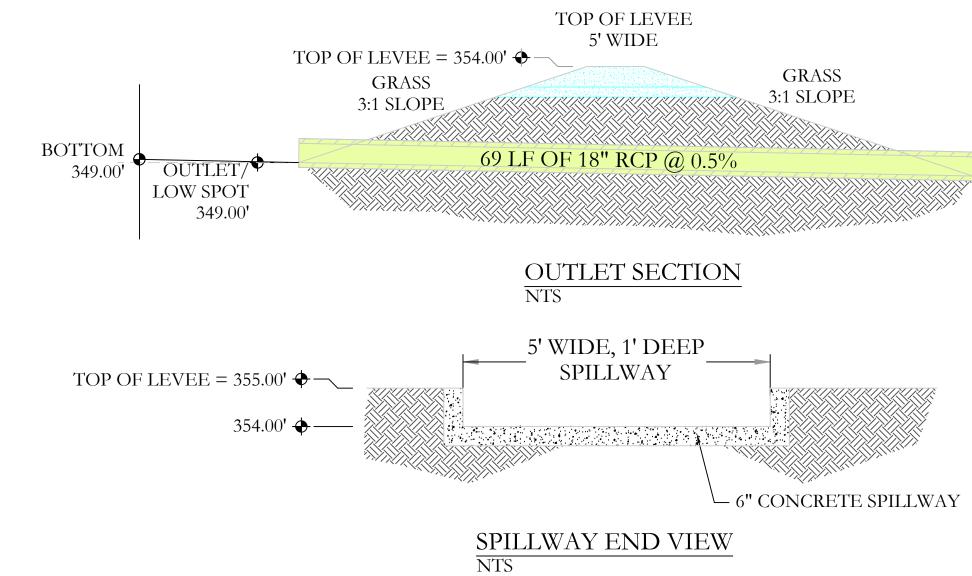
Culvert / Orifice Structures Weir Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 18.00	Inactive	Inactive	0.00	Crest Len (ft)	= 5.00	0.00	0.00	0.00
Span (in)	= 18.00	18.00	0.00	0.00	Crest El. (ft)	= 355.00	0.00	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 349.00	347.00	0.00	0.00	Weir Type	= Rect			
Length (ft)	= 70.00	30.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 0.50	0.50	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Wet area)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00			

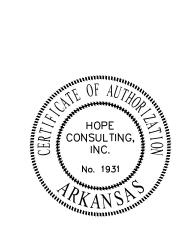
Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

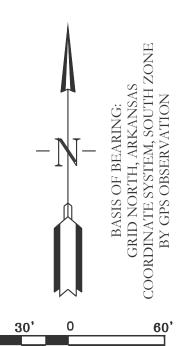






<u>LEGEND</u>





VICINITY MAP:

CONSULTING ENGINEERS + SURVEYORS

129 N. Main Street, Benton, Arkansas 72015 PH. (501)315-2626 FAX (501) 315-0024 www.hopeconsulting.com

FOR USE AND BENEFIT OF: STUART FINLEY

ARKANSAS STORAGE CENTER

RETENTION POND PLAN BRYANT, SALINE COUNTY, ARKANSAS

DATE:	10-25-2023		C.A.D. BY:					DRAWING NUMBER:			
REVISED:	EVISED: CHECKED BY:								0000		
SHEET:	C-4.4		SCALE:						-0800		
500	00 01S		14W	0	21	300)	62	1762		



Background

There will be one retention pond in this project. The retention pond is located at the North-East of the subject property. It is designed to temporarily detain stormwater to meet water quantity criteria before discharging off the property.

Routine Maintenance

The property owners association will maintain the drainage easements. Routine maintenance will include but not be limited to: -Mowing of the bank slopes and area around the pond on a monthly basis during the growing season and as needed during the cooler months.

-The outlet pipe from the pond and other areas will be inspected monthly for debris which could inhibit the proper flow of discharge. Any debris will be removed immediately and disposed of or placed in a location to prevent future maintenance and prevent future erosion. to not cause impact up or downstream of the structure.

-Trash will be removed from around the pond to prevent entering the pond. Generally, the site should be kept free of loose trash which could be carried off site by wind or rain.

-Inspect the pond and outlet pipe for non-routine maintenance need.

Periodic or Non-Routine Maintenance

The routine inspection of the pond area and discharge pipe will identify needed repairs and non-routine maintenance. These items may include but not be limited to:

-Re-growth of trees on or around the pond bank. These should be cut and removed from the pond area.

-Sediment from the site may accumulate in the pond bottom and reduce the pond to below design volume requirements. The pond should be excavated if the pond bottom elevation reached a level that allows excessive aquatic growth or reduces the pond efficiency such, that the sediments are passing the discharge structure and release off site.

-Stabilization or re-grading of side slopes may be required periodically or after excessive rain events. Any disturbance of slopes should be reseeded or may require installation of erosion control materials until seeding can reestablish adequate grasses to

-Any other maintenance or repairs which would minimize other maintenance to the pond or outfall structures.

