Bryant Planning Commission Meeting

Boswell Municipal Complex - City Hall Court Room

210 SW 3rd Street

YouTube: https://www.youtube.com/c/bryantarkansas

Date: September 08, 2025 - **Time:** 6:00 PM

Call to Order

Approval of Minutes

1. Planning Commission Meeting Minutes 8/11/2025

- 2025-08-11 Planning Commission Minutes.pdf

Presentations and Announcements

2. Water and Wastewater Rate Analysis Presentation

- 2025 09 02 Bryant Rate Study Committee Presentation.pdf

DRC Report

3. FSBC - New Site Additions - 604 Reynolds - Changes to Outfall of Retention Pond

Hope Consulting - Requesting Approval for changes to the retention pond outfall - APPROVED, Contingent upon small comments being worked out with city engineer

- 0912-DRN-03.pdf
- 0912-DRN-04.pdf

4. State Farm - 515 N Reynolds Road - Site Plan Changes

Richardson Engineering - Requesting Approval for modification to previously approved Site Plan - APPROVED

- 0956-DRN-04.pdf
- 0956-PLN-05.pdf

5. New Beginnings - HWy 5 and Midland Rd - Site Plan

PLE - Requesting Site Plan Approval - APPROVED

- 0977-PLN-03.pdf
- 0977-DRN-03.pdf
- 0977-SLC-01.pdf

6. 20 Tanglewood Dr - Conditional Use Permit - Additional Square Footage for Addition to Accessory Structure

David Harris - Requesting Approval of Conditional Use Permit - RECOMMENDED APPROVAL

Public Hearing

7. 20 Tanglewood Dr - Conditional Use Permit - Additional Square Footage for Addition to Accessory Structure

David Harris - Requesting Approval of Conditional Use Permit

- 0983-PUB-01.pdf
- 0983-PLN-01.pdf
- 0983-APP-01.pdf

Old Business

New Business

Adjournments

Bryant Planning Commission Meeting Minutes
Monday, August 11, 2025
Boswell Municipal Complex – City Hall Courtroom
6:00 PM

Agenda

CALL TO ORDER

- Chairman Lance Penfield calls the meeting to order.
- Commissioners Present: Statton, Hooten, Penfield, Johnson, Thompson, Edwards, Erwin, Speed
- Commissioners Absent: None

ANNOUNCEMENTS

Ted Taylor, Directory of Planning & Development, reported that the City of Bryant is in contract negotiations with Crafton Tull for the Comprehensive Growth Plan. Once initiated, a steering committee will be formed.

Ted Taylor also announced that Public Works is developing its Capital Improvement Plan and will present the Water/Wastewater Master Plan at the next Planning Commission Meeting on September 8.

APPROVAL OF MINUTES

1. **Planning Commission Meeting Minutes 7/14/2025**

   Motion to Approve Minutes made by Commissioner Statton, Seconded by Commissioner Edwards. Voice Vote, 8 Yays, 0 Nays, 0 Absent

Vice-Chairman Hooten read the DRC Report.

DRC REPORT

2. **Creekside Addition Phase 2 - Replat -** Lot 15 and Tract E
   GarNat Engineering - Requesting Recommendation for Approval of Replat - RECOMMENDED APPROVAL, Contingent upon remaining comments being addressed.

3. **The Shoppes at Dogwood Springs -** Commercial Plat
   Richardson Engineering - Requesting Recommendation for Approval of Commercial Plat - RECOMMENDED APPROVAL

4. **Sky Blue Duplexes Subdivision -** Final Plat
   Hope Consulting - Requesting Recommendation for Final Plat Approval - RECOMMENDED APPROVAL, Contingent upon remaining comments being addressed.

5. **Bryant Schools - ALE Addition -** 1200 S Reynolds Road
   Josh Minton - Requesting Approval for Site Plan Addition - APPROVED

6. **Good Day Farms - 3205 HWY 5 -** Site Plan
   Regan Ethridge - Requesting Site Plan Approval - APPROVED, Contingent upon completing the remaining comments.

7. **Kinfolk Acres - 22000 I-30 -** Sign Permit
   Ace Sign Company - Requesting Sign Permit Approval - STAFF APPROVED

8. **Wine Night - 6221 Hwy 5 -** Sign Permit
   Amy Clark - Requesting Sign Permit Approval - STAFF APPROVED

9. **FCPS - 212 McClanahan Dr -** Sign Permit
   Lumatech - Requesting Sign Permit Approval - STAFF APPROVED

10. **Childcare Network - 2168 N Prickett -** Sign Permit
    Pinnacle Signs - Requesting Sign Permit approval for freestanding monument sign. The two facade signs in the application meet the code and have been approved by staff. - APPROVED, contingent upon remaining 7.5ft from edge of sign foundation to waterline.

11. **Goodwill - 5095 Hwy 5 -** Sign Permit
    Ace Sign Company - Requesting Sign Permit Approval - APPROVED, contingent upon remaining 7.5ft from edge of sign foundation to waterline.

12. **Attorney's Title Group - 3125 Hwy 5, STE 3 -** Sign Permit
    Condray Signs - Requesting Sign Permit Approval - STAFF APPROVED

13. **Gen Wealth Financial - 4756 Bryant Parkway -** Sign Permit
    Ace Sign Company - Requesting Sign Permit Approval - STAFF APPROVED

OLD BUSINESS

None

NEW BUSINESS

14. **Creekside Addition Phase 2 - Replat -** Lot 15 and Tract E

    GarNat Engineering - Requesting Approval for Replat

    After discussion on the item, Chairman Penfield called for a roll call vote to approve. 8 Yays, 0 Nays, 0 Absent.

15. **The Shoppes at Dogwood Springs -** Commercial Plat

    Richardson Engineering - Requesting Approval for Commercial Plat

    After discussion on the item, Chairman Penfield called for a roll call vote to approve. 7 Yays, 0 Nays, 1 Abstained, 0 Absent.

16. **Sky Blue Duplexes Subdivision -** Final Plat

    Hope Consulting - Requesting Final Plat Approval

    After discussion on the item, Chairman Penfield called for a roll call vote to approve. 8 Yays, 0 Nays, 0 Absent.

17. **REQUEST TO ADD: Blessings Addition Subdivision -** Replat of Lots 2, 3, 4 into Lots 1A and 1B

    PLE - Requesting Approval for Replat

    Motion to add agenda item, Blessings Addition Subdivision, made by Commissioner Statton, Seconded by Commissioner Speed. Voice Vote, 8 Yays, 0 Nays, 0 Absent

    After discussion on the item, Chairman Penfield called for a roll call vote to approve, subject to the correction of "Lots 1A and 1B" to "Lots 2R and 3R". 8 Yays, 0 Nays, 0 Absent.

18.

ADJOURNMENT

Motion to Adjourn made by Vice-Chairman Hooten, seconded by Commissioner Edwards. Voice Vote, 8 Yays, 0 Nays, 0 Absent. The meeting was adjourned.

_____ _____

Chairman, Lance Penfield *Date*

_____ _____

Secretary, Rebecca Kidder *Date*

2025 WATER AND WASTEWATER RATE STUDY
WATER ADVISORY COMMITTEE PRESENTATION

September 2025

WILLDAN

Updated: August 25 2025

AGENDA

- Background on Rates
- Customers and Volumes
- Current and Forecast Cost of Service
- Initial Rate Plan Scenarios
- Summary

BACKGROUND ON RATES

Facts about Water and Wastewater Service in the 21st Century

- Average utility has been increasing rates 5-6% per year, a trend that is expected to continue
- American Water Works Association (AWWA) forecasts that water and wastewater rates across the U.S. will **triple** over the next 15 years
- Rate adjustments are primarily due to reasons beyond a utility's direct control – inflation, necessary Capital Improvement Plans, wholesale costs, and other indirect expenses
- 30-40% of utilities charge rates that **do not cover their costs**

CURRENT RATE STRUCTURE | EFFECTIVE JAN 2025

WATER RATE STRUCTURE	
Base Charge by Meter Size (includes 2,000 Gal for City customers)	RES and NON-RES
5/8”	$14.53
1”	21.80
1 1/2”	36.33
2”	72.65
3”	116.24
4”	217.96
6” and Above	726.53

WATER RATE STRUCTURE			
Volume Rate per 100 Gal	RES and NON-RES	COB PARKS	WOOD HILLS
All Gal	$0.698	$0.2166*	$0.2166*

***same rate as CAW**

WW RATE STRUCTURE			
MIN Charge (includes 2,000 Gal)	RES and NON-RES	SALEM SEWER**	QUAIL RIDGE**
All Meter Sizes	$20.70	$41.40	$41.40

WW RATE STRUCTURE			
Volume Rate per 100 Gal	RES and NON-RES	SALEM SEWER	QUAIL RIDGE
All Gallons	$1.104	$1.104	$1.104

WW RATE STRUCTURE – SPECIAL RATE	
Volume Rate per 1 Gal	SALINE COUNTY LANDFILL
Volume rate per 1 Gal	$0.0575

****OCL sewer customers pay 2x times inside base charge**

SYSTEM INFRASTRUCTURE FEE | EFFECTIVE JAN 2025

SYSTEM INFRASTRUCTURE FEE	
Based on Water Meter Size	ALL SEWER CUSTOMERS
5/8”	$15.75
1”	52.50
1 1/2”	105.00
2”	168.00
3”	336.00
4”	530.25
6” and Above	1060.50

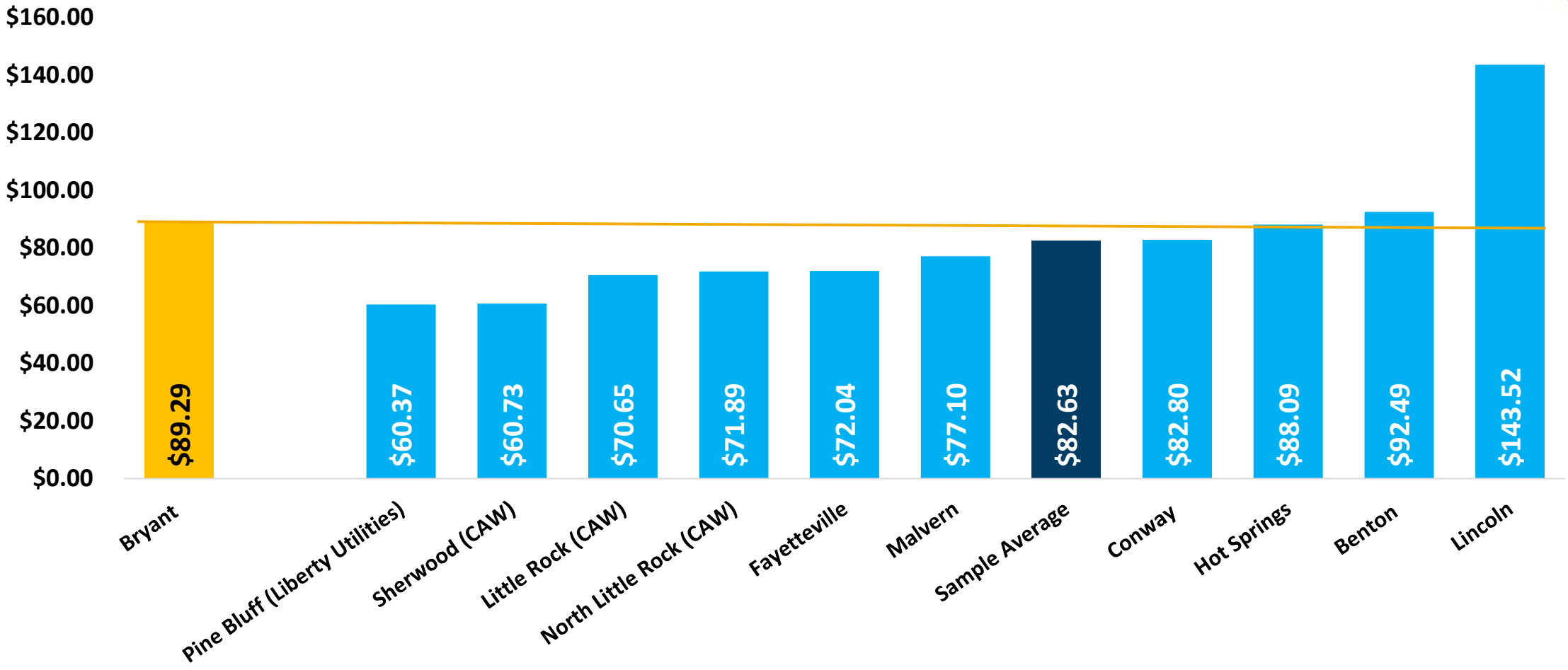
System Infrastructure Fee was adopted in 2023 and is assessed to ALL sewer accounts

The fee is used for repayment of sewer or water system bonds and loans

The fee is charged in addition to all other water and sewer rates and charges

The fee is set to automatically increase by 5% each year

CURRENT MONTHLY CHARGES | 5KGAL WATER & WW



*Rates as of May 2025
Average RES water use: **4,470** Gal/month

NOTE: Does not include wastewater infrastructure fee

CUSTOMERS & VOLUMES

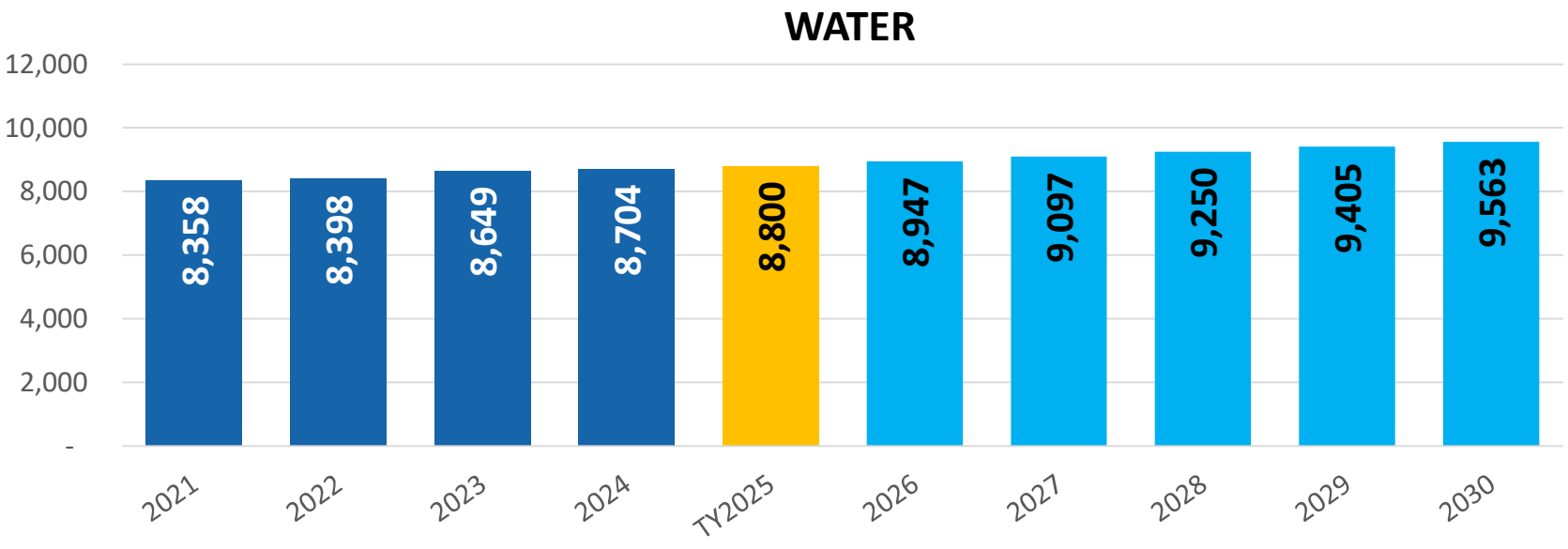
NUMBER OF ACCOUNTS | TEST YEAR 2025

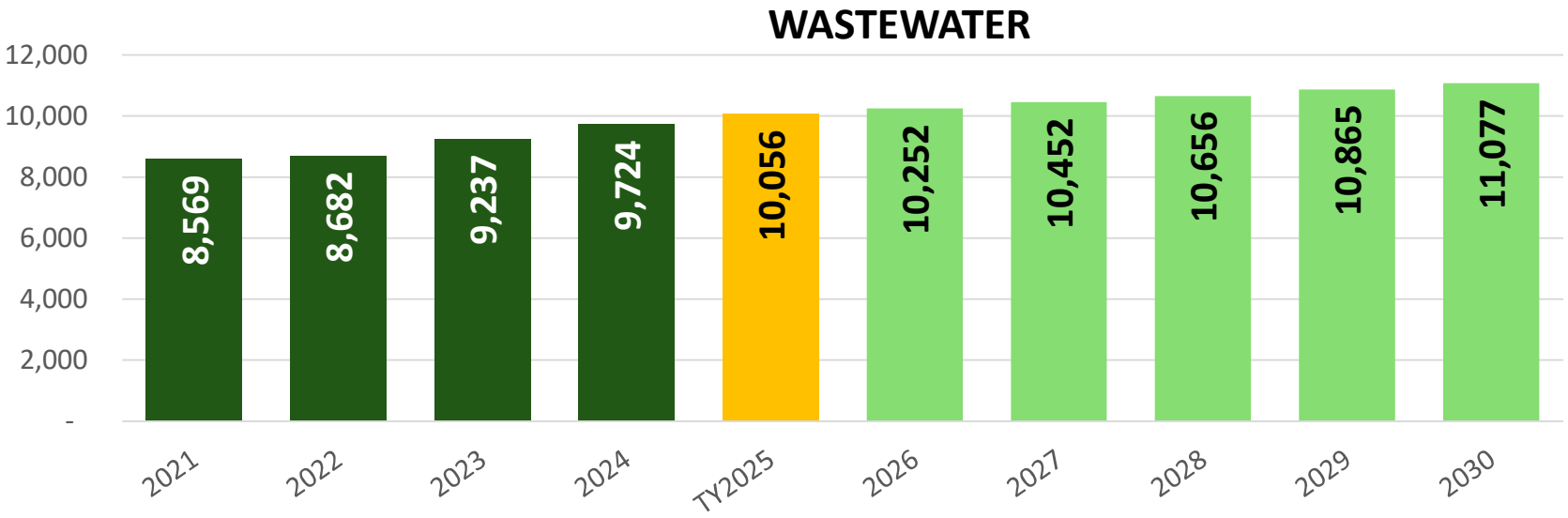
WATER Customer Accounts	
Residential & Non-Residential	8,376
Sprinkler	308
Multy-Family	99
Wholesale (COB Parks & Rec)	7
Woodland Hills*	1
No Charge	9
Total Accounts	**8,800**

WASTEWATER Customer Accounts	
Residential & Non-Residential	7,894
Salem Metered Sewer	1,414
Salem Sewer	701
Quail Ridge	46
Drain Water (County Landfield)	1
Total Accounts	**10,056**

* 1 Master meter serves 529 connections

ACCOUNT FORECAST | WATER AND WASTEWATER



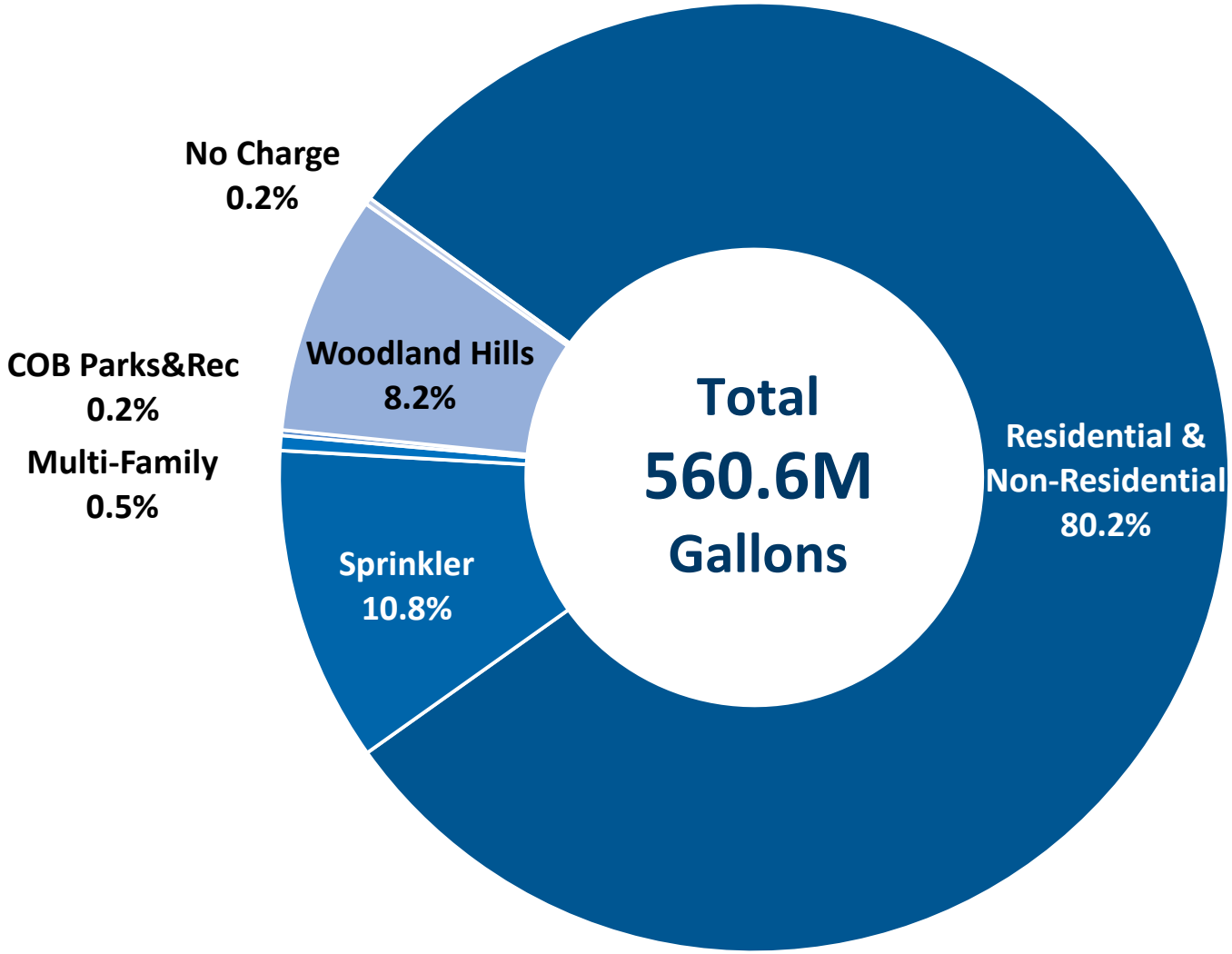


Project team is forecasting 1.7% annual water account growth rate – 158 new accounts per year

Wastewater accounts are growing faster as Bryant expands sewer services to the County

Average wastewater growth rate is 2.0% -- 213 accounts per year

TEST YEAR | WATER CONSUMPTION BY RATE CLASS

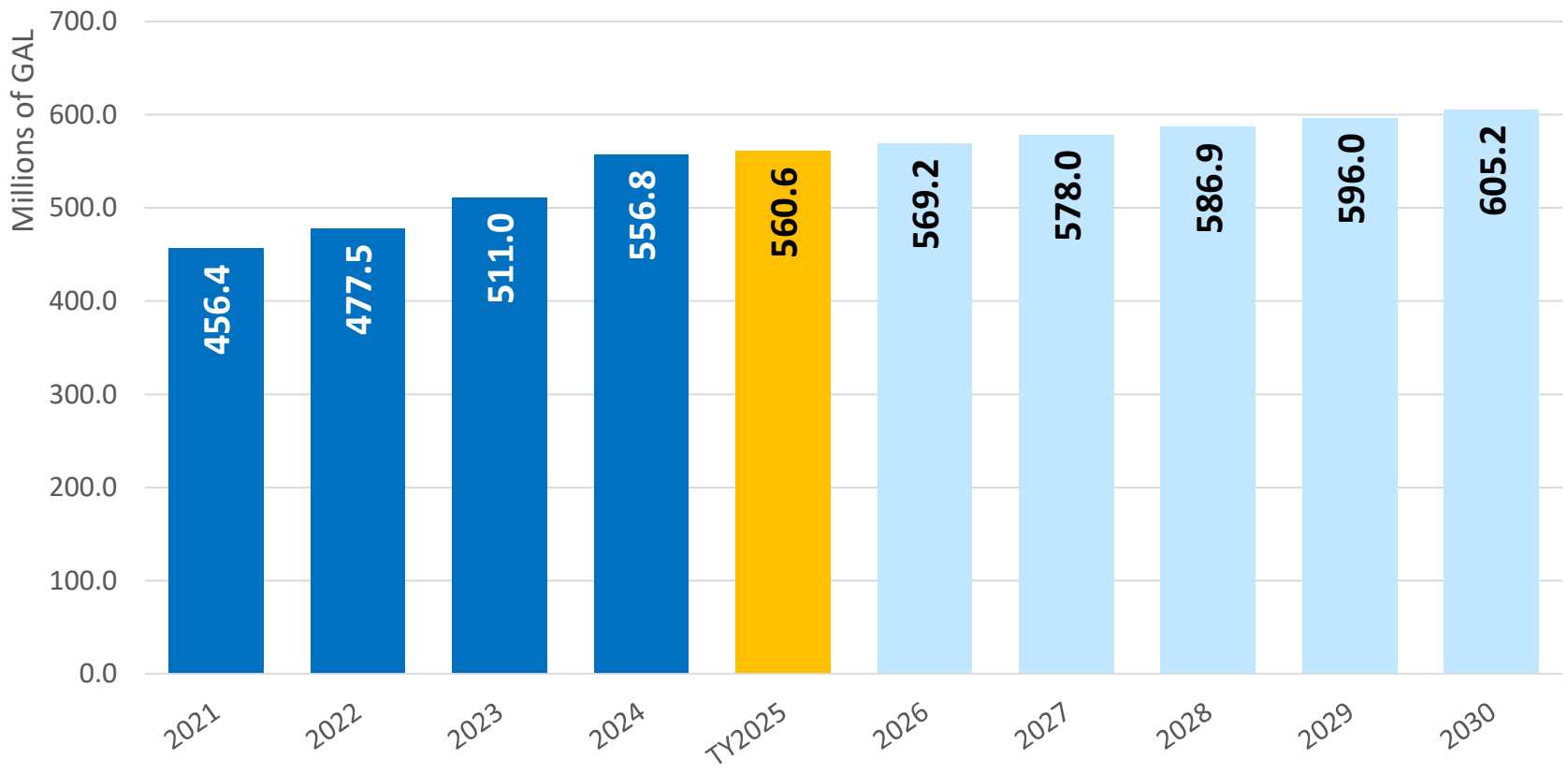


Residential and Non-Residential customer class comprises a clear majority of consumption and accounts within the City

We are not forecasting any significant change to the consumption by class makeup

Average Monthly Residential Consumption per Account = **4,470** gallons

CONSUMPTION FORECAST| WATER



Water consumption is forecast to increase by 1.55% per year

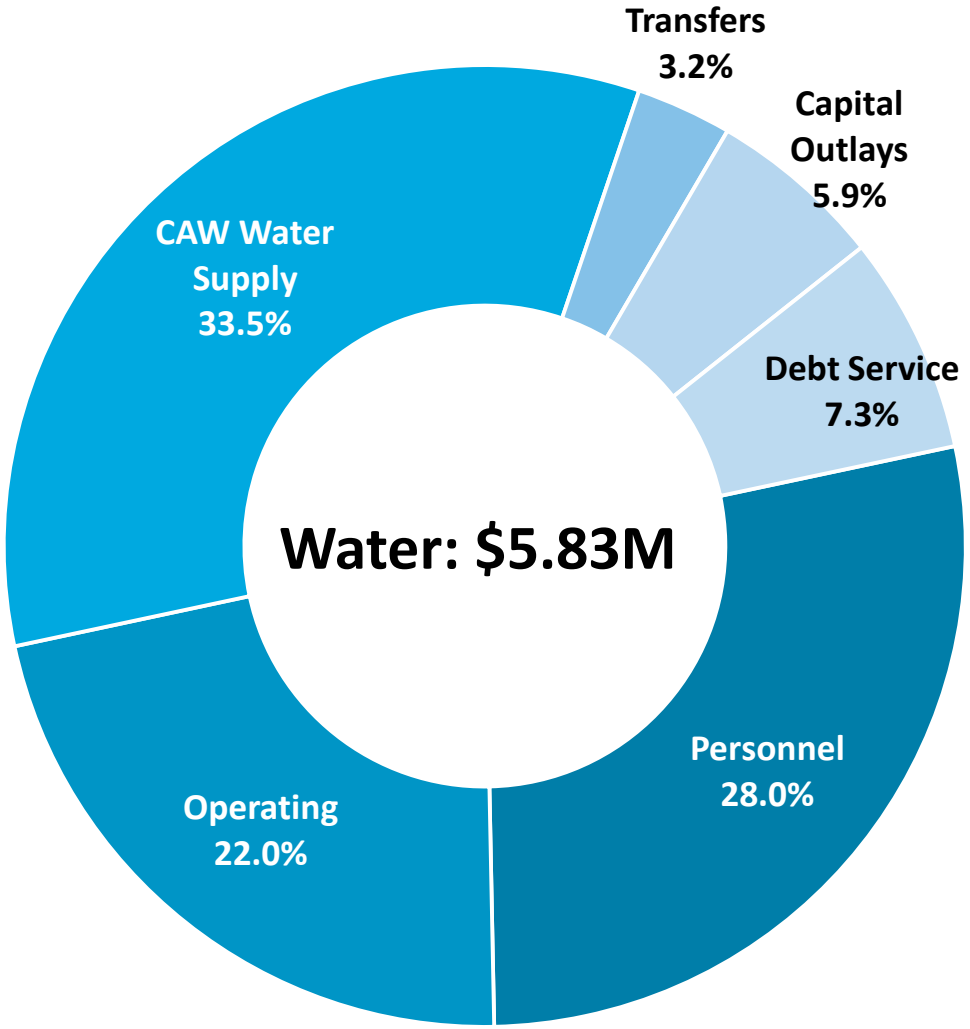
Actual water use may vary considerably with weather patterns. The model is based on the overall trend

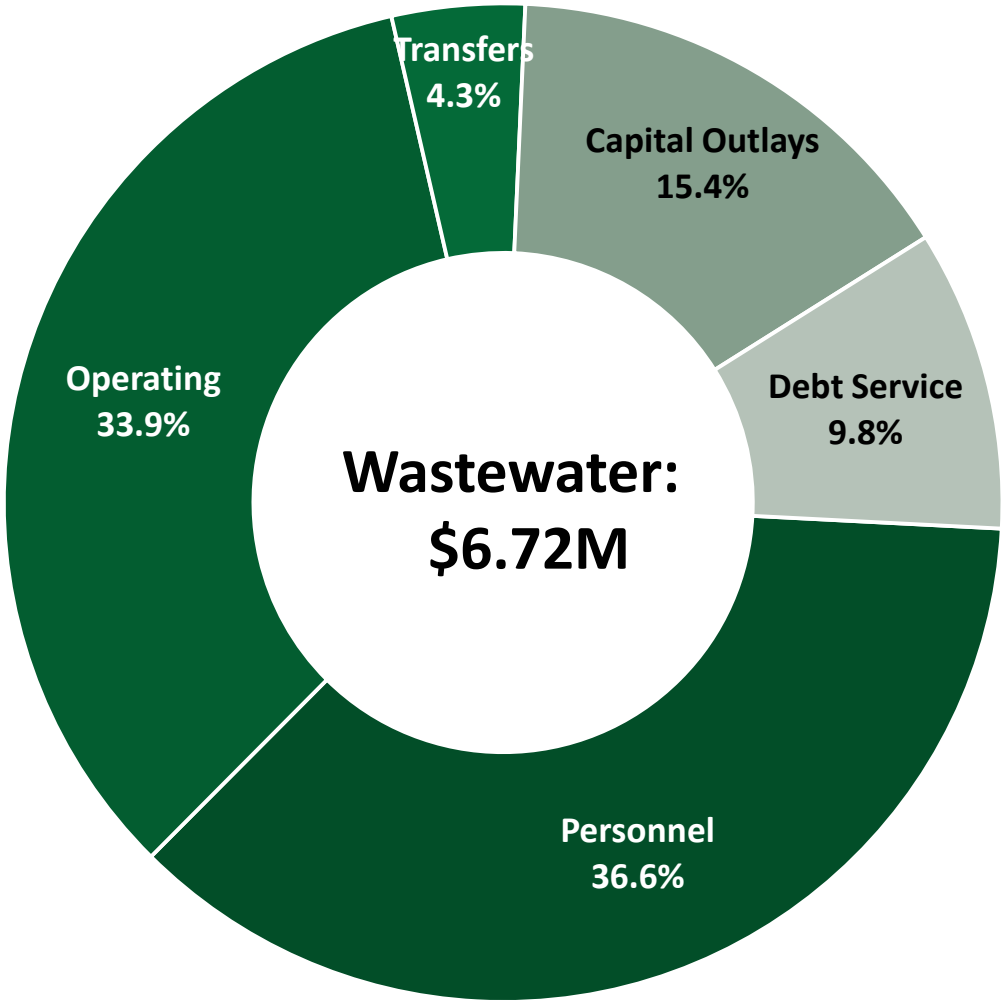
WATER & WASTEWATER
CURRENT AND FORECAST COST OF SERVICE

KEY ASSUMPTIONS DRIVING RATE PLAN

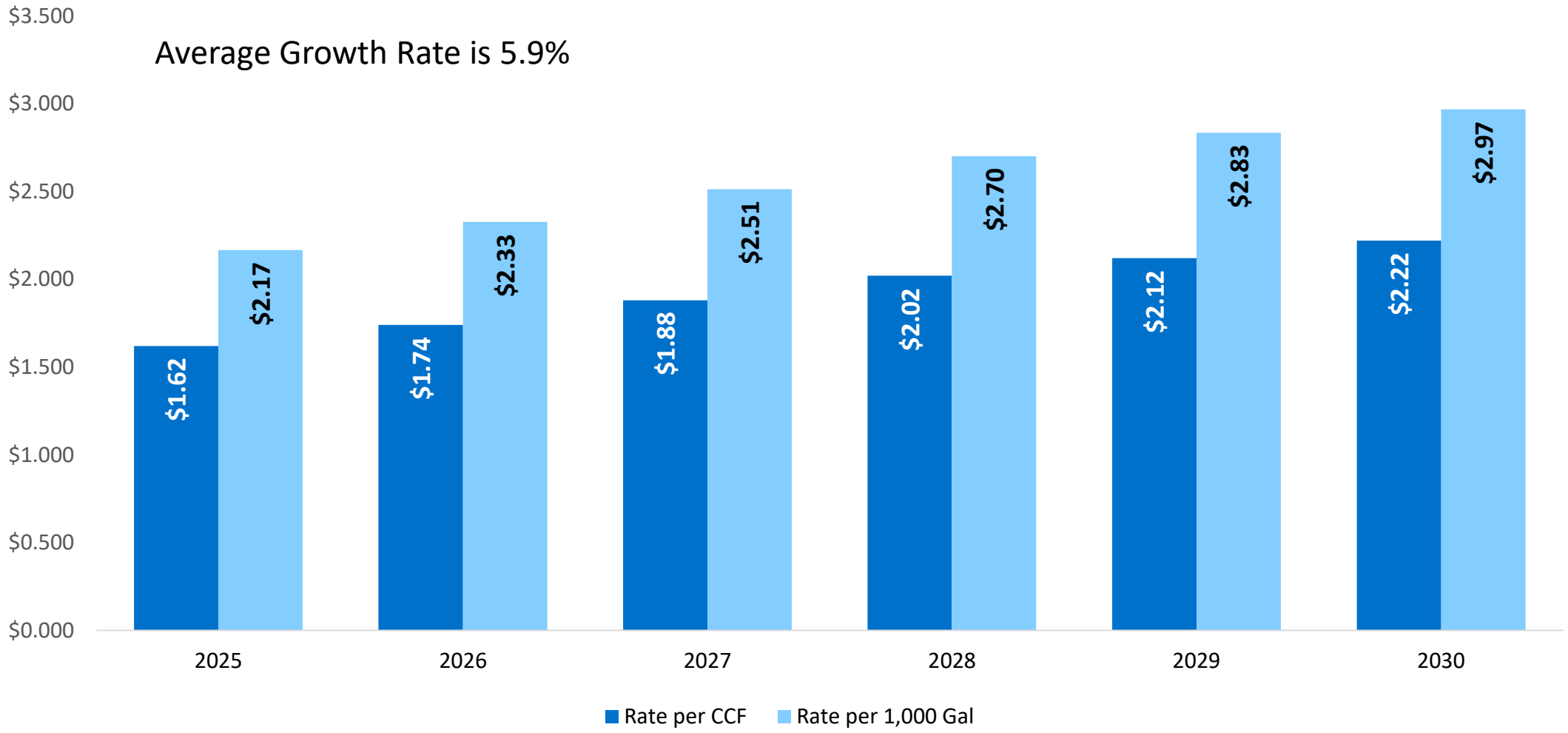
- Most operating expenses increase at 3% per year; certain expenses increase at a higher rate (e.g. insurance, fuel, chemicals, etc.)
- 1 FTE is added to water utility and 1 FTE to WW utility each year for the next 5 years
- Identified near- and mid-term Water CIP (2025-2030): **$61.80 million**
- CIP primarily funded through long-term debt: **20-years, 4% int.**
- 100% of system infrastructure fee revenue is used for debt payments

TEST YEAR 2025 COST OF SERVICE

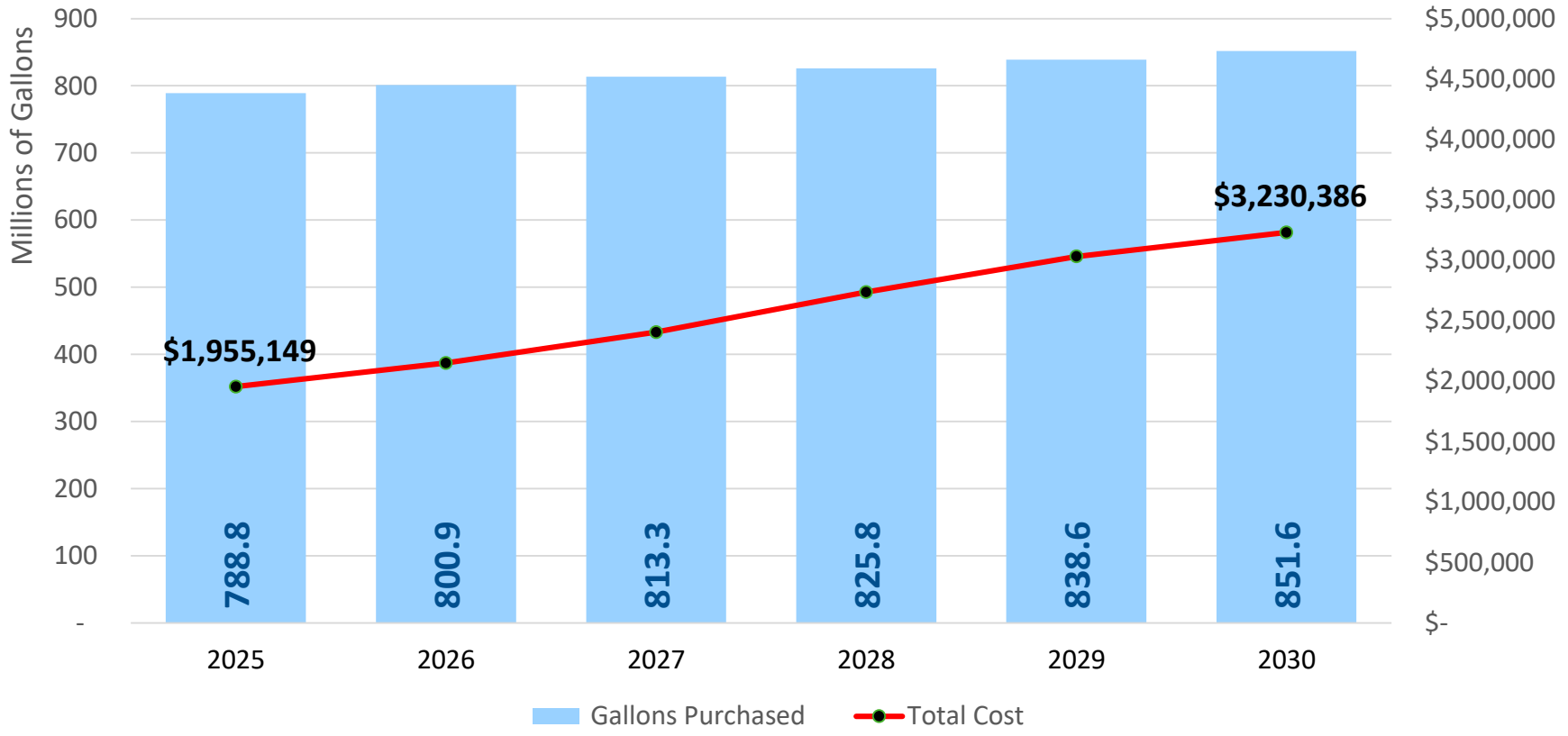




CENTRAL ARKANSAS WATER RATE FORECAST



CAW FORECAST WATER COSTS



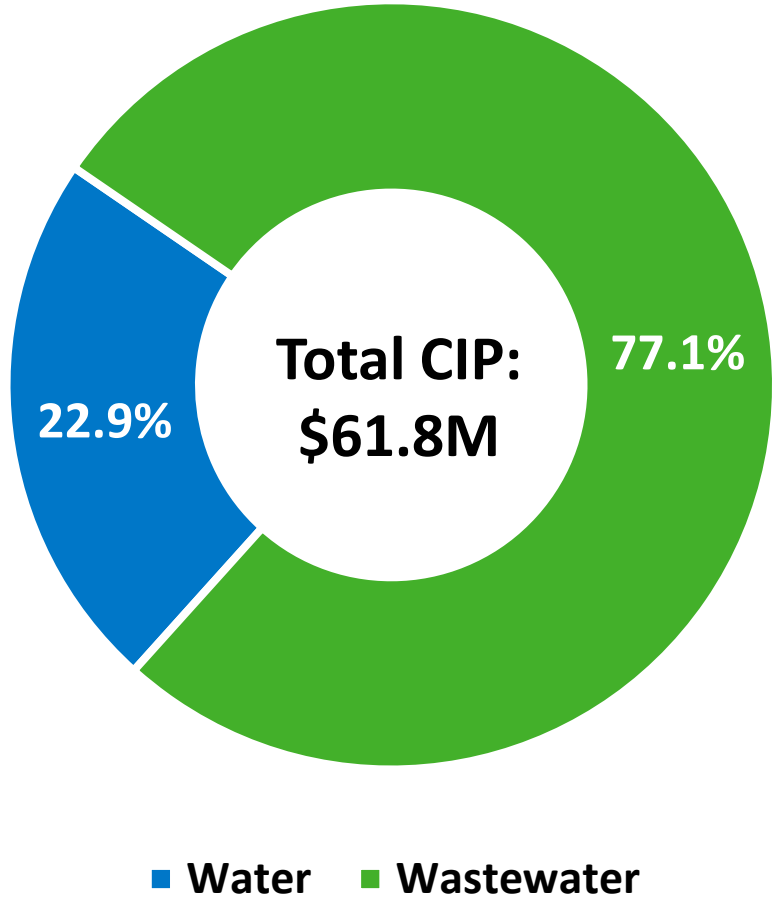
CAW forecast water costs are increasing at a higher pace than purchased volumes due to a combination of CAW rate and CAW infrastructure fee increases

Volume Growth: 1.6%
CAW Rate Increase: 5.9%
CAW Infrastructure Fee Increase: 22.8%
CAW Watershed Protection Fee Increase: 0.0%
CAW Total Water Costs Increase: 8.8%

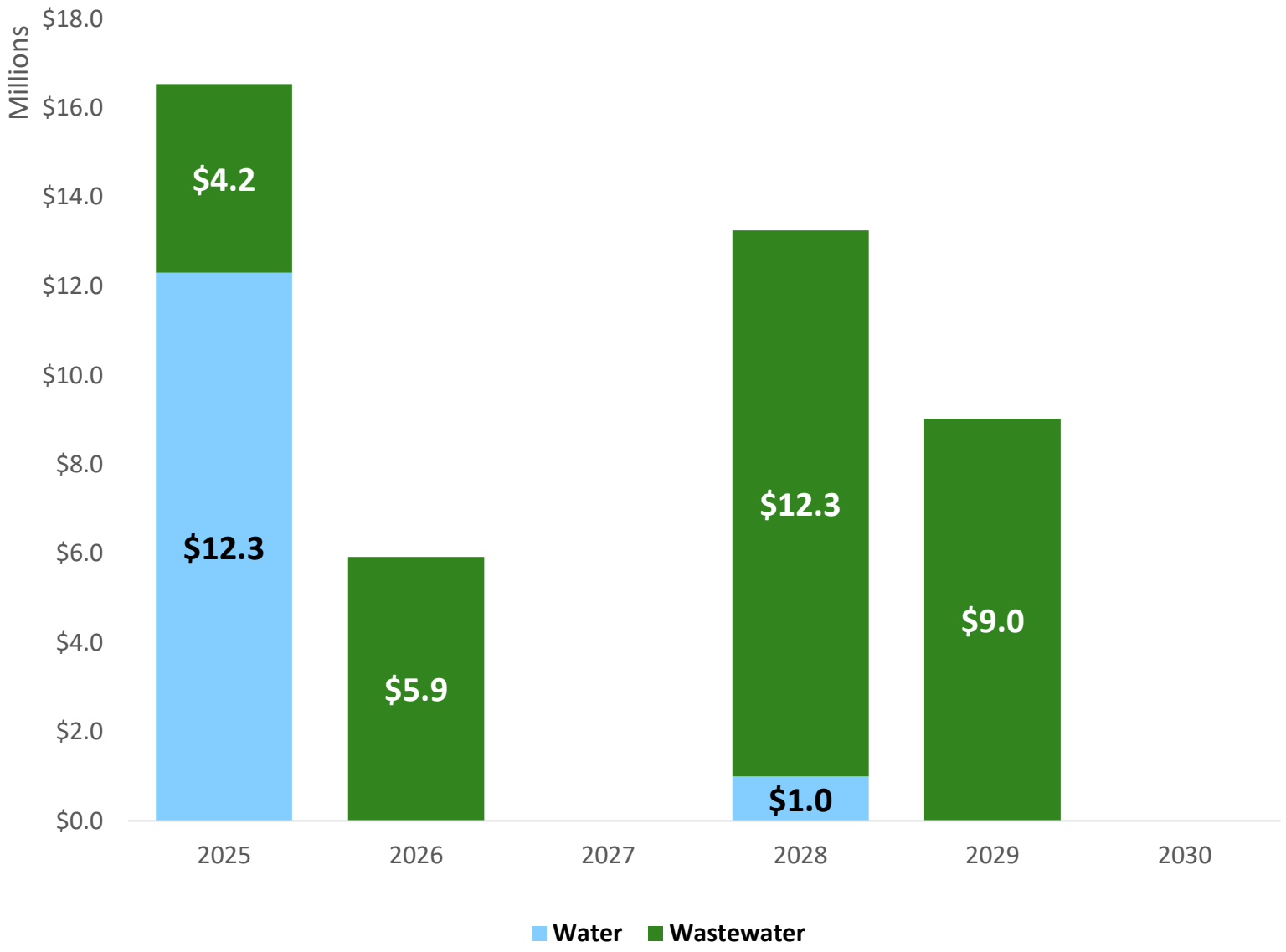
PROJECTED CAPITAL IMPROVEMENT PLAN (FY2025-2030)

WATER PROJECTS	
NEAR TERM (2025 -- 2029)	
1.5M Gallon Tank @ N. Reynolds / High School	$ 11,000,000
12" extension Boon Rd	1,300,000
System Transmission, Springhill, I-30 to Hwy 5N	1,000,000
Bryant Pkwy I-30 to Johnswood	740,000
Fireflow Improvemnet Project -- N. Reynolds Rd at Rogers Rd.Crossig	40,000
Woody Dr to Steeplechase Cir	80,000
Total	**14,160,000**

WASTEWATER PROJECTS	
NEAR TERM (2025 -- 2029)	
Disinfection / Contact Basin / Dissolved Oxygen Basin Improvements	$ 4,230,000
Headworks & Grit Removal Improvements	5,922,000
Activated Sludge Process Improvements	12,250,000
Clarification Improvements / WAS / RAS	9,024,000
BR-04 Lift Station LS-05 Upgrade	8,625,000
Lift Station LS-05 Parallel Force Main	7,618,000
Total	**47,669,000**
TOTAL CIP	**$ 61,829,000**



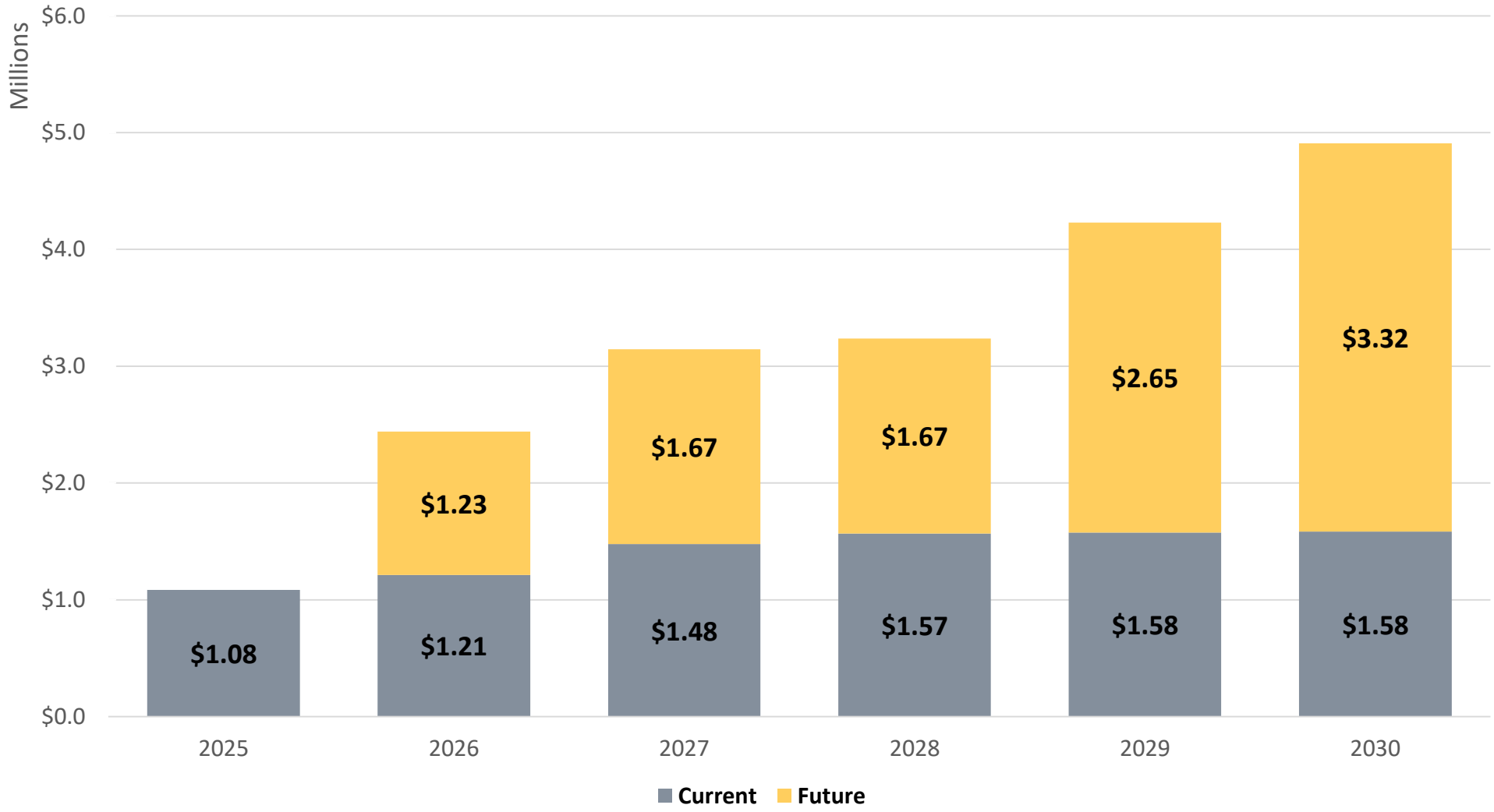
FORECAST DEBT ISSUES



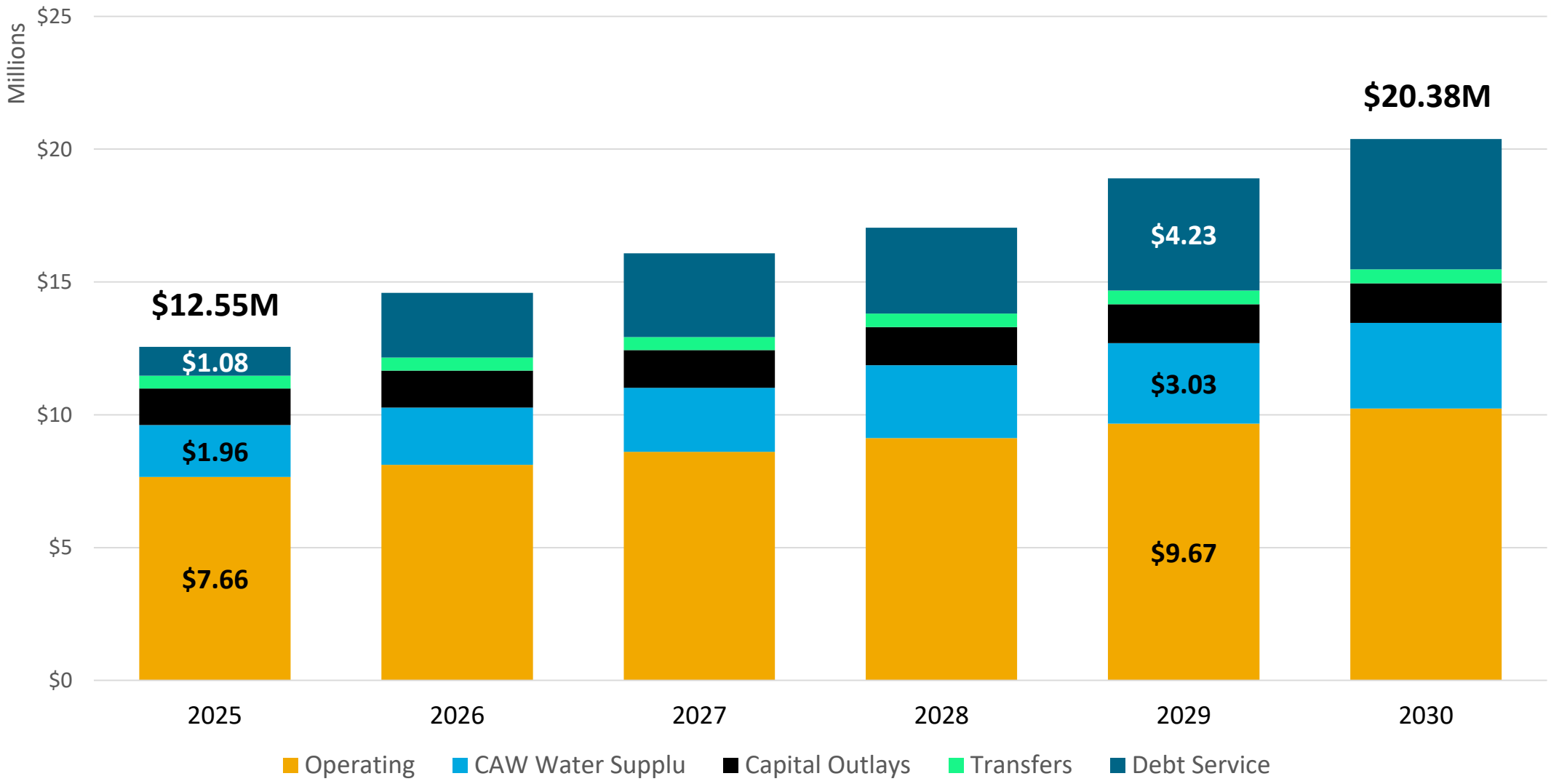
Water Debt: $13,300,000
WW Debt: $31,426,000

Total Debt: $44,726,000

FORECAST DEBT SERVICE PAYMENTS



FORECAST WATER & WASTEWATER COST OF SERVICE

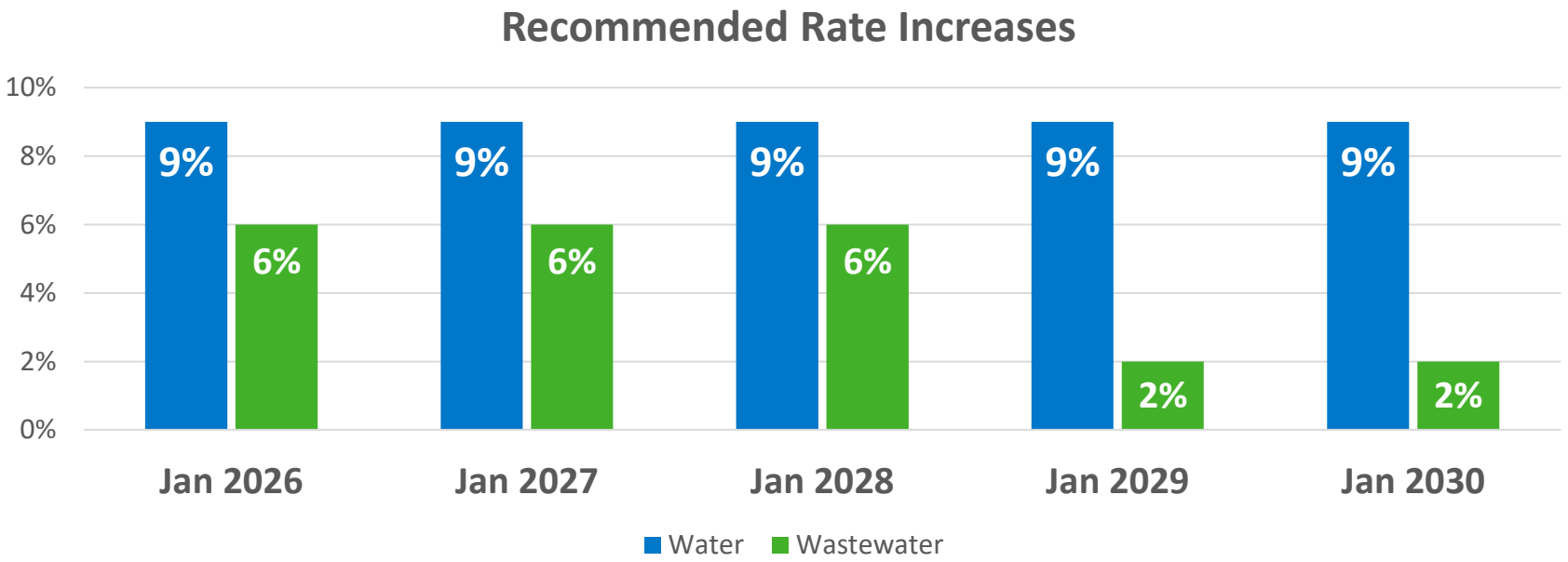


WATER & WASTEWATER
RATE PLAN

RATE PROPOSAL | OVERVIEW

Recommended rate adjustments for next 5 years:

- Annual Rate Adjustments in January of each year
- No change in the water and wastewater rate structure
- Uniform percentage adjustments for base volume charges
- Higher increases for water



The rate plan is designed to fund all known aspects of the water and wastewater utility based on the existing market conditions and assumptions.

RATE PROPOSAL | WATER

		Current	Effective Jan-26	Jan-27	Jan-28	Jan-29	Jan-30
WATER							
Residential and Non Residential							
Monthly Minimum Charge							
5/8"		$ 14.53	**$ 15.84**	$ 17.26	$ 18.82	$ 20.51	$ 22.36
1"		21.80	**23.76**	25.90	28.23	30.77	33.54
1 1/2"		36.33	**39.60**	43.16	47.05	51.28	55.90
2"		72.65	**79.19**	86.32	94.08	102.55	111.78
3"		116.24	**126.70**	138.10	150.53	164.08	178.85
4"		217.96	**237.58**	258.96	282.26	307.67	335.36
6"		726.53	**791.92**	863.19	940.88	1,025.56	1,117.86
Volume Rate Per 100 Gal							
2,000	Above	0.698	**0.761**	0.829	0.904	0.985	1.074

RATE PROPOSAL | WASTEWATER

		Current	Effective Jan-26	Jan-27	Jan-28	Jan-29	Jan-30
WASTEWATER							
Residential & Non-Residential Inside							
Monthly Minimum Charge		$ 20.70	**$ 21.94**	$ 23.26	$ 24.65	$ 25.15	$ 25.65
Volume Rate/100 Gal (2,000-Above)		1.104	**1.170**	1.240	1.315	1.341	1.368
Salem/Quail Ridge							
Monthly Minimum Charge		$ 41.40	**$ 43.88**	$ 46.52	$ 49.31	$ 50.29	$ 51.30
Volume Rate/100 Gal		1.104	**1.170**	1.240	1.315	1.341	1.368
Drain Water (County Landfield)							
Volume Rate/per Gallon		0.0575	**0.0610**	0.0646	0.0685	0.0699	0.0713
WW Infrastructure Monthly Charge							
(in addition to mthly chgs)	**5/8"**	**$ 15.75**	**$ 10.00**	$ 10.50	$ 11.03	$ 11.58	$ 12.16
	1"	52.50	**55.13**	57.88	60.78	63.81	67.00
	1 1/2"	105.00	**110.25**	115.76	121.55	127.63	134.01
	2"	168.00	**176.40**	185.22	194.48	204.21	214.42
	3"	336.00	**352.80**	370.44	388.96	408.41	428.83
	4"	530.25	**556.76**	584.60	613.83	644.52	676.75
	6"	1,060.50	**1,113.53**	1,169.20	1,227.66	1,289.04	1,353.50
	8"	1,060.50	**1,113.53**	1,169.20	1,227.66	1,289.04	1,353.50

IMPACT OF RATE PLAN ON COMBINED MONTHLY CHARGES

		Current	Effective Jan-26	Jan-27	Jan-28	Jan-29	Jan-30
Residential Monthly Charges -- 5/8"							
2,000 Water	2,000 WW	$ 50.98	**$ 47.78**	$ 51.02	$ 54.50	$ 57.23	$ 60.16
	Increase -- $		**(3.20)**	3.24	3.47	2.74	2.93
	Increase -- %		**-6.3%**	6.8%	6.8%	5.0%	5.1%
5,000 Water	5,000 WW	105.04	**105.71**	113.11	121.06	127.03	133.42
	Increase -- $		**0.67**	7.40	7.95	5.97	6.39
	Increase -- %		**0.6%**	7.0%	7.0%	4.9%	5.0%
10,000 Water	10,000 WW	195.14	**202.26**	216.60	232.00	243.35	255.52
	Increase -- $		**7.12**	14.34	15.40	11.35	12.17
	Increase -- %		**3.7%**	7.1%	7.1%	4.9%	5.0%
Commercial Monthly Charges -- 2"							
25,000 Water	25,000 WW	$ 675.81	**$ 721.67**	$ 770.84	$ 823.55	$ 866.99	$ 913.50
	Increase -- $		**45.86**	49.16	52.71	43.44	46.51
	Increase -- %		**6.8%**	6.8%	6.8%	5.3%	5.4%

SUMMARY

BENEFITS OF THE PROPOSED RATE PLAN

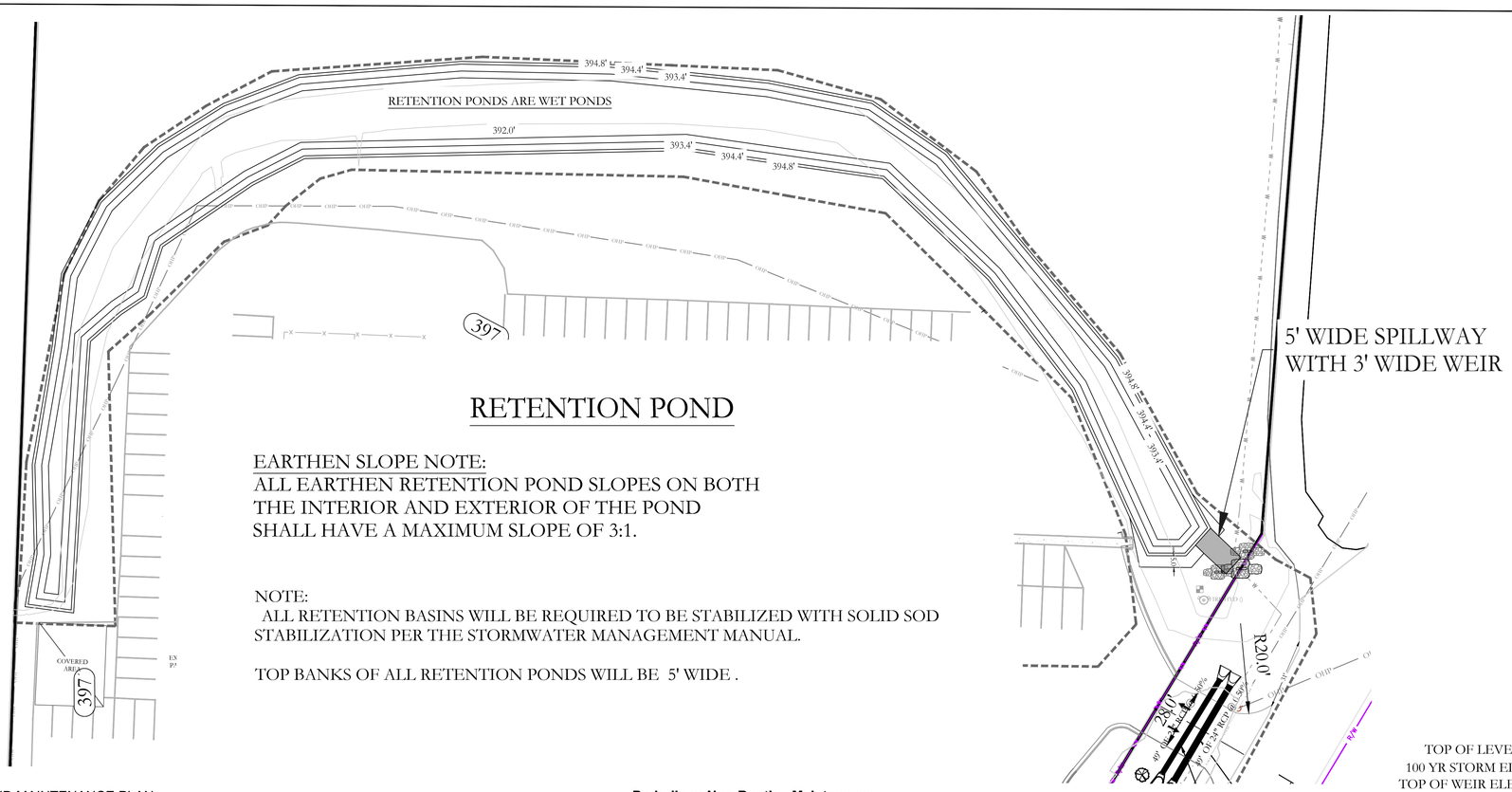
- The proposed rate plan enables the City to fully fund all water and wastewater costs and CIP over the next five years
- Will enable the Utility Fund to operate self-sufficiently with no need for subsidies from fund balance
- Enables the City to provide safe drinking water and effectively treat wastewater continually for forecast period

QUESTIONS & DISCUSSION

Disclosure

Willdan Financial Services ("Willdan") is registered as a "municipal advisor" pursuant to Section 15B of the Securities Exchange Act and rules and regulations adopted by the United States Securities and Exchange Commission ("SEC") and the Municipal Securities Rulemaking Board ("MSRB"). The MSRB has made available on its website (www.msrb.org) a municipal advisory client brochure that describes the protections that may be provided by MSRB rules and how to file a complaint with the appropriate regulatory authority. As part of its SEC registration Willdan is required to disclose to the SEC information regarding criminal actions, regulatory actions, investigations, terminations, judgments, liens, civil judicial actions, customer complaints, arbitrations and civil litigation involving Willdan. Pursuant to MSRB Rule G-42, Willdan is required to disclose any legal or disciplinary event that is material to Client's evaluation of Willdan or the integrity of its management or advisory personnel. Willdan has determined that no such event exists. Copies of Willdan's filings with the United States Securities and Exchange Commission can currently be found by accessing the SEC's EDGAR system Company Search Page which is currently available at: https://www.sec.gov/edgar/searchedgar/companysearch.html and searching for either Willdan Financial Services or for our CIK number which is 0001782739.

For the avoidance of doubt and without limiting the foregoing, in connection with any revenue projections, cash-flow analyses, feasibility studies and/or other analyses Willdan may provide the municipality with respect to financial, economic or other matters relating to a prospective, new or existing issuance of municipal securities of the municipality, (A) any such projections, studies and analyses shall be based upon assumptions, opinions or views (including, without limitation, any assumptions related to revenue growth) established by the municipality, in conjunction with such of its municipal, financial, legal and other advisers as it deems appropriate; and (B) under no circumstances shall Willdan be asked to provide, nor shall it provide, any advice or recommendations or subjective assumptions, opinions or views with respect to the actual or proposed structure, terms, timing, pricing or other similar matters with respect to any municipal financial products or municipal securities issuances, unless formally engaged to provide such information.



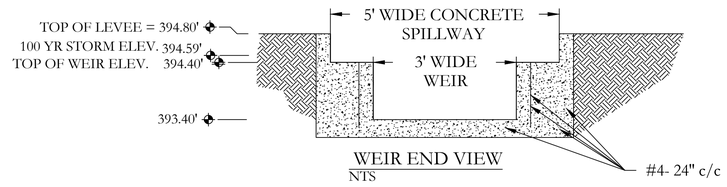
RETENTION POND

EARTHEN SLOPE NOTE:
ALL EARTHEN RETENTION POND SLOPES ON BOTH THE INTERIOR AND EXTERIOR OF THE POND SHALL HAVE A MAXIMUM SLOPE OF 3:1.

NOTE:
ALL RETENTION BASINS WILL BE REQUIRED TO BE STABILIZED WITH SOLID SOD STABILIZATION PER THE STORMWATER MANAGEMENT MANUAL.

TOP BANKS OF ALL RETENTION PONDS WILL BE 5' WIDE .

VICINITY MAP:



WEIR END VIEW
NTS

DETENTION POND MAINTENANCE PLAN

Background
The Retention pond is located on the north-east of the property.It is designed to temporarily detain storm water to meet water quantity criteria before discharging off the property.

Routine Maintenance:
The property owner will maintain the drainage easements . Routine maintenance will include but not be limited to:
-Mowing of the bank slopes and area around the pond on a monthly basis during the growing season and as needed during the cooler months.

-The outlet pipe from the pond and other areas will be inspected monthly for debris which could inhibit the proper flow of discharge. Any debris will be removed immediately and disposed of or placed in a location to prevent future maintenance and to not cause impact up or downstream of the structure.

-Trash will be removed from around the pond to prevent entering the pond. Generally, the site should be kept free of loose trash which could be carried off site by wind or rain.

-Inspect the pond and outlet pipe for non-routine maintenance need.

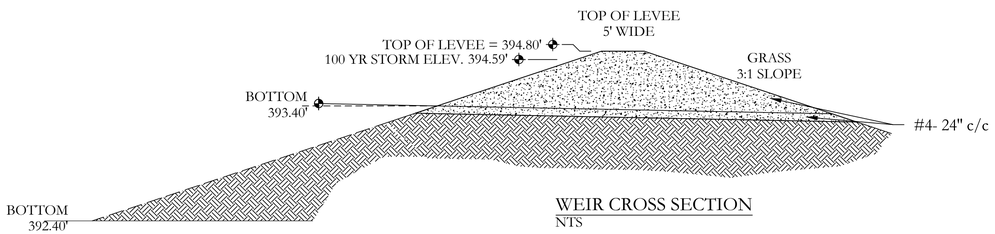
Periodic or Non-Routine Maintenance
The routine inspection of the ponds areas and discharge pipes will identify needed repairs and non-routine maintenance. These items may include but not be limited to:

-Re-growth of trees on or around the pond bank. These should be cut and removed from the pond area.

-Sediment from the site may accumulate in the pond bottom and reduce the pond to below design volume requirements. The pond should be excavated if the pond bottom elevation reached a level that allows excessive aquatic growth or reduces the pond efficiency such, that the sediments are passing the discharge structure and release off site.

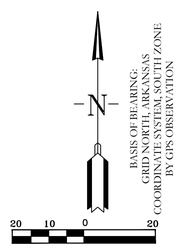
-Stabilization or re-grading of side slopes may be required periodically or after excessive rain events. Any disturbance of slopes should be reseeded or may require installation of erosion control materials until seeding can reestablish adequate grasses to prevent future erosion.

-Any other maintenance or repairs which would minimize other maintenance to the pond or outfall structures.



WEIR CROSS SECTION
NTS

CERTIFICATE OF AUTHORIZATION
HOPE CONSULTING, INC.
No. 1931
ARKANSAS



HOPE CONSULTING
ENGINEERS - SURVEYORS
129 N. Main Street,
Benton, Arkansas 72015
PH. (501)315-2626
FAX (501) 315-0024
www.hopeconsulting.com

FOR USE AND BENEFIT OF:
FIRST SOUTHERN BAPTIST CHURCH OF BRYANT

FSCB EXPANSION & REMODEL PHASE 1
RETENTION POND PLAN AND DETAILS
604 S REYNOLDS ROAD
BRYANT, SALINE COUNTY, ARKANSAS

DATE:	05-16-2025	C.A.D. BY:	DRAWING NUMBER:
REVISED:		CHECKED BY:	24-0260
SHEET:	C-6.0	SCALE:	

500	01S	14W	0	34	310	62	1664

K:\CLAND PROJECTS 2024\COMMERCIAL\2024-24-0260 FSCB EXPANSION AND REMODEL\CIVIL\DWG\24-0260 FSCB EXPANSION & REMODEL_8.5_2025.DWG

RETENTION POND

First Southern Baptist Church of Bryant

604 S REYNOLDS ROAD, BRYANT, AR 72022

DRAINAGE REPORT

FOR
City of Bryant, Saline County, AR

September 2024

Owner & Developer: Peter Cunningham.

By:

HOPE
CONSULTING
ENGINEERS - SURVEYORS

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ITEM DESCRIPTION

1. Narrative & Summary
2. Hydrograph Report

Narrative & Summary

PROJECT TITLE

First Southern Baptist Church of Bryant

PROJECT PROPERTY OWNER

Peter Cunningham

PROJECT LOCATION

604 S Reynolds Road, Bryant, AR

PROJECT DESCRIPTION

The proposed development is on South Reynolds Road, Bryant, AR. Total development site area is 7.58 acres.

DRAINAGE ANALYSIS

On Site Drainage- Rational method was used to determine the existing and proposed flows from proposed site. There will be one retention pond to detain water from this development. Detailed drainage calculations considering the future expected development have been conducted to determine the required detention pond and culvert dimensions. Summary of the calculations are below:

Retention Pond

- Pond is situated on the north-east side of the property.
- Pre-development area 7.36 acres.
- Post-development area 7.34 acres.
- Pre-development runoff cumulative coefficient 0.65.
- Post-development runoff cumulative coefficient 0.72
- Pond has a bottom area of 16,570 sqft with bottom elevation of 393.4'.
- A 5' wide spillway with a 3' wide weir outlet structure.

Peak flows for Pre and post development phase of onsite area have been tabulated below-

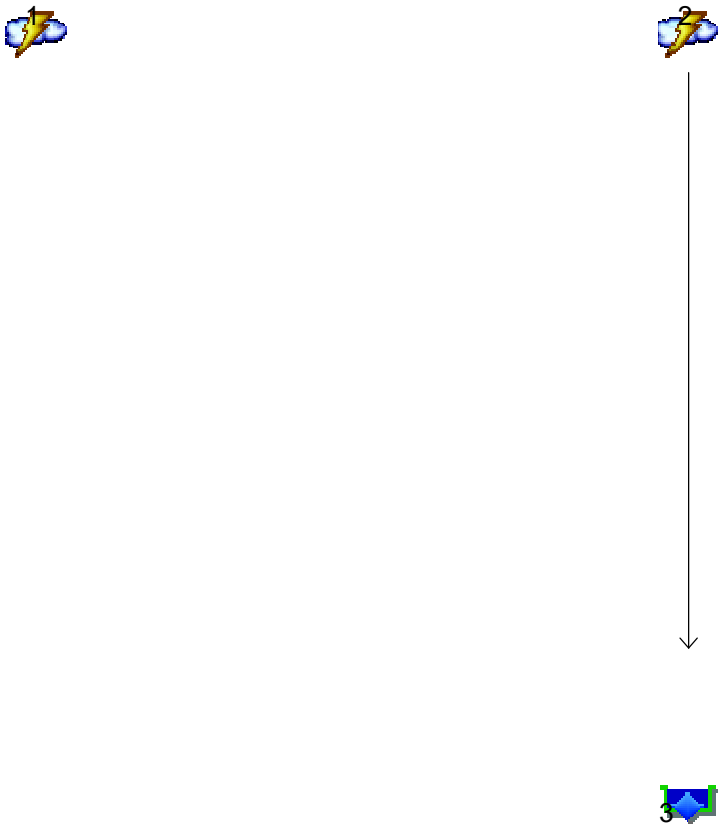
Period of time	Pre-development	Post-dev. Without detention	Post-dev. With detention
	Peak Flow (cfs)	Peak Flow (cfs)	Peak Flow (cfs)
2-Year	18.69	22.67	5.733
5-Year	20.65	25.15	6.587
10-Year	24.35	29.23	8.068
25-Year	27.93	33.44	9.693
50-Year	31.84	38.07	11.94
100-Year	33.86	40.40	13.17

CONCLUSION

From the onsite drainage calculation, it is seen that there is decrease in flow for all storm events due to the proposed retention pond.

Hydrograph Summary Report

Watershed Model Schematic



Legend

Hyd.	Origin	Description
1	Rational	Pre-Dev Flow
2	Rational	Development Generated Flow
3	Reservoir	Post Development Flow

Multi-Hydrograph Plot

Hyd. No. 1

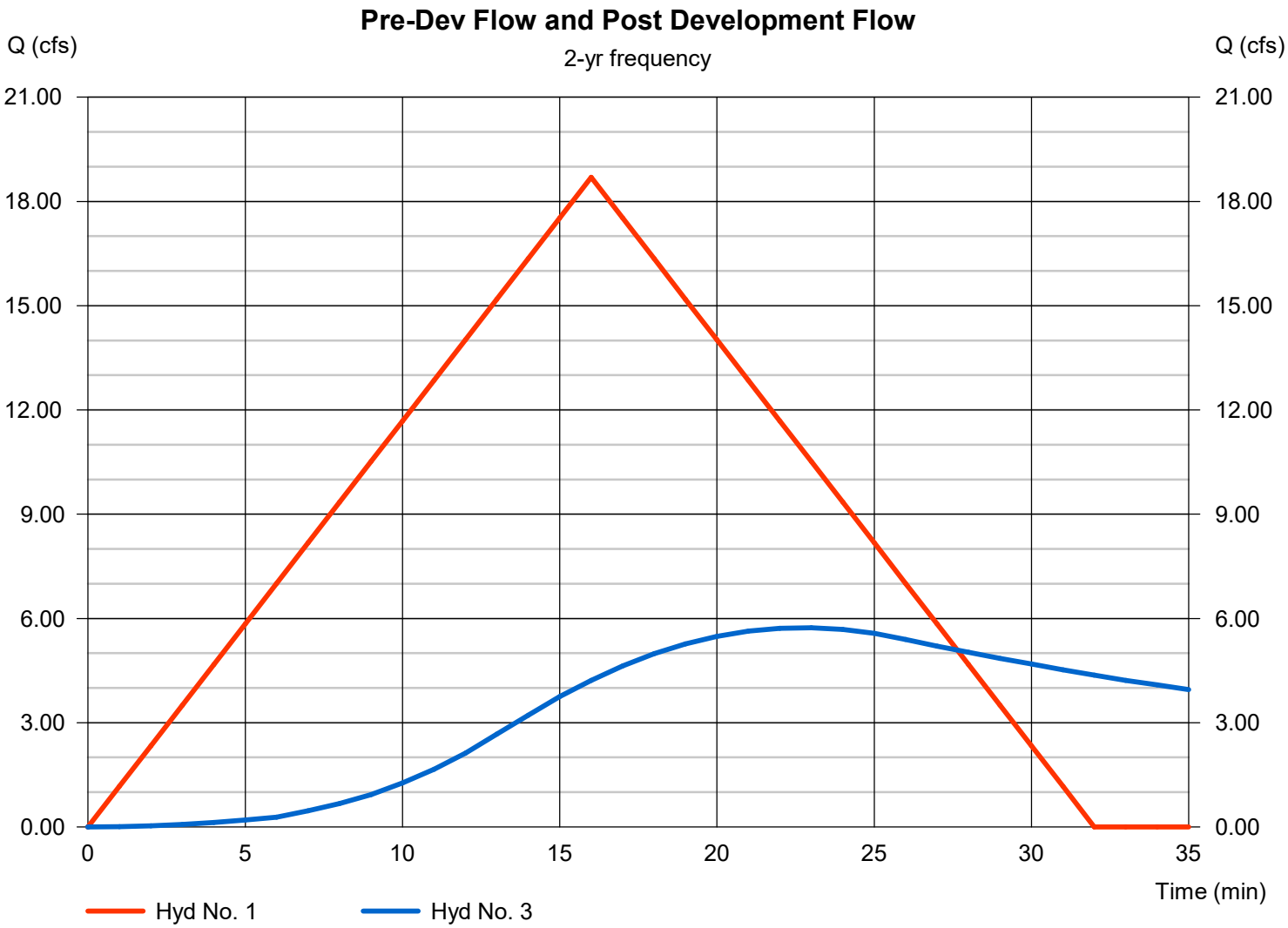
Pre-Dev Flow

Hydrograph type	=	Rational
Peak discharge	=	18.69 cfs
Time to peak	=	16 min
Hyd. Volume	=	17,943 cuft

Hyd. No. 3

Post Development Flow

Hydrograph type	=	Reservoir
Peak discharge	=	5.73 cfs
Time to peak	=	23 min
Hyd. Volume	=	17,672 cuft



Multi-Hydrograph Plot

Hyd. No. 1

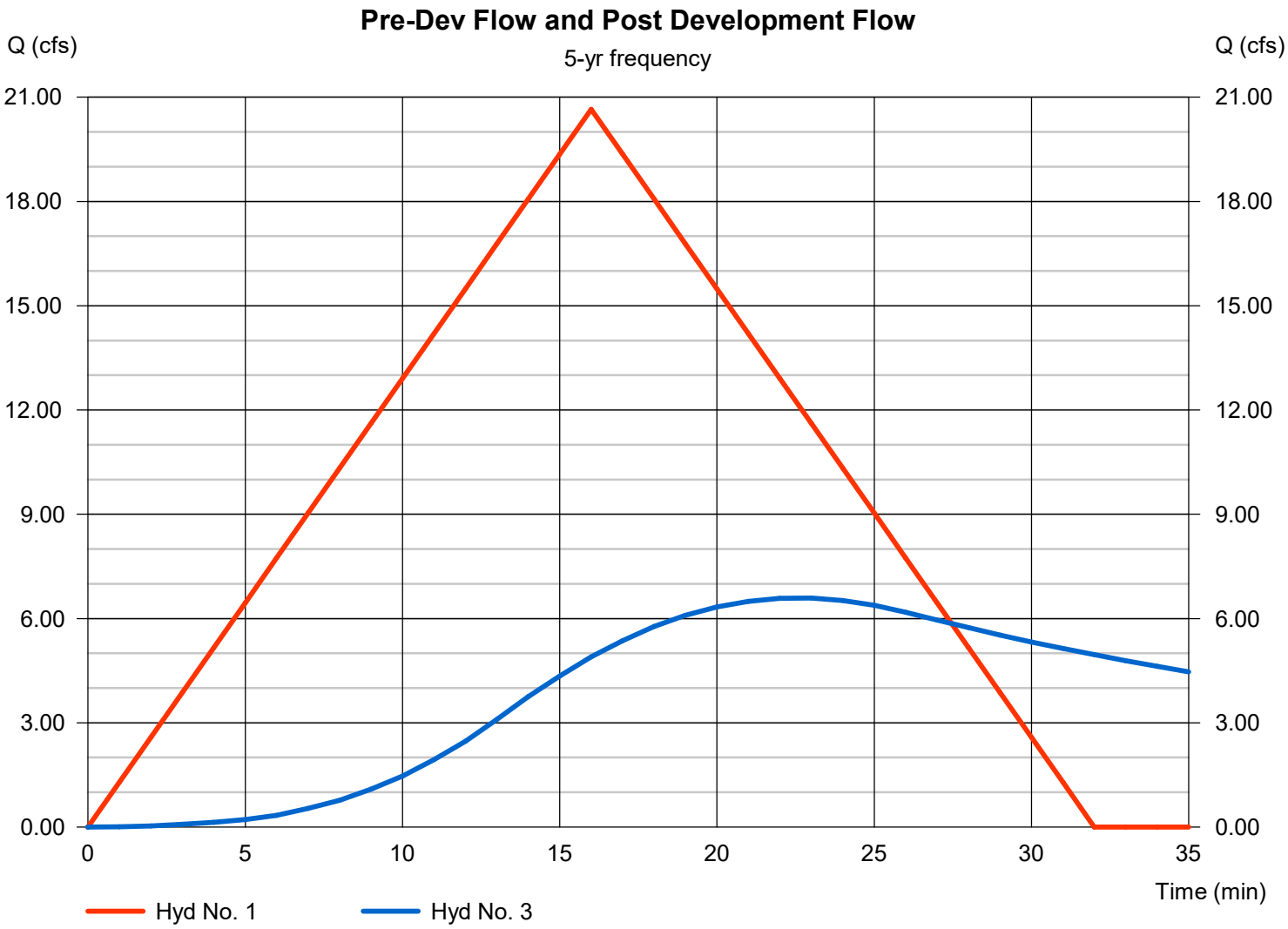
Pre-Dev Flow

Hydrograph type	=	Rational
Peak discharge	=	20.65 cfs
Time to peak	=	16 min
Hyd. Volume	=	19,826 cuft

Hyd. No. 3

Post Development Flow

Hydrograph type	=	Reservoir
Peak discharge	=	6.59 cfs
Time to peak	=	23 min
Hyd. Volume	=	19,608 cuft



Multi-Hydrograph Plot

Hyd. No. 1

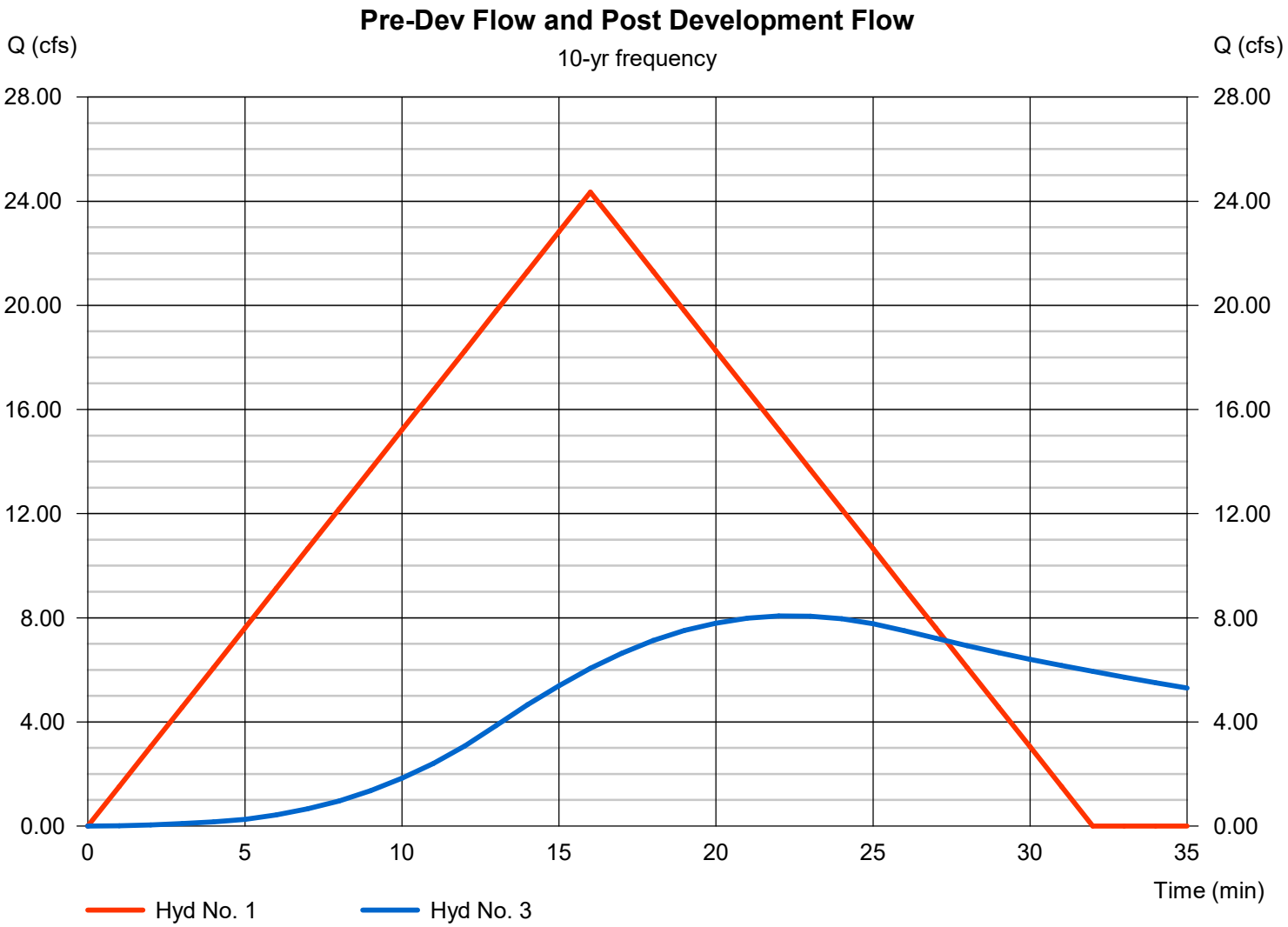
Pre-Dev Flow

Hydrograph type	=	Rational
Peak discharge	=	24.35 cfs
Time to peak	=	16 min
Hyd. Volume	=	23,373 cuft

Hyd. No. 3

Post Development Flow

Hydrograph type	=	Reservoir
Peak discharge	=	8.07 cfs
Time to peak	=	22 min
Hyd. Volume	=	22,791 cuft



Multi-Hydrograph Plot

Hyd. No. 1

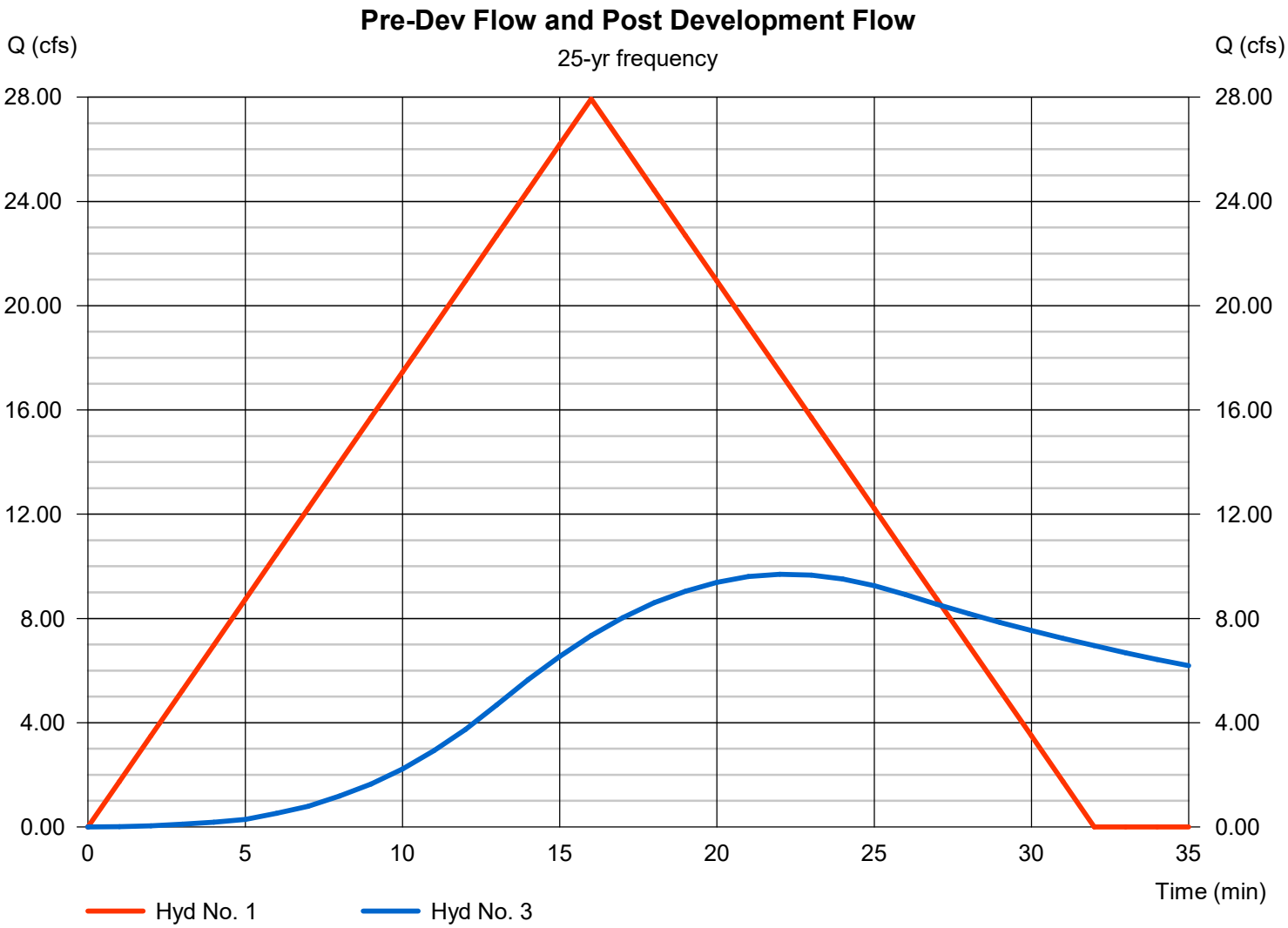
Pre-Dev Flow

Hydrograph type	=	Rational
Peak discharge	=	27.93 cfs
Time to peak	=	16 min
Hyd. Volume	=	26,812 cuft

Hyd. No. 3

Post Development Flow

Hydrograph type	=	Reservoir
Peak discharge	=	9.69 cfs
Time to peak	=	22 min
Hyd. Volume	=	26,080 cuft



Multi-Hydrograph Plot

Hyd. No. 1

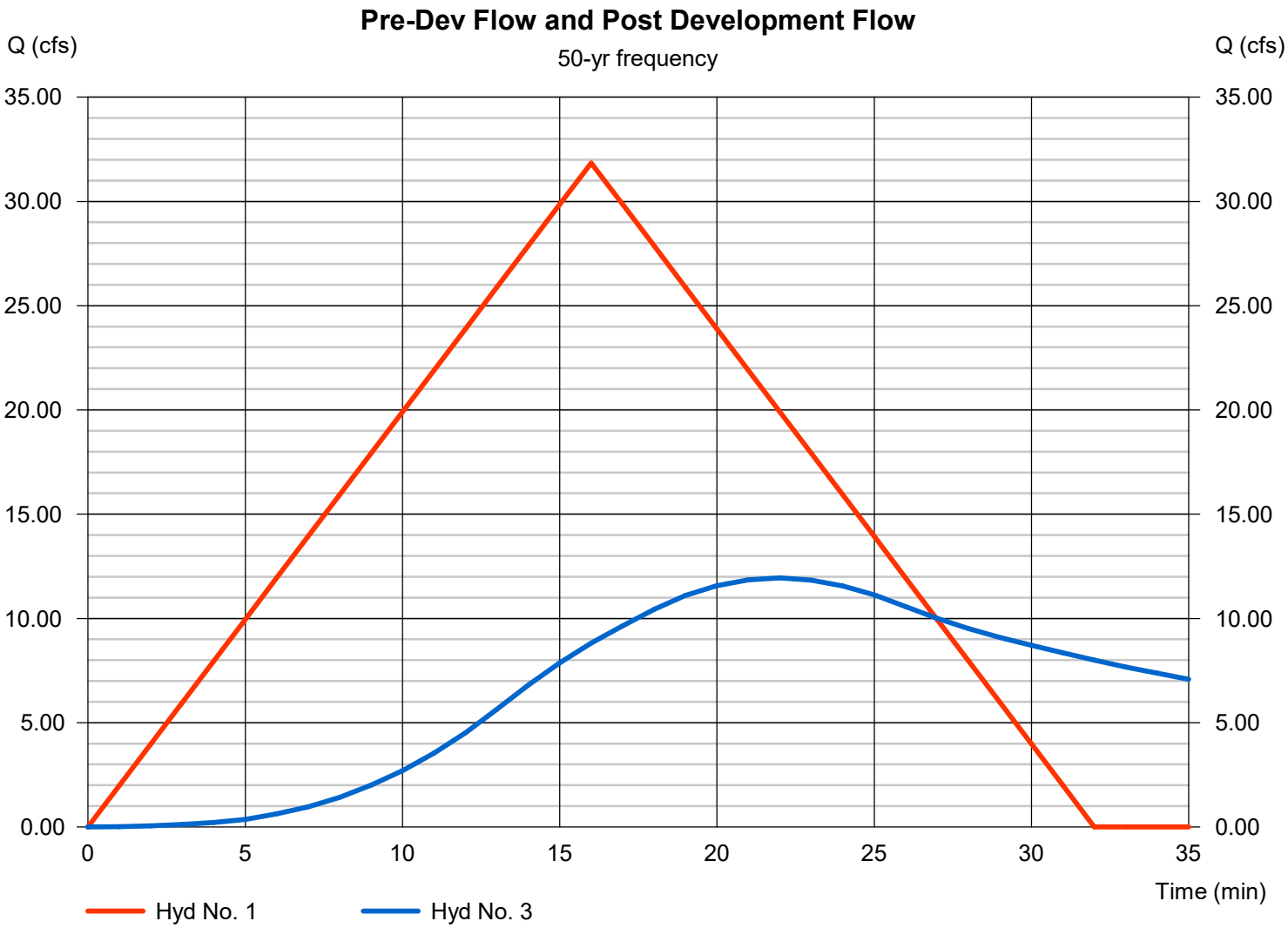
Pre-Dev Flow

Hydrograph type	=	Rational
Peak discharge	=	31.84 cfs
Time to peak	=	16 min
Hyd. Volume	=	30,570 cuft

Hyd. No. 3

Post Development Flow

Hydrograph type	=	Reservoir
Peak discharge	=	11.94 cfs
Time to peak	=	22 min
Hyd. Volume	=	29,692 cuft



Multi-Hydrograph Plot

Hyd. No. 1

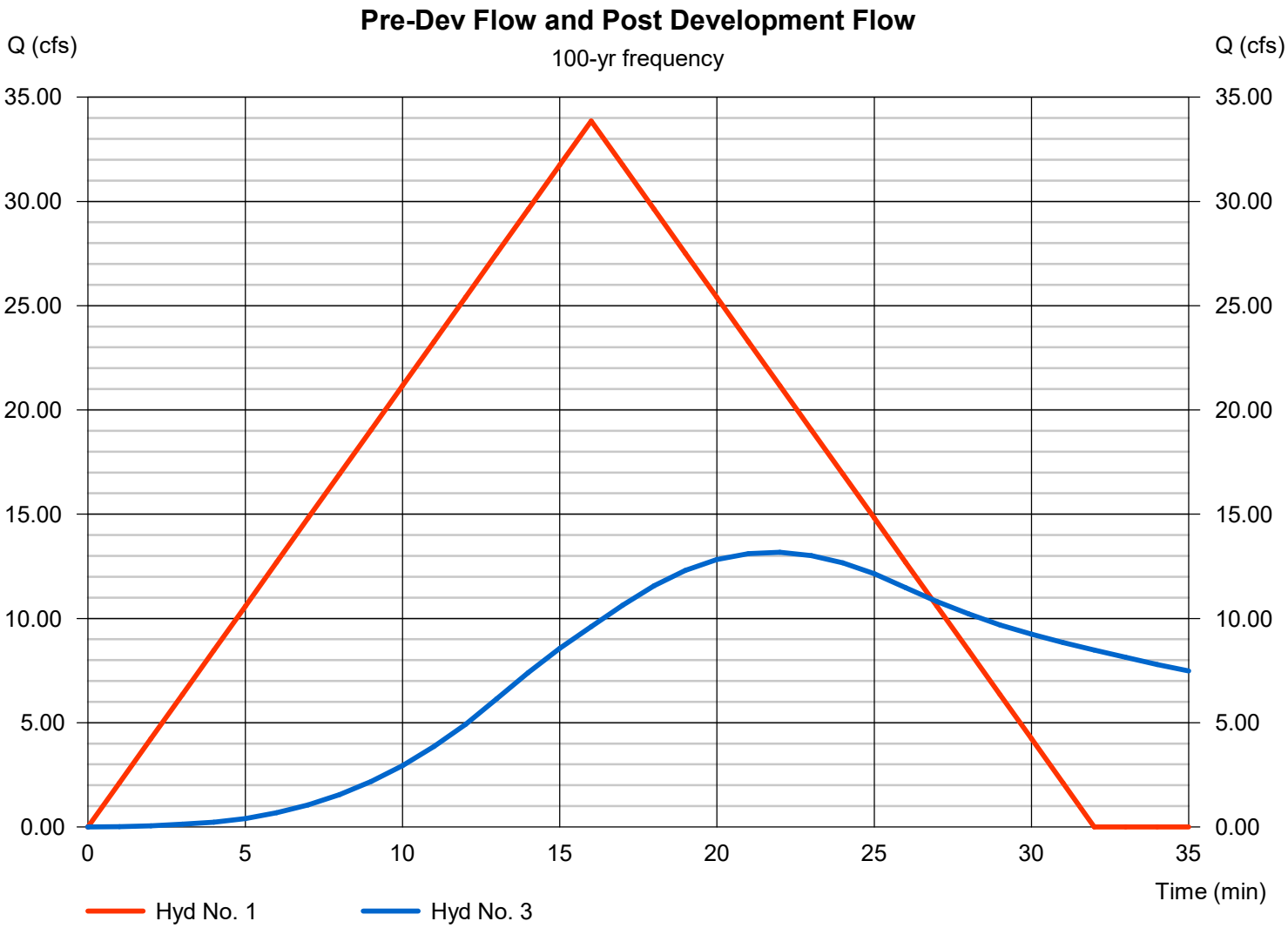
Pre-Dev Flow

Hydrograph type	=	Rational
Peak discharge	=	33.86 cfs
Time to peak	=	16 min
Hyd. Volume	=	32,504 cuft

Hyd. No. 3

Post Development Flow

Hydrograph type	=	Reservoir
Peak discharge	=	13.17 cfs
Time to peak	=	22 min
Hyd. Volume	=	31,502 cuft



Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026 Thursday, 08 / 7 / 2025

Pond No. 1 - Retention Pond

Pond Data

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 393.40 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	393.40	16,570	0	0
1.00	394.40	21,182	18,827	18,827
1.40	394.80	23,045	8,842	27,669

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	Inactive	Inactive	Inactive	Inactive
Span (in)	= 8.00	8.00	0.00	0.00
No. Barrels	= 1	1	0	0
Invert El. (ft)	= 393.40	393.40	0.00	0.00
Length (ft)	= 25.00	25.00	0.00	0.00
Slope (%)	= 0.52	0.52	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

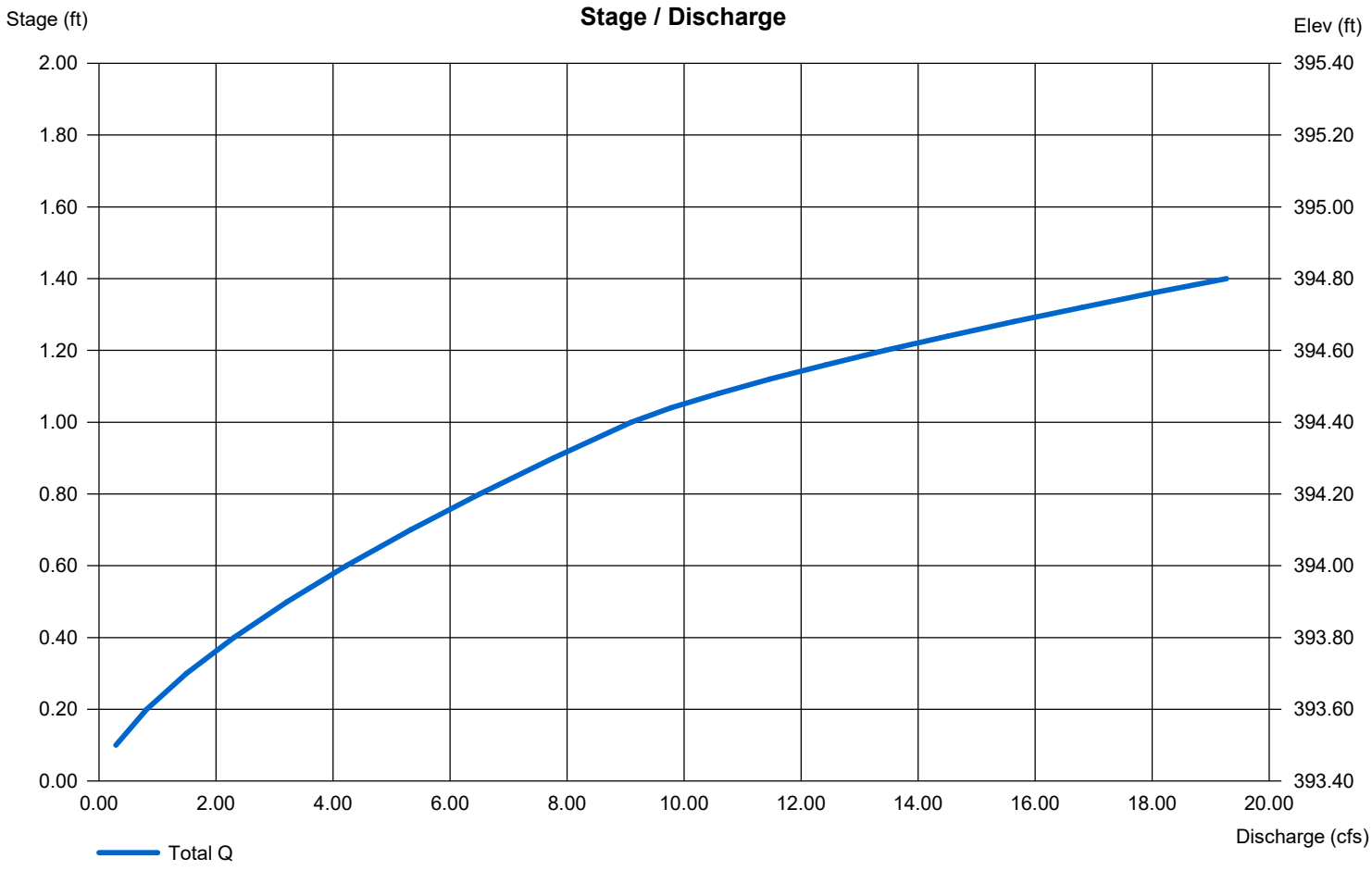
Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 3.00	5.00	0.00	0.00
Crest El. (ft)	= 393.40	394.40	0.00	0.00
Weir Coeff.	= 3.03	3.33	3.33	3.33
Weir Type	= Rect	Rect	---	---
Multi-Stage	= No	No	No	No

Exfil.(in/hr) = 0.000 (by Contour)

TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	18.69	1	16	17,943	------	------	------	Pre-Dev Flow
2	Rational	22.67	1	13	17,679	------	------	------	Development Generated Flow
3	Reservoir	5.733	1	23	17,672	2	394.13	13,831	Post Development Flow

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	20.65	1	16	19,826	------	------	------	Pre-Dev Flow
2	Rational	25.15	1	13	19,614	------	------	------	Development Generated Flow
3	Reservoir	6.587	1	23	19,608	2	394.21	15,185	Post Development Flow

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	24.35	1	16	23,373	------	------	------	Pre-Dev Flow
2	Rational	29.23	1	13	22,797	------	------	------	Development Generated Flow
3	Reservoir	8.068	1	22	22,791	2	394.32	17,379	Post Development Flow

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	27.93	1	16	26,812	------	------	------	Pre-Dev Flow
2	Rational	33.44	1	13	26,086	------	------	------	Development Generated Flow
3	Reservoir	9.693	1	22	26,080	2	394.44	19,606	Post Development Flow

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	31.84	1	16	30,570	------	------	------	Pre-Dev Flow
2	Rational	38.07	1	13	29,698	------	------	------	Development Generated Flow
3	Reservoir	11.94	1	22	29,692	2	394.54	21,917	Post Development Flow

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2026

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	33.86	1	16	32,504	------	------	------	Pre-Dev Flow
2	Rational	40.40	1	13	31,509	------	------	------	Development Generated Flow
3	Reservoir	13.17	1	22	31,502	2	394.59	23,012	Post Development Flow

Drainage Report

For

Jamey South Parking Lot

Bryant, Saline County, Arkansas

August 14, 2025

Prepared by:

RICHARDSON ENGINEERING, PLLC

325 W. South St.
Benton, AR 72015
501-315-7225

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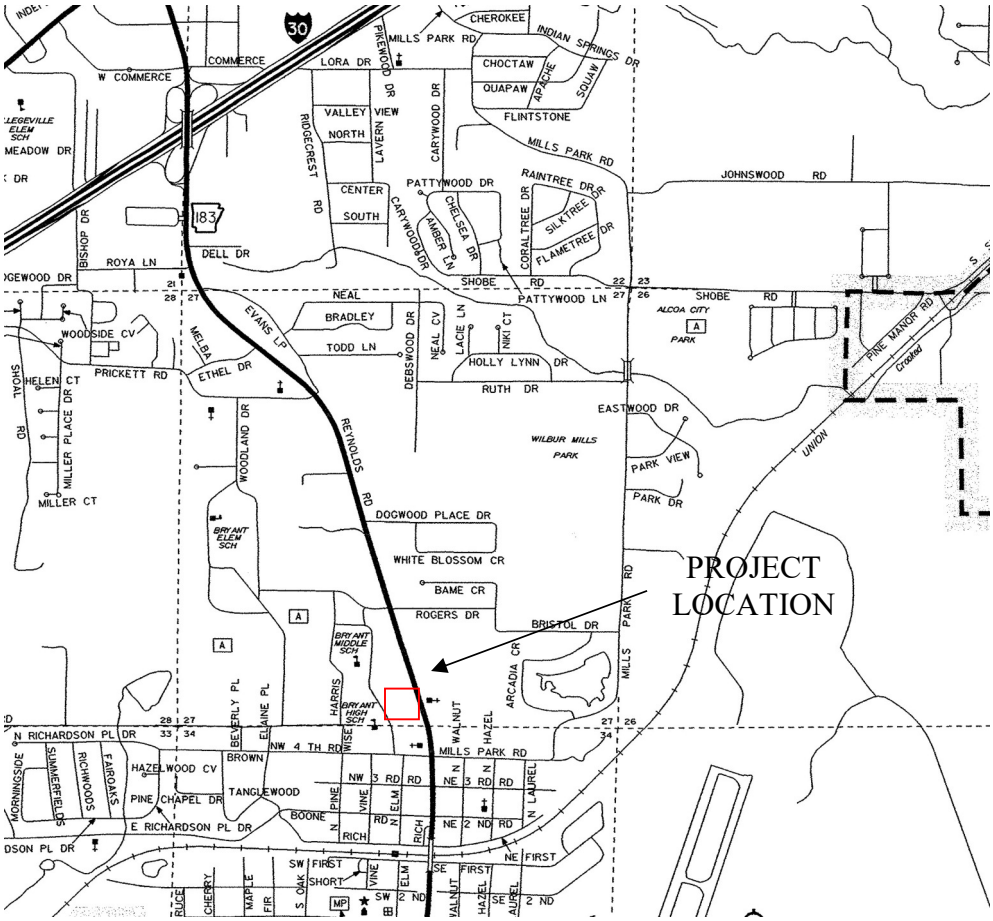
Project Owner Information

Jamey South
515 N Reynolds Road
Bryant, AR 72022

Project Location and Description

The project is located on West side of N Reynolds Road, part of the Southwest Quater of the Southeast Quarter, Section 27, Township 1-S, Range 14-W, Saline County, Arkansas.



Vicinity Map – N.T.S

This project is a proposed Commercial Development for a parking lot, located in the City of Bryant, Saline County.

Site Drainage

Pre-Development

The pre-developed runoff for the site flows to the west. The pre-development runoff condition consists of a mix of a small commercial development as well as a portion of undeveloped wooded property.

Post-Development

The site drainage starts on the East side of the project and flows to the West. The drainage is sheet flows across the proposed driving surface. The storm water is discharged off the parking lot to the west and sheet flows across a grass buffer/filter area. The post-development runoff conditions changed from developed/undeveloped to commercial development.

Runoff Summary

Basin Design Point
Development Drainage Study Area = 0.56 Ac
Existing Condition runoff Coefficient: C = 0.61
Proposed runoff Coefficient: C = 0.75
Tc Undeveloped = 9 Minutes (TR55 Method)
Tc Developed = 8 Minutes (TR55 Method)
Detention Basin Required Volume: 586 CF

Design Storm	Pre-Development Flow Rate (cfs)	Post-Development Flow Rate (cfs)
2-yr	1.61	2.09
10-yr	2.15	2.79
25-yr	2.47	3.21
50-yr	2.71	3.51
100-yr	2.94	3.81

Recommendations/Summary

The proposed drainage improvements include a grass buffer/filter area on the West side of the project. The proposed development slightly increases the peak flow leaving the site, but it is not considered to be significant.

Appendices

Runoff Coefficient Calculations
Site Drainage Map
Pond and Post Development Hydrographs (Hydrology Studio)

Runoff Coefficient Calculations

RICHARDSON ENGINEERING
Planning ▪ Engineering ▪ Development Consulting

325 West South Street
Benton, AR 72015
(501) 315-7225

(1/1)

PROJECT 025-007 DRAINAGE CALCULATIONS DATE 08/13/2025

EXISTING C:

DEVELOPED: 0.16 C = 0.95

UNDEVELOPED: 0.40 C = 0.47 (AVG 2-7% FOREST/WOODS)

$$C = \frac{(0.16)(0.95) + (0.40)(0.47)}{0.56} = 0.61$$

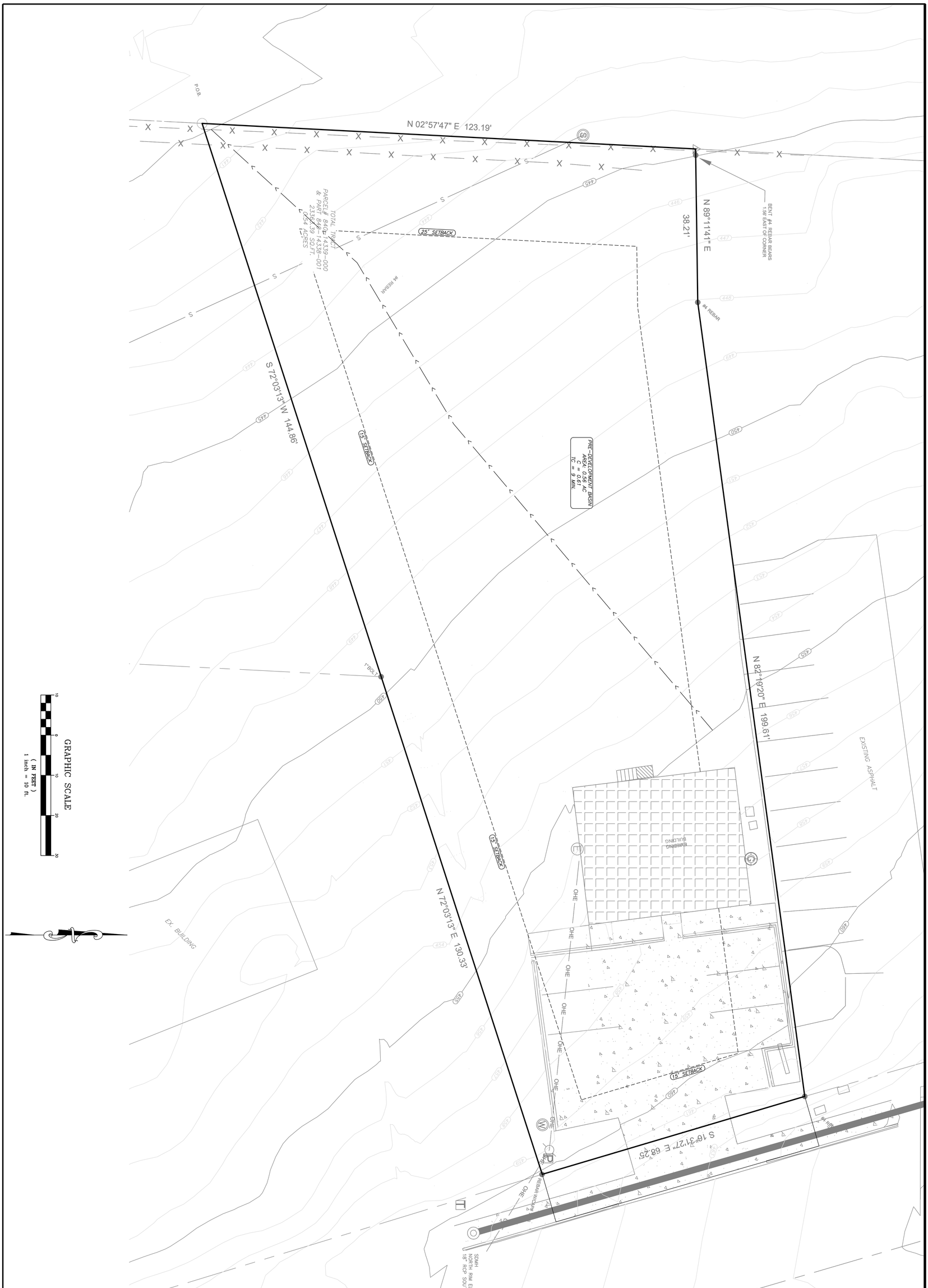
POST-DEV:

DEVELOPED: 0.33 C = 0.95

UNDEVELOPED: 0.23 C = 0.45 (AVG 2-7% LAWNS, SANDY SOIL)

$$C = \frac{(0.33)(0.95) + (0.23)(0.45)}{0.56} = 0.75$$

Site Drainage Basin Maps



PROJECT #: 025-007	No.	Revisions	Date	Prepared For: JAMEY SOUTH	PRE-DEVELOPMENT DRAINAGE BASIN STATE FARM – JAMEY SOUTH PROPOSED PARKING LOT 515 N. REYNOLDS ROAD BRYANT, ARKANSAS	CERTIFICATE OF AUTHORIZATION Richardson Engineering, PLLC No. 2519 ARKANSAS	RICHARDSON ENGINEERING Planning • Engineering • Development Consulting 325 W. SOUTH STREET, BENTON, AR 72015 (501)315-7225
Scale: 1" = 10'				515 N. REYNOLDS ROAD			
Date: 8/14/2025				BRYANT, AR 72022			
Sheet: 1 of 2							

PROJECT #: 025-007	No.	Revisions	Date	Prepared For: JAMEY SOUTH 515 N. REYNOLDS ROAD BRYANT, AR 72022	POST-DEVELOPMENT DRAINAGE BASIN STATE FARM – JAMEY SOUTH PROPOSED PARKING LOT 515 N. REYNOLDS ROAD BRYANT, ARKANSAS	CERTIFICATE OF AUTHORIZATION Richardson Engineering, PLLC No. 2519 ARKANSAS	RICHARDSON ENGINEERING Planning • Engineering • Development Consulting 325 W. SOUTH STREET, BENTON, AR 72015 (501)315-7225
Scale: 1" = 10' Date: 8/14/2025							
Sheet: 2 of 2							

Pre and Post Development Hydrographs (Hydrology Studio)

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Basin Model

Hydrology Studio v 3.0.0.27

Project Name: Jamey South Parking Lot

08-14-2025

Hydrograph by Return Period

Project Name: Jamey South Parking Lot

08-14-2025

Hydrology Studio v 3.0.0.27

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Outflow (cfs)							
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
1	Rational	Pre-Dev Basin		1.608			2.154	2.474	2.705	2.938
2	Rational	Post-Dev Basin		2.085			2.792	3.207	3.506	3.809

Hydrograph 2-yr Summary

Project Name: Jamey South Parking Lot

Hydrology Studio v 3.0.0.27

08-14-2025

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	Rational	Pre-Dev Basin	1.608	0.15	1,159	----		
2	Rational	Post-Dev Basin	2.085	0.13	1,336	----		

Hydrograph Report

Project Name: Jamey South Parking Lot

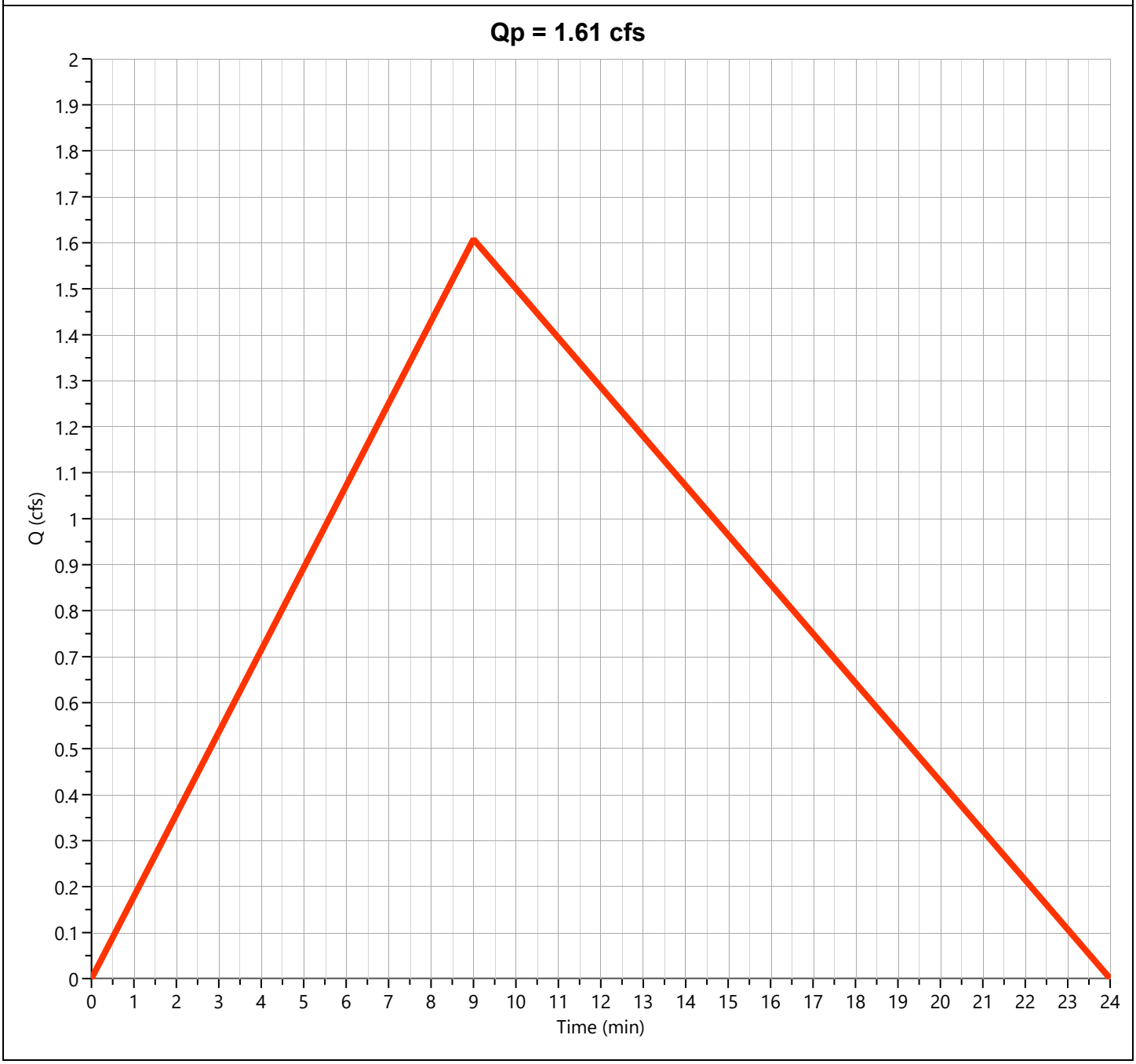
Hydrology Studio v 3.0.0.27

08-14-2025

Pre-Dev Basin

Hyd. No. 1

Hydrograph Type	= Rational	Peak Flow	= 1.608 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.15 hrs
Time Interval	= 1 min	Runoff Volume	= 1,159 cuft
Drainage Area	= 0.56 ac	Runoff Coeff.	= 0.61
Tc Method	= TR55 (See Worksheet)	Time of Conc. (Tc)	= 9.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 4.71 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	= 1/1.67



Tc by TR55 Worksheet

Project Name: Jamey South Parking Lot

Hydrology Studio v 3.0.0.27

08-14-2025

Pre-Dev Basin
Rational

Hyd. No. 1

Description	Segments			
	A	B	C	Tc (min)
Sheet Flow				
Description				
Manning's n	0.300	0.013	0.013	
Flow Length (ft)	100			
2-yr, 24-hr Precip. (in)	4.36	2.28	2.28	
Land Slope (%)	7.7			
Travel Time (min)	**8.52**	**0.00**	**0.00**	**8.52**
Shallow Concentrated Flow				
Flow Length (ft)	95			
Watercourse Slope (%)	7.60	0.00	0.00	
Surface Description	Unpaved	Paved	Paved	
Average Velocity (ft/s)	4.45			
Travel Time (min)	**0.36**	**0.00**	**0.00**	**0.36**
Channel Flow				
X-sectional Flow Area (sqft)				
Wetted Perimeter (ft)				
Channel Slope (%)				
Manning's n	0.013	0.013	0.013	
Velocity (ft/s)				
Flow Length (ft)				
Travel Time (min)	**0.00**	**0.00**	**0.00**	**0.00**
Total Travel Time				**9 min**

Hydrograph Report

Project Name: Jamey South Parking Lot

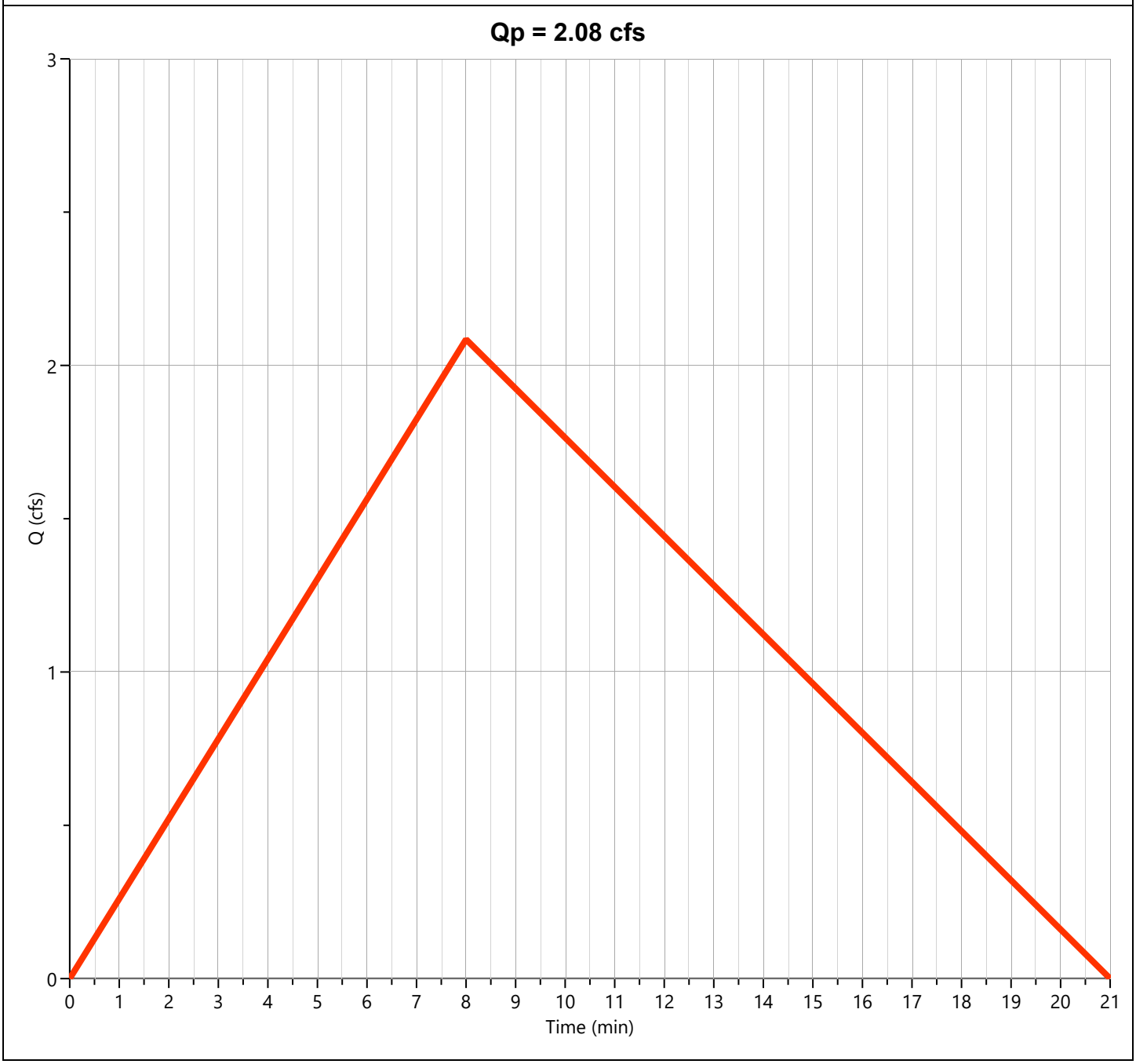
Hydrology Studio v 3.0.0.27

08-14-2025

Post-Dev Basin

Hyd. No. 2

Hydrograph Type	= Rational	Peak Flow	= 2.085 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.13 hrs
Time Interval	= 1 min	Runoff Volume	= 1,336 cuft
Drainage Area	= 0.56 ac	Runoff Coeff.	= 0.75
Tc Method	= TR55 (See Worksheet)	Time of Conc. (Tc)	= 8.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 4.96 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	= 1/1.67



Tc by TR55 Worksheet

Project Name: Jamey South Parking Lot

Hydrology Studio v 3.0.0.27

08-14-2025

Post-Dev Basin
Rational

Hyd. No. 2

Description	Segments			
	A	B	C	Tc (min)
Sheet Flow				
Description				
Manning's n	0.410	0.013	0.013	
Flow Length (ft)	72			
2-yr, 24-hr Precip. (in)	4.36	2.28	2.28	
Land Slope (%)	8.8			
Travel Time (min)	**7.98**	**0.00**	**0.00**	**7.98**
Shallow Concentrated Flow				
Flow Length (ft)				
Watercourse Slope (%)	0.00	0.00	0.00	
Surface Description	Paved	Paved	Paved	
Average Velocity (ft/s)				
Travel Time (min)	**0.00**	**0.00**	**0.00**	**0.00**
Channel Flow				
X-sectional Flow Area (sqft)				
Wetted Perimeter (ft)				
Channel Slope (%)				
Manning's n	0.013	0.013	0.013	
Velocity (ft/s)				
Flow Length (ft)				
Travel Time (min)	**0.00**	**0.00**	**0.00**	**0.00**
Total Travel Time				**8 min**

Hydrograph 10-yr Summary

Project Name: Jamey South Parking Lot

08-14-2025

Hydrology Studio v 3.0.0.27

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	Rational	Pre-Dev Basin	2.154	0.15	1,552	----		
2	Rational	Post-Dev Basin	2.792	0.13	1,789	----		

Hydrograph Report

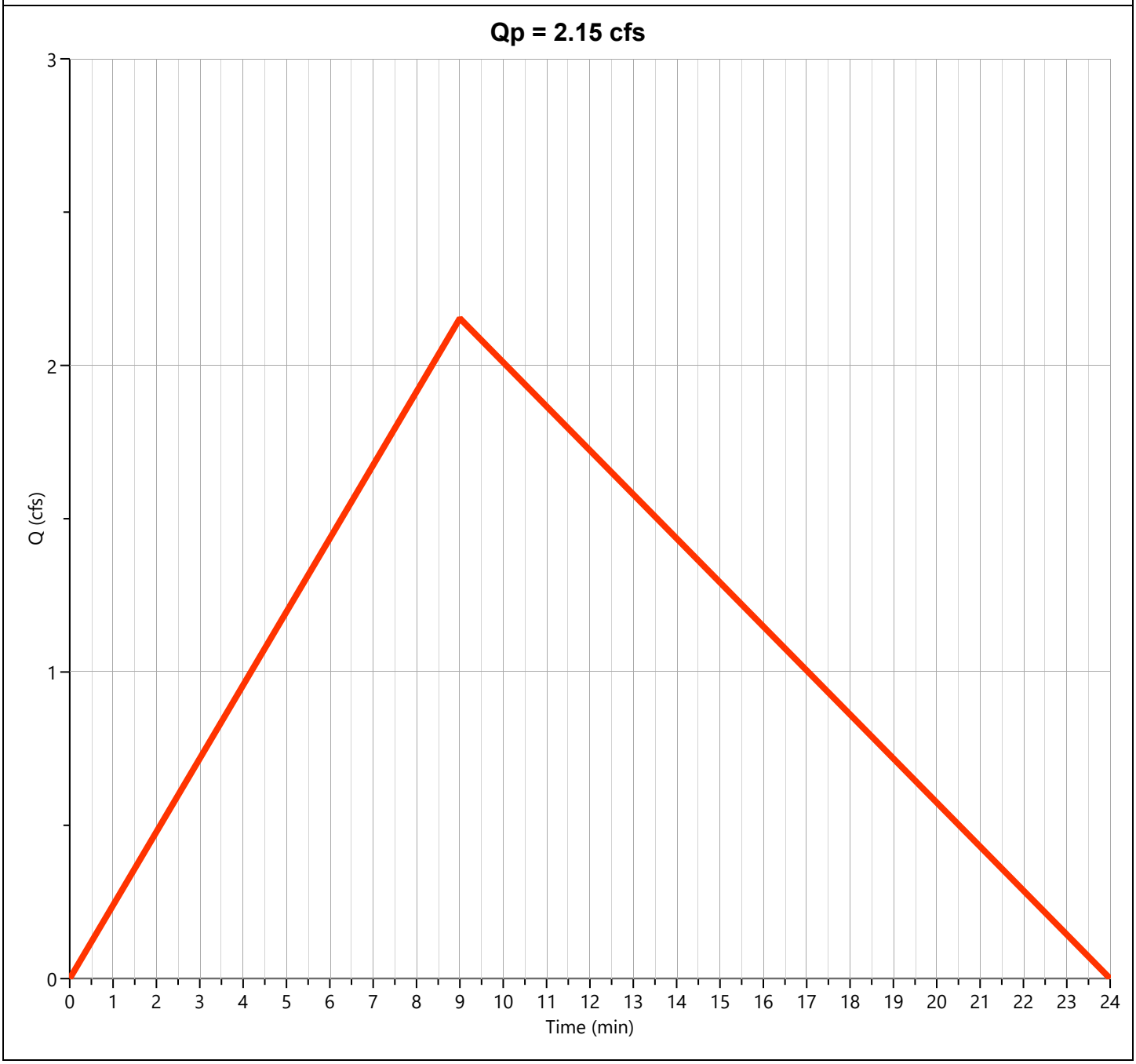
Hydrology Studio v 3.0.0.27

08-14-2025

Pre-Dev Basin

Hyd. No. 1

Hydrograph Type	= Rational	Peak Flow	= 2.154 cfs
Storm Frequency	= 10-yr	Time to Peak	= 0.15 hrs
Time Interval	= 1 min	Runoff Volume	= 1,552 cuft
Drainage Area	= 0.56 ac	Runoff Coeff.	= 0.61
Tc Method	= TR55 (See Worksheet)	Time of Conc. (Tc)	= 9.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 6.30 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	= 1/1.67



Hydrograph Report

Project Name: Jamey South Parking Lot

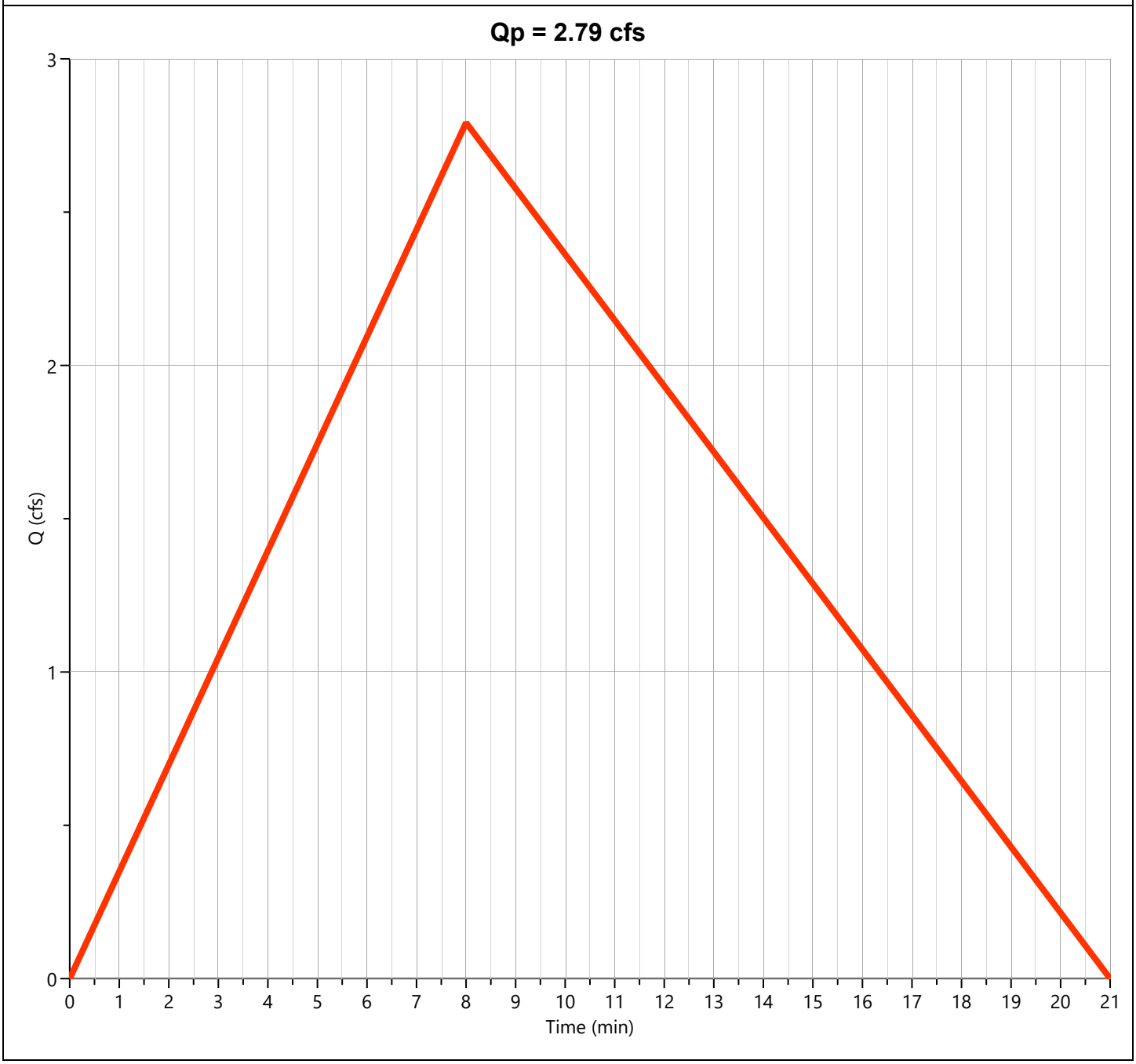
Hydrology Studio v 3.0.0.27

08-14-2025

Post-Dev Basin

Hyd. No. 2

Hydrograph Type	= Rational	Peak Flow	= 2.792 cfs
Storm Frequency	= 10-yr	Time to Peak	= 0.13 hrs
Time Interval	= 1 min	Runoff Volume	= 1,789 cuft
Drainage Area	= 0.56 ac	Runoff Coeff.	= 0.75
Tc Method	= TR55 (See Worksheet)	Time of Conc. (Tc)	= 8.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 6.65 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	= 1/1.67



Hydrograph 25-yr Summary

Project Name: Jamey South Parking Lot

08-14-2025

Hydrology Studio v 3.0.0.27

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	Rational	Pre-Dev Basin	2.474	0.15	1,784	----		
2	Rational	Post-Dev Basin	3.207	0.13	2,055	----		

Hydrograph Report

Project Name: Jamey South Parking Lot

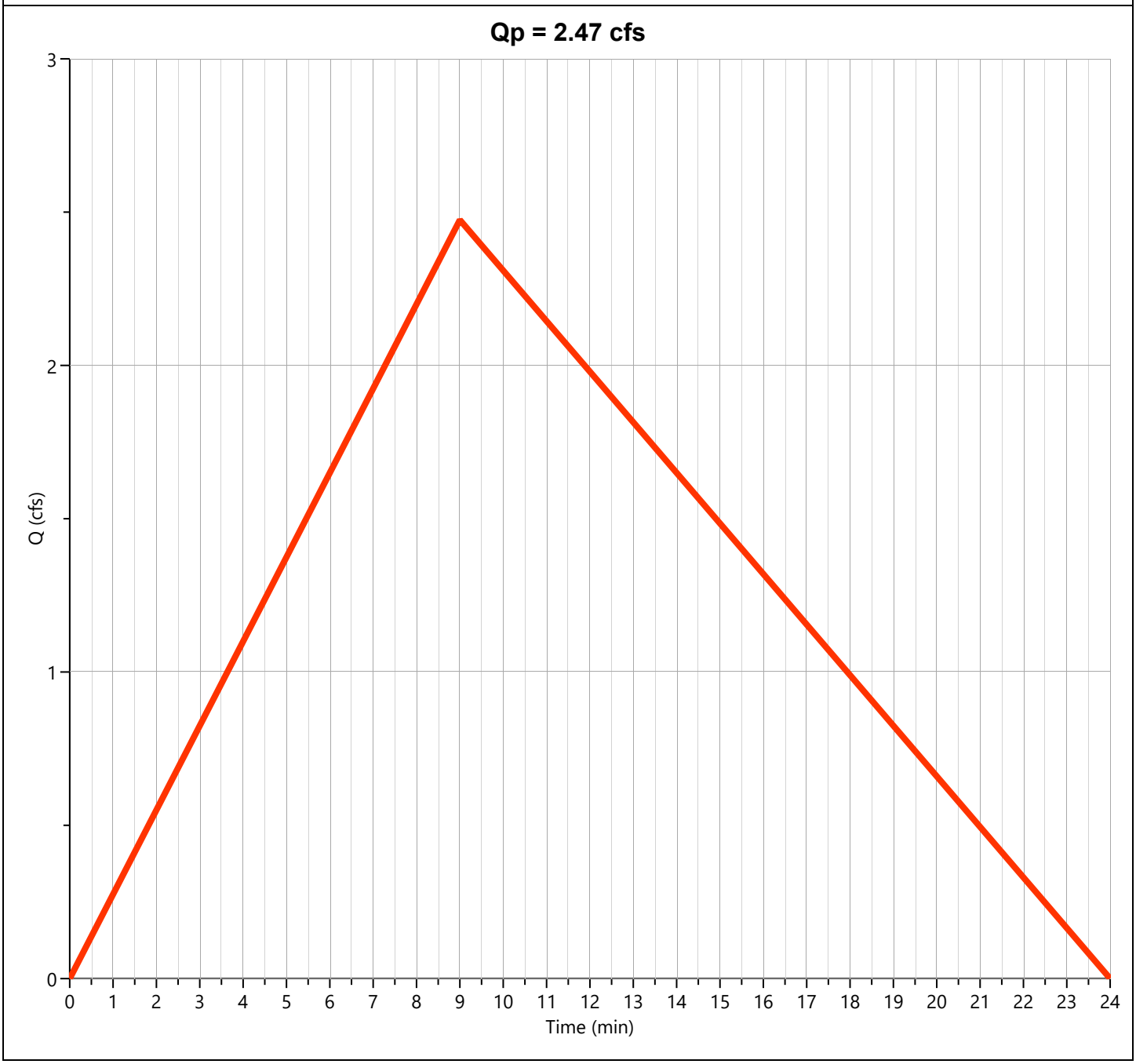
Hydrology Studio v 3.0.0.27

08-14-2025

Pre-Dev Basin

Hyd. No. 1

Hydrograph Type	= Rational	Peak Flow	= 2.474 cfs
Storm Frequency	= 25-yr	Time to Peak	= 0.15 hrs
Time Interval	= 1 min	Runoff Volume	= 1,784 cuft
Drainage Area	= 0.56 ac	Runoff Coeff.	= 0.61
Tc Method	= TR55 (See Worksheet)	Time of Conc. (Tc)	= 9.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 7.24 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	= 1/1.67



Hydrograph Report

Hydrology Studio v 3.0.0.27

08-14-2025

Post-Dev Basin

Hyd. No. 2

Hydrograph Type	= Rational	Peak Flow	= 3.207 cfs
Storm Frequency	= 25-yr	Time to Peak	= 0.13 hrs
Time Interval	= 1 min	Runoff Volume	= 2,055 cuft
Drainage Area	= 0.56 ac	Runoff Coeff.	= 0.75
Tc Method	= TR55 (See Worksheet)	Time of Conc. (Tc)	= 8.0 min
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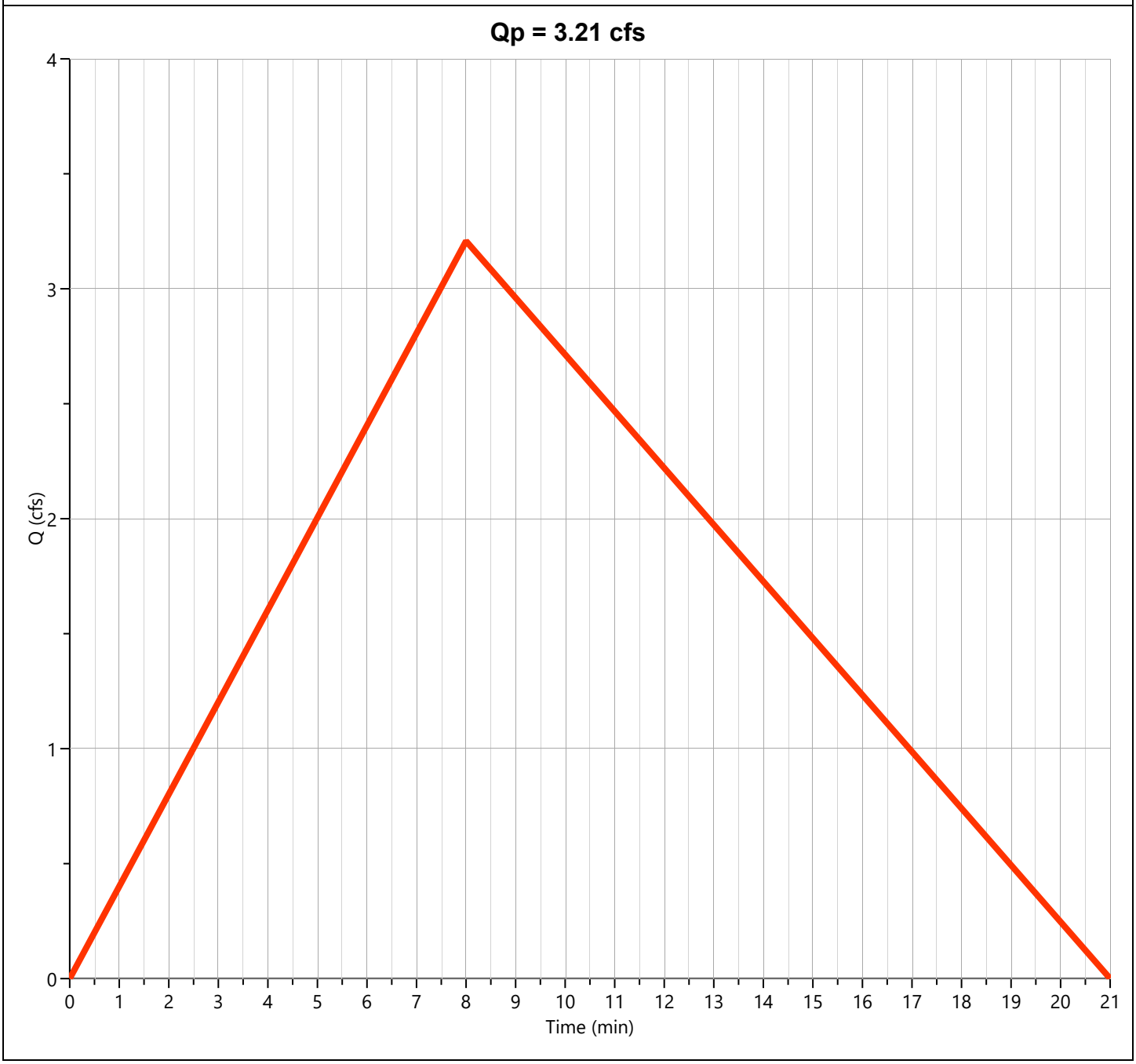


Hydrograph 50-yr Summary

Project Name: Jamey South Parking Lot

08-14-2025

Hydrology Studio v 3.0.0.27

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	Rational	Pre-Dev Basin	2.705	0.15	1,950	----		
2	Rational	Post-Dev Basin	3.506	0.13	2,246	----		

Hydrograph Report

Project Name: Jamey South Parking Lot

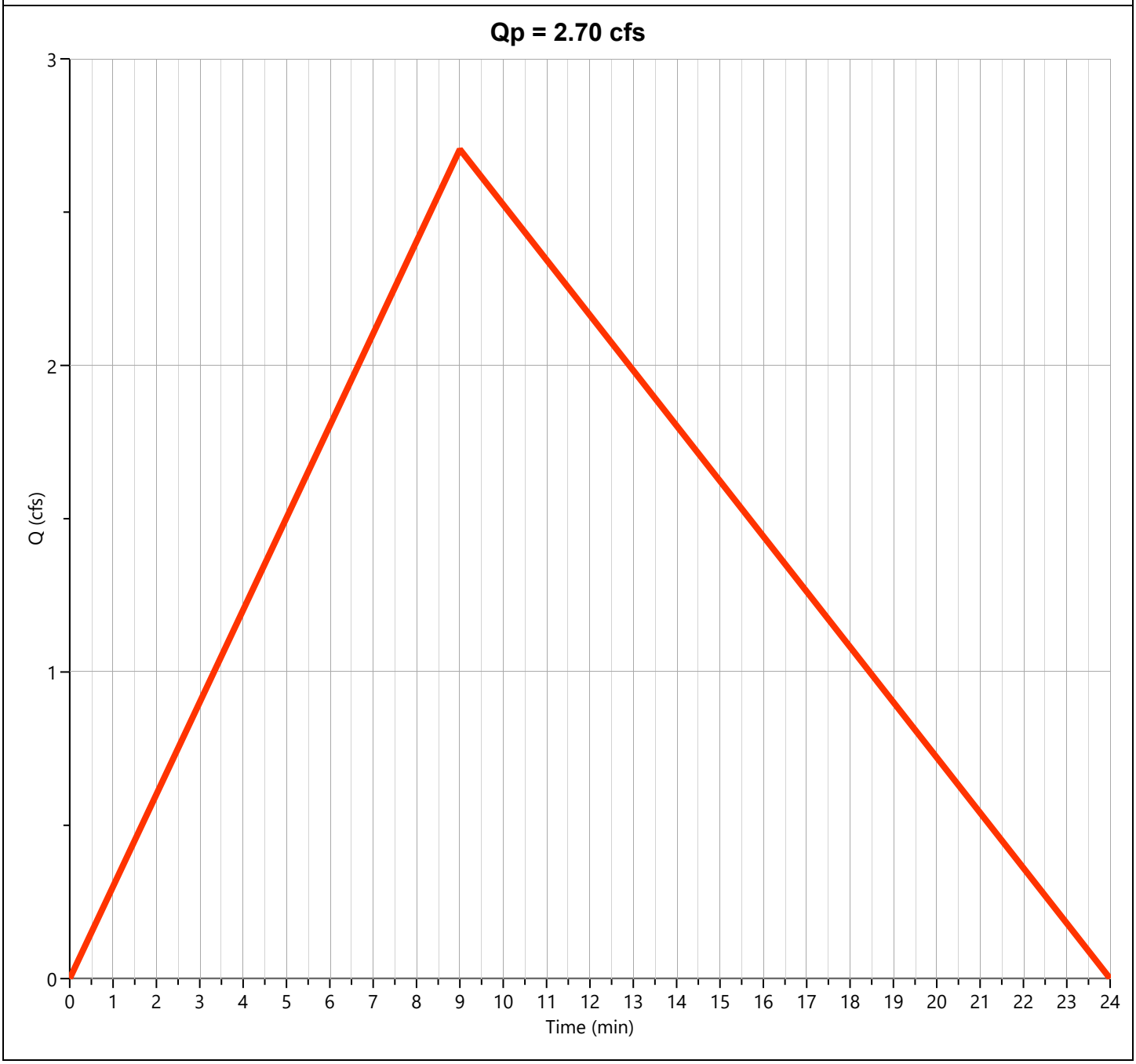
Hydrology Studio v 3.0.0.27

08-14-2025

Pre-Dev Basin

Hyd. No. 1

Hydrograph Type	= Rational	Peak Flow	= 2.705 cfs
Storm Frequency	= 50-yr	Time to Peak	= 0.15 hrs
Time Interval	= 1 min	Runoff Volume	= 1,950 cuft
Drainage Area	= 0.56 ac	Runoff Coeff.	= 0.61
Tc Method	= TR55 (See Worksheet)	Time of Conc. (Tc)	= 9.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 7.92 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	= 1/1.67



Hydrograph Report

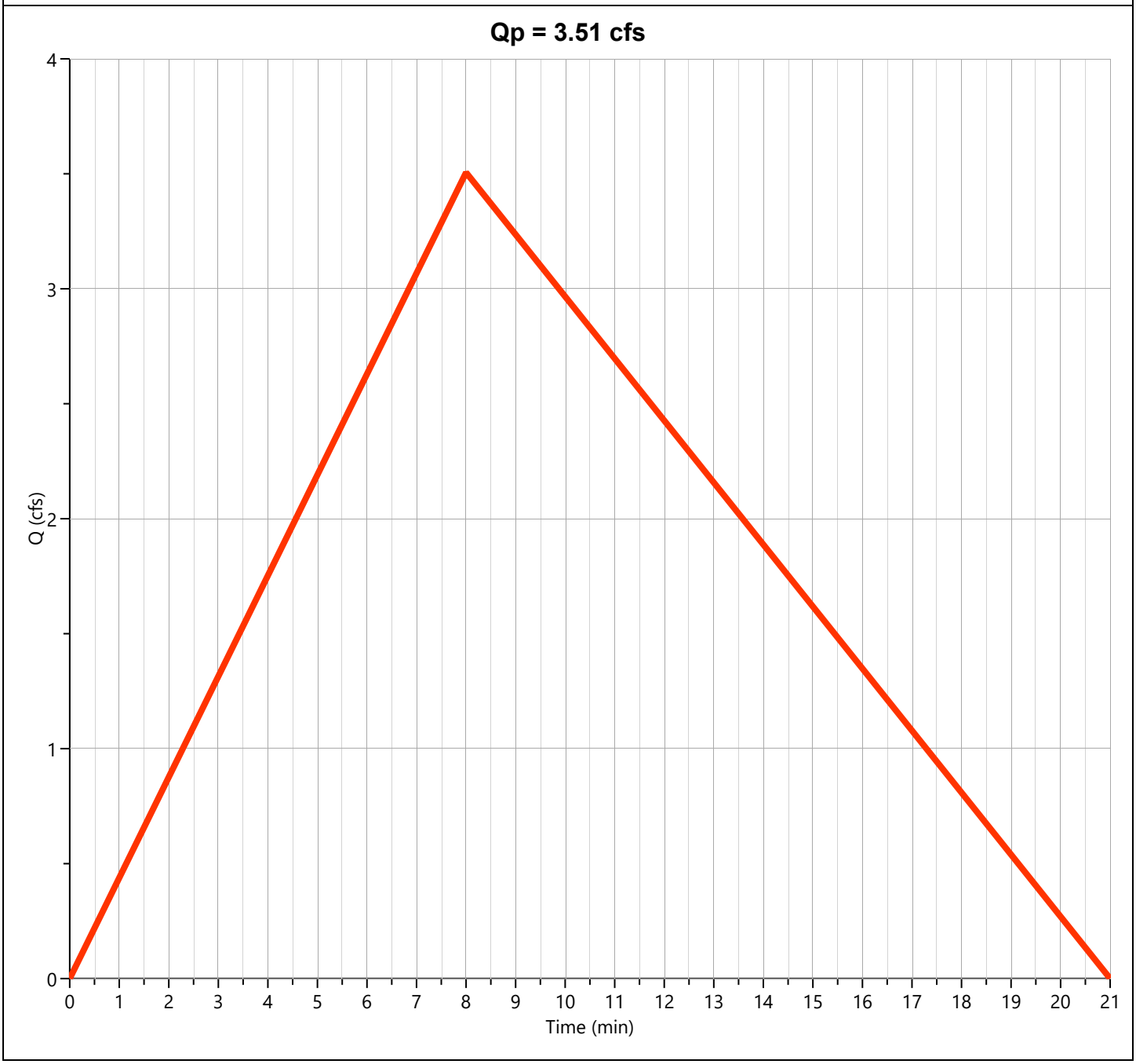
Hydrology Studio v 3.0.0.27

08-14-2025

Post-Dev Basin

Hyd. No. 2

Hydrograph Type	= Rational	Peak Flow	= 3.506 cfs
Storm Frequency	= 50-yr	Time to Peak	= 0.13 hrs
Time Interval	= 1 min	Runoff Volume	= 2,246 cuft
Drainage Area	= 0.56 ac	Runoff Coeff.	= 0.75
Tc Method	= TR55 (See Worksheet)	Time of Conc. (Tc)	= 8.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 8.35 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	= 1/1.67



Hydrograph 100-yr Summary

Project Name: Jamey South Parking Lot

08-14-2025

Hydrology Studio v 3.0.0.27

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	Rational	Pre-Dev Basin	2.938	0.15	2,118	----		
2	Rational	Post-Dev Basin	3.809	0.13	2,441	----		

Hydrograph Report

Project Name: Jamey South Parking Lot

Hydrology Studio v 3.0.0.27

08-14-2025

Pre-Dev Basin

Hyd. No. 1

Hydrograph Type	= Rational	Peak Flow	= 2.938 cfs
Storm Frequency	= 100-yr	Time to Peak	= 0.15 hrs
Time Interval	= 1 min	Runoff Volume	= 2,118 cuft
Drainage Area	= 0.56 ac	Runoff Coeff.	= 0.61
Tc Method	= TR55 (See Worksheet)	Time of Conc. (Tc)	= 9.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 8.60 in/hr
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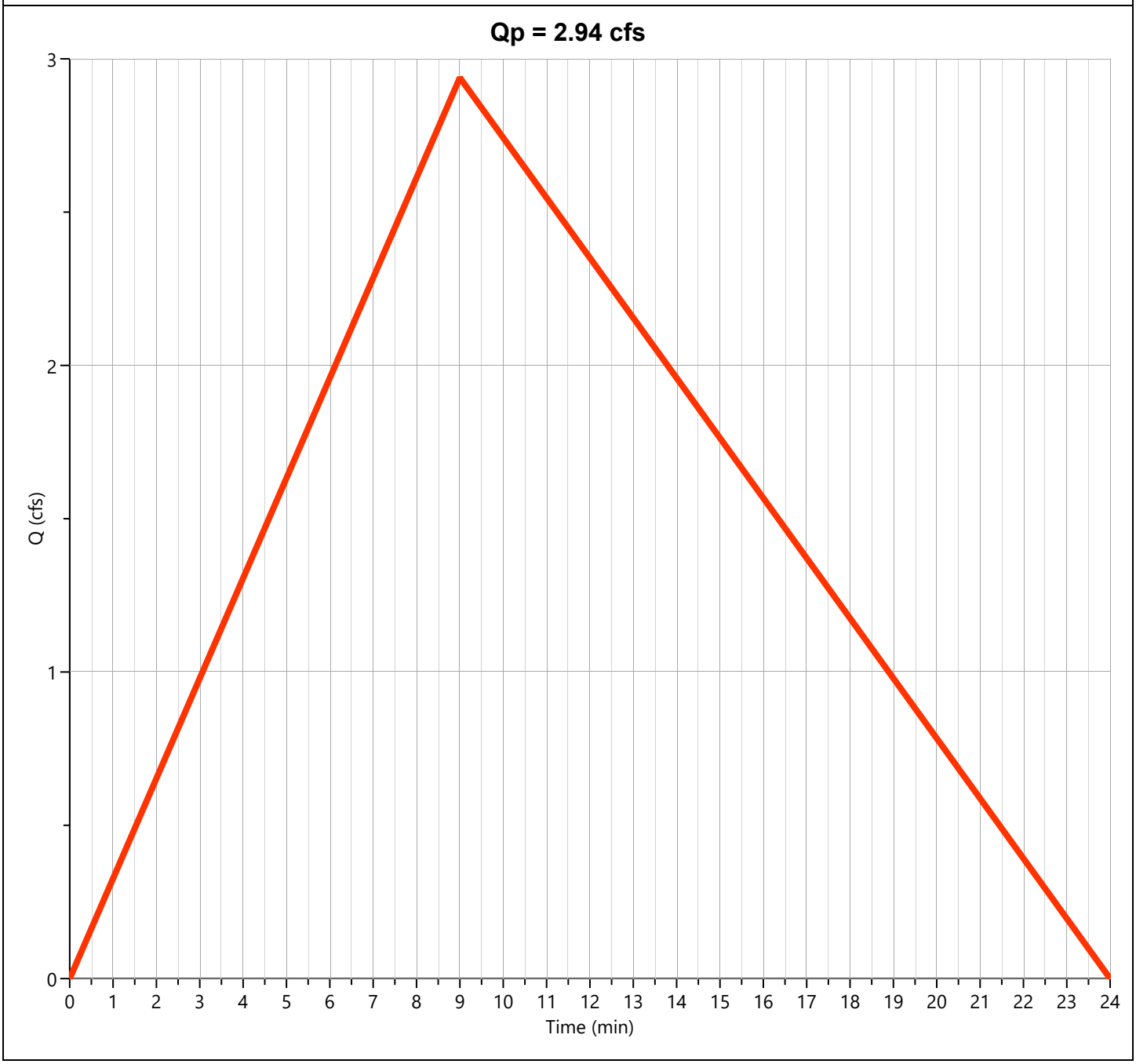

Hydrograph Report

Project Name: Jamey South Parking Lot

Hydrology Studio v 3.0.0.27

08-14-2025

Post-Dev Basin

Hyd. No. 2

Hydrograph Type	= Rational	Peak Flow	= 3.809 cfs
Storm Frequency	= 100-yr	Time to Peak	= 0.13 hrs
Time Interval	= 1 min	Runoff Volume	= 2,441 cuft
Drainage Area	= 0.56 ac	Runoff Coeff.	= 0.75
Tc Method	= TR55 (See Worksheet)	Time of Conc. (Tc)	= 8.0 min
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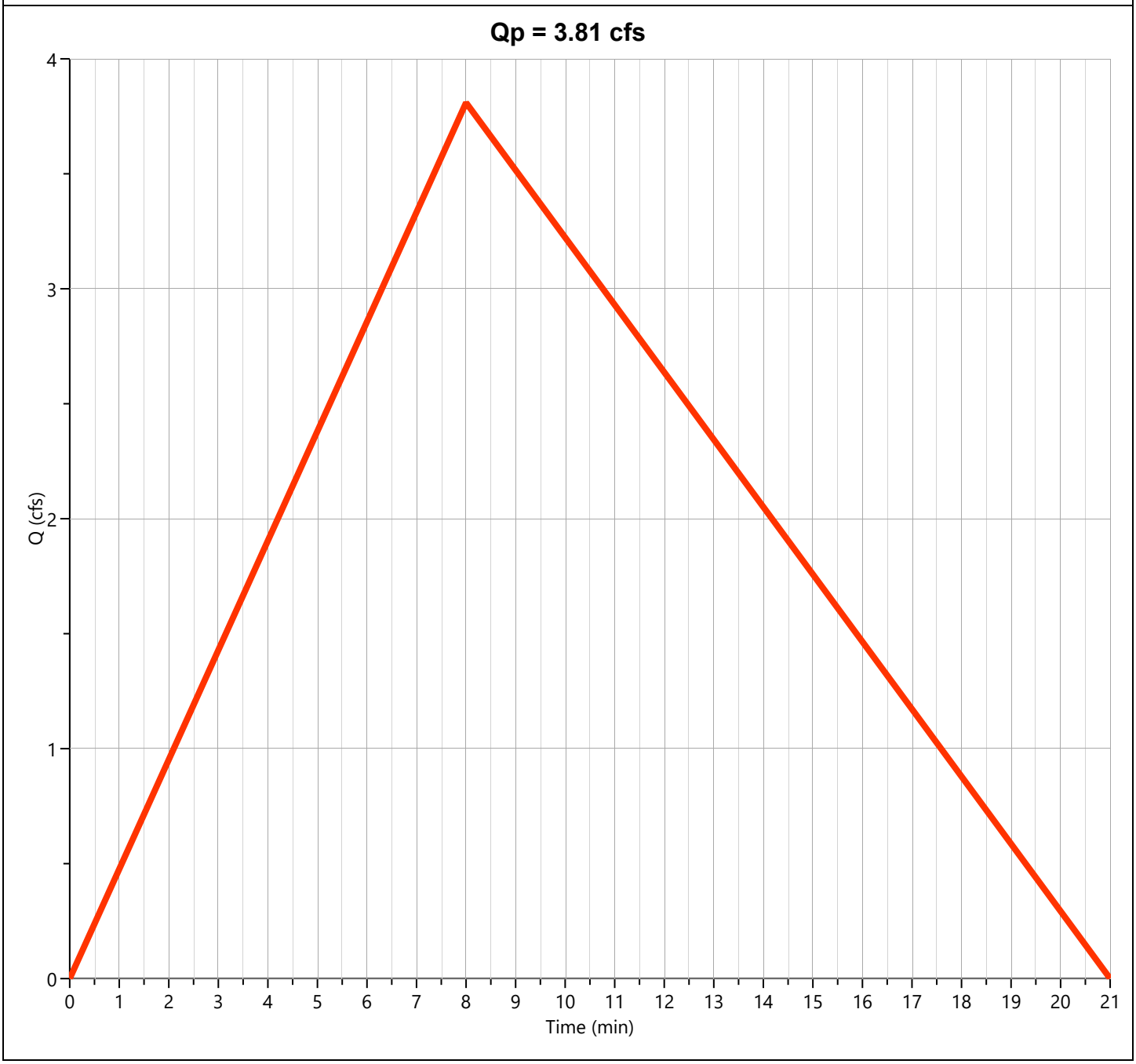


DETAILED PLANS:

STATE FARM – JAMEY SOUTH

PROPOSED PARKING LOT

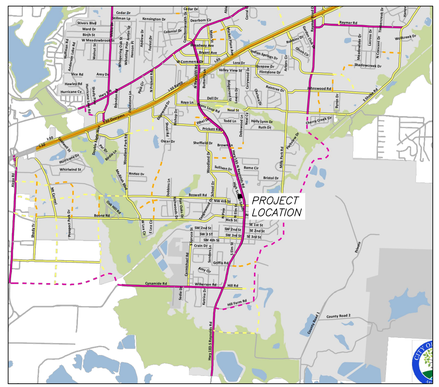
515 N. REYNOLDS ROAD
BRYANT, ARKANSAS

PRE–CONSTRUCTION COPY –

PLANS FOR BIDDING PURPOSES. QUANTITIES TO BE VERIFIED PRIOR TO CONSTRUCTION. CONTRACTOR TO VERIFY GRADES WITH ENGINEER PRIOR TO CONSTRUCTION.

4/22/2025 REV: 8/13/2025

VICINITY MAP
1" = 2000'



PREPARED FOR:

JAMEY SOUTH
515 N. REYNOLDS ROAD
BRYANT, AR 72022

STATE OF ARKANSAS
LICENSED PROFESSIONAL ENGINEER
No. 21954
TRISTIN SCOTT PHILLIPS

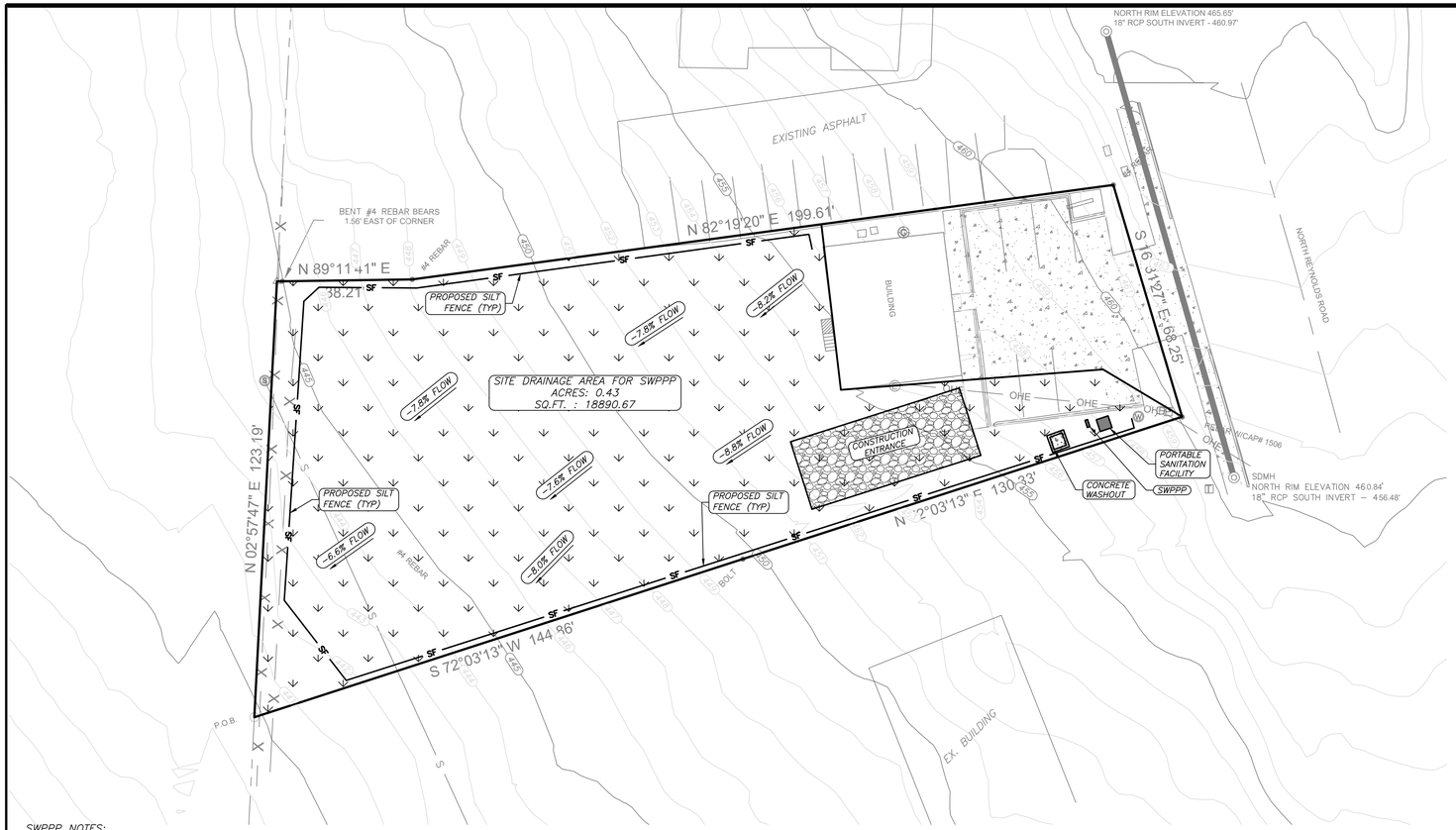
Prepared By:

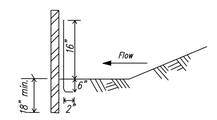
RICHARDSON ENGINEERING
Planning • Engineering • Development Consulting
325 W. SOUTH STREET, BENTON, AR 72015 (501)315-7225

	Revisions	Date
1	BRYANT DRC COMMENTS	5/6/2025
2	REMOVED BUILDING	7/30/2025
3	REMOVED DETENTION BASIN	8/13/2025

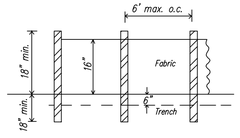
INDEX OF SHEETS

COVER SHEET	1
SWPPP	2
DEMOLITION PLAN	3
SITE/LANDSCAPE PLAN	4
GRADING PLAN	5





SIDE VIEW

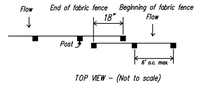


FRONT VIEW

NOTE:
Use wood or steel posts

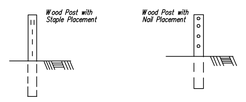
SF-A SILT FENCE (TYPE A)
NTS

*** ALL SILT FENCE SHALL BE WIRE REINFORCED.**



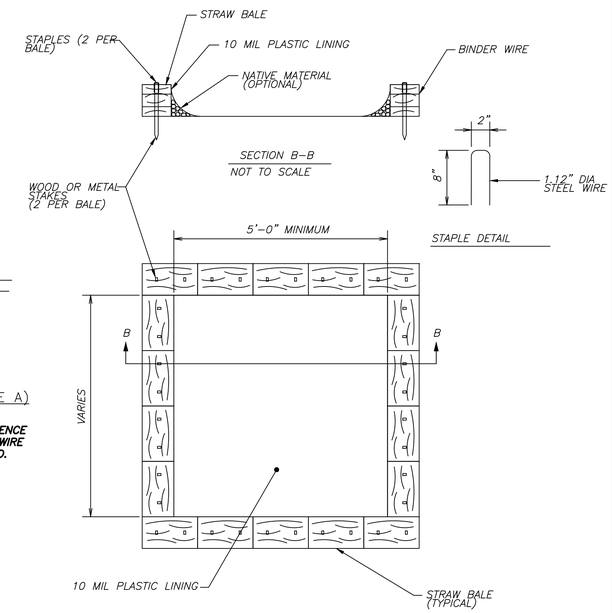
TOP VIEW - (Not to scale)

OVERLAP AT FABRIC ENDS



FRONT VIEWS

FASTENERS FOR SILT FENCES



SECTION B-B
NOT TO SCALE

STAPLE DETAIL

CONCRETE WASHOUT
NOT TO SCALE

SWPPP NOTES:

1.) DEVELOPMENT SHALL ADHERE TO BRYANT AND ADEQ STORMWATER REQUIREMENTS.
2.) SEDIMENT BARRIERS SHALL BE CONSTRUCTED AT EACH INLET UNTIL GRADING IS COMPLETE AND TOP SOIL IS STABILIZED.
3.) A SILT FENCE AND STRAW BALE DIKE SHALL BE PLACED AT POTENTIAL LOCATIONS OF HEAVY EROSION.
4.) TEMPORARY STRAW BALE DIKES ARE TO BE CONSTRUCTED NOT TO POND WATER ON ADJACENT PROPERTY.
5.) ROCK DIKE CHECK DAMS TO BE CONSTRUCTED TO MEET ADEQ REQUIREMENTS WHEN USED IN LIEU OF STRAW BALE DIKES.
6.) ALL TEMPORARY EROSION CONTROLS SHALL BE MAINTAINED UNTIL ALL CONSTRUCTION IS COMPLETE AND PERMANENT GROUND COVER HAS BEEN ESTABLISHED.
7.) ONE OF THE FOLLOWING GROUND COVER METHODS SHALL BE USED AT AREAS OF CLEARING OTHER THAN FUTURE PAVEMENT SURFACES.
STRAW OR HAY-LOOSE 2.0 TONS/ACRE
STRAW OR HAY-TIED, ANCHORED, OR TACKED 1.5 TONS/ACRE
8.) SOIL EXPOSED FOR MORE THAN 14 DAYS WITH NO CONSTRUCTION ACTIVITY SHALL BE SEEDED OR REEMITTED.
9.) CONSTRUCTION ENTRANCES/EXITS SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OF MUD ON TO PUBLIC RIGHT-OF-WAY.
10.) ADDITIONAL EROSION CONTROL MEASURES TO BE EMPLOYED WHERE NECESSARY BY SITE CONDITIONS.
11.) CONTRACTOR SHALL USE "BEST MANAGEMENT PRACTICES" (BMP'S) WHEN IMPLEMENTING AND MAINTAINING SEDIMENT AND RUN-OFF CONTROLS.

GENERAL EROSION CONTROL NOTES

1. THE LOCATION OF KNOWN SURFACE & SUBSURFACE STRUCTURES, PIPE, POWER, GAS, PHONE, ETC. ARE SHOWN ON THE PLANS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING INFORMATION AND SATISFYING THEMSELVES AS TO THE LOCATION OF THE AFOREMENTIONED ITEMS, SHOWN AND NOT SHOWN. ALL REPAIRS OR RELOCATIONS NECESSARY SHALL BE MADE AS REQUIRED BY THE OWNER OF THE UTILITY OR STRUCTURE. THE COST OF SUCH REPAIRS OR RELOCATIONS NECESSARY SHALL BE BORNE BY THE CONTRACTOR.

2. ALL STREETS, DRIVES, WALKS, DRAINAGE STRUCTURES, FENCES, ETC. THAT ARE DISTURBED SHALL BE RESTORED TO THEIR ORIGINAL OR BETTER CONDITION USING LIKE MATERIALS. COST OF SUCH REPAIRS SHALL BE BORNE BY THE CONTRACTOR UNLESS PROVISION FOR PAYMENT IS MADE IN THE PROPOSAL.

3. EROSION CONTROL DEVICES SHALL BE INSTALLED AS THE PROJECT PROGRESSES AND AREAS ARE DISTURBED.

4. THE CONTRACTOR IS REQUIRED TO NOTIFY THE ONE CALL CENTER AT 1-800-482-8998 48 HOURS PRIOR TO DIGGING IN ORDER THAT UNDERGROUND UTILITIES IN THE AREA CAN BE LOCATED.

5. ALL INLETS ON AND ADJACENT TO THE SITE SHALL BE PROTECTED FROM SEDIMENT.

6. STORM WATER CONTROLS SHALL BE INSPECTED AT LEAST ONCE EVERY 7 DAYS.

7. ALL EROSION CONTROL DEVICES ARE TO BE MAINTAINED IN GOOD WORKING CONDITION THROUGHOUT THE DURATION OF THIS PROJECT AND UNTIL PERMANENT VEGETATION IS ESTABLISHED. ALL SEDIMENT TRAPS ARE TO BE CLEANED AS REQUIRED.

8. ALL DISTURBED AREAS LEFT IDLE FOR A PERIOD OF 14 DAYS OR LONGER ARE TO RECEIVE TEMPORARY VEGETATION AND MULCHED.

9. REMOVE EROSION CONTROL DEVICES AFTER PERMANENT VEGETATION IS ESTABLISHED.

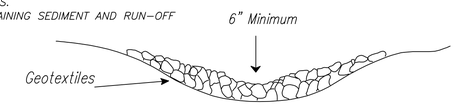
10. DEVELOPMENT SHALL ADHERE TO CITY OF BRYANT AND ADEQ STORM WATER REQUIREMENTS.

11. ADDITIONAL EROSION CONTROL MEASURES TO BE EMPLOYED WHERE NECESSARY BY SITE CONDITIONS.

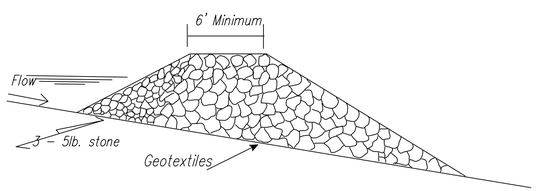
12. CONTRACTOR TO COMPLY WITH ALL OSHA SAFETY REQUIREMENTS.

EXISTING CONTOURS

SF PROPOSED SILT FENCE

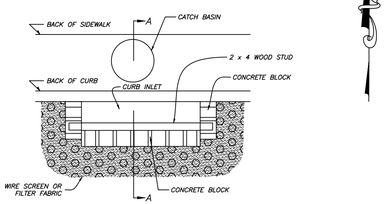


NOTE: Sediment Trap is to be cleaned out when volume becomes half full.

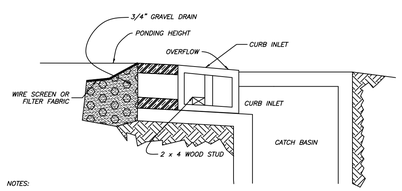


NOTES:
1. FOR USE IN SMALL CHANNELS WITH DRAINAGE AREAS OF 50 ACRES OR LESS.
2. MUST BE USED IN CONJUNCTION WITH OTHER APPROPRIATE SEDIMENT CONTROL MEASURES.
3. USE BELOW CULVERT INSTALLATIONS, DAM CONSTRUCTION, OR ANY PROJECT THAT MAY INVOLVE GRADING ACTIVITY DIRECTLY IN A STREAM.
4. NOT INTENDED TO SUBSTANTIALLY IMPOUND WATER.
5. USE AT THE UPSTREAM END OF PONDS OR LAKES.
6. EDGES SHOULD NOT BE HIGHER THAN THE CHANNEL BANKS.
7. CENTER SHOULD BE AT LEAST 6 INCHES LOWER THAN THE OUTER EDGES OF THE DAM AT THE CHANNEL BANKS.
8. HEIGHT SHOULD NOT EXCEED ELEVATION OF UPSTREAM PROPERTY LINE.
9. SIDE SLOPES SHOULD BE 2:1 OR FLATTER.
10. TOP WIDTH SHOULD BE GREATER THAN 6 FEET.
11. EXTEND COMPLETELY ACROSS THE CHANNEL AND SECURELY TIE INTO BOTH CHANNEL BANKS
12. REQUIRES PERIODIC INSPECTION AND MAINTENANCE.
13. SEDIMENT REMOVED WHEN IT REACHES ONE-HALF OF THE ORIGINAL DAM HEIGHT

ROCK DAM
NOT TO SCALE



PLAN VIEW



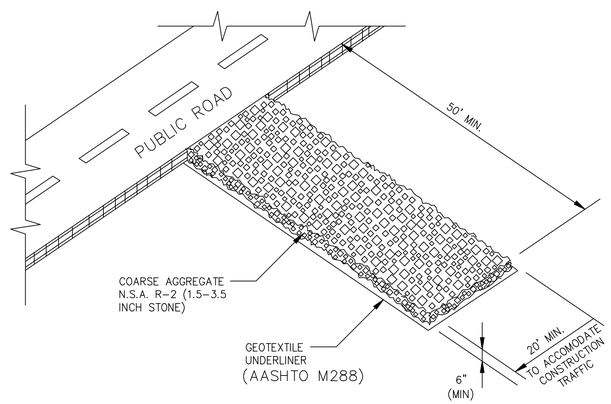
NOTES:
1.) USE BLOCK AND GRAVEL TYPE SEDIMENT BARRIER WHEN CURB INLET IS LOCATED IN GENTLY SLOPING STREET SEGMENT, WHERE WATER CAN POND AND ALLOW SEDIMENT TO SEPARATE FROM RUNOFF.
2.) BARRIER SHALL ALLOW FOR OVERFLOW FROM SEVERE STORM EVENT.
3.) INSPECT BARRIERS AND REMOVE SEDIMENT AFTER EACH STORM EVENT. SEDIMENT AND GRAVEL MUST BE REMOVED FROM THE TRAVELED WAY IMMEDIATELY.

SECTION A - A

CURB INLET SEDIMENT BARRIER

GRAPHIC SCALE
20 0 20 40 60
(IN FEET)
1 inch = 20 ft.

1. LOCATE STONE STABILIZED PAD AT ANY POINT WHERE VEHICULAR TRAFFIC WILL BE LEAVING THE CONSTRUCTION SITE ONTO A PUBLIC RIGHT-OF-WAY, STREET, ALLEY, SIDEWALK, OR PARKING AREA OR ANY OTHER AREA WHERE THERE IS A TRANSITION FROM BARE SOIL TO A PAVED AREA.
2. WIDTH - 20'-0 MINIMUM BUT NOT LESS THAN FULL WIDTH OF ALL POINTS OF VEHICULAR EGRESS. LENGTH - 50'-0 MINUMUM
3. MAINTAIN THE EXIT TO PREVENT TRACKING OR FLOW OF MUD ONTO PUBLIC RIGHTS- OF-WAY. PROVIDE PERIODIC TOP DRESSING WITH 1.5 - 3.5" STONE, AS CONDITIONS DEMAND. IMMEDIATELY REMOVE ALL MATERIALS SPILLED, DROPPED, WASHED, OR TRACKED FROM VEHICLE OR SITE ONTO ROADWAY OR INTO STORM DRAINS. CLEAN WHEELS AND TIRES TO REMOVE MUD PRIOR TO ENTRANCE ON TO PUBLIC RIGHTS-OF-WAY.
4. WHEN WASHING IS REQUIRED, DO SO ON AREAS STABILIZED WITH CRUSHED STONE DRAINING INTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN.



CONSTRUCTION EXIT
NOT TO SCALE

RICHARDSON ENGINEERING
Planning • Engineering • Development Consulting
325 W. SOUTH STREET, BENTON, AR 72015 (501)315-7225

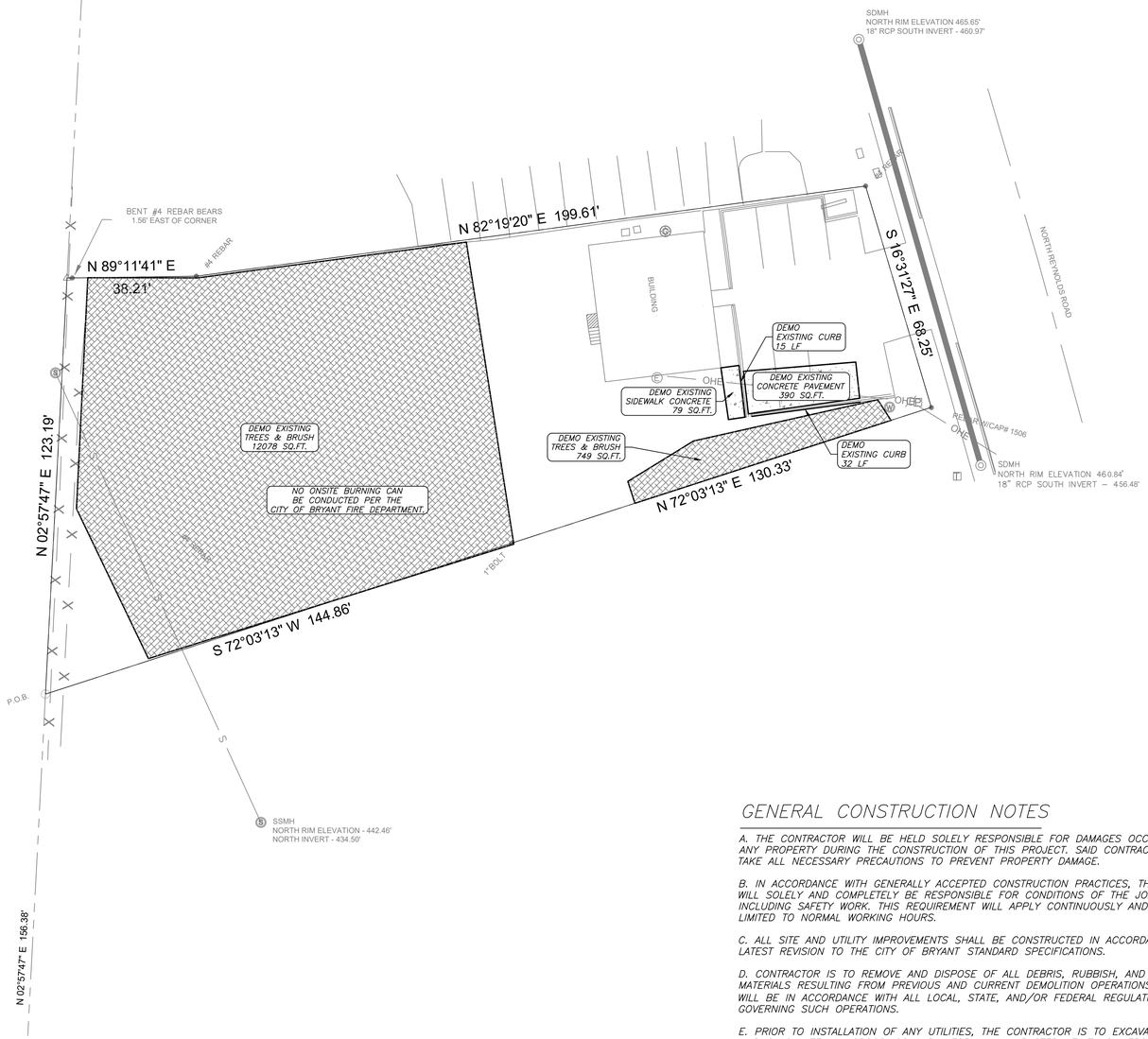
CERTIFICATE OF AUTHORIZATION
Richardson Engineering, PLLC
No. 2519
STATE OF ARKANSAS

SWPPP PLAN
STATE FARM - JAMEY SOUTH
PROPOSED PARKING LOT
515 N. REYNOLDS ROAD
BRYANT, ARKANSAS

Prepared For:
JAMEY SOUTH
515 N. REYNOLDS ROAD
BRYANT, AR 72022

No.	Revisions	Date
1	BRYANT DRC COMMENTS	5/6/2025
2	REMOVED BUILDING	7/30/2025
3	REMOVED DETENTION BASIN	8/13/2025

PROJECT #: 025-007
Date: 4/22/2025
REV: 8/13/2025
Scale: 1" = 20'
Sheet: 2 of 5



DEMO NOTES:
1) QUANTITIES ARE APPROXIMATE. CONTRACTOR IS RESPONSIBLE FOR VERIFYING QUANTITIES.

GENERAL CONSTRUCTION NOTES

A. THE CONTRACTOR WILL BE HELD SOLELY RESPONSIBLE FOR DAMAGES OCCURRING TO ANY PROPERTY DURING THE CONSTRUCTION OF THIS PROJECT. SAID CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PREVENT PROPERTY DAMAGE.

B. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL SOLELY AND COMPLETELY BE RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND WILL NOT BE LIMITED TO NORMAL WORKING HOURS.

C. ALL SITE AND UTILITY IMPROVEMENTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST REVISION TO THE CITY OF BRYANT STANDARD SPECIFICATIONS.

D. CONTRACTOR IS TO REMOVE AND DISPOSE OF ALL DEBRIS, RUBBISH, AND OTHER MATERIALS RESULTING FROM PREVIOUS AND CURRENT DEMOLITION OPERATIONS. DISPOSAL WILL BE IN ACCORDANCE WITH ALL LOCAL, STATE, AND/OR FEDERAL REGULATIONS GOVERNING SUCH OPERATIONS.

E. PRIOR TO INSTALLATION OF ANY UTILITIES, THE CONTRACTOR IS TO EXCAVATE, VERIFY, AND CALCULATE ALL CROSSINGS AND INFORM ANY IMPACTED UTILITY OWNERS OF ANY CONFLICTS PRIOR TO CONSTRUCTION.

F. CONSTRUCTION SHALL NOT START ON ANY WATER UTILITY TIE–INS UNTIL APPROVAL IS GIVEN BY CITY OF BRYANT. CONTRACTOR SHALL NOT OPERATE ANY VALVE, HYDRANT, OR WATER UTILITY APPURTENANCE NOR SHALL HE ATTACH TO OR TAP ANY WATER UTILITY MAIN WITHOUT APPROVAL FROM BRYANT. THE CONTRACTOR SHALL BEAR THE COST AND CONSEQUENCE OF ANY DISRUPTION OF UTILITY OPERATION CAUSED BY CONSTRUCTION.

G. FIBER OPTIC CABLE ON AND/OR ADJACENT TO THIS SITE WERE NOT LOCATED BY THE SURVEY AND ARE NOT SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ANY FIBER OPTIC CABLES AT TO NEAR THIS SITE AND TAKE ALL NECESSARY AND REQUIRED PRECAUTIONS TO PROTECT ANY EXISTING FIBER OPTIC CABLES. CONTRACTORS SHALL COORDINATE ALL EFFORTS WITH OWNER OF FIBER OPTIC CABLES OR THEIR DESIGNATED REPRESENTATIVE.

H. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING "ONECALL" SERVICE TO MARK ALL UTILITIES PRIOR TO ANY DEMOLITION, EARTHWORK, OR UTILITY WORK ON THIS SITE.

I. ANY LAND CLEARING, CONSTRUCTION, OR DEVELOPMENT INVOLVING THE MOVEMENT OF MATERIALS, UTILITIES OR GROUND EXCAVATION SHALL BE IN ACCORDANCE WITH THE STORMWATER POLLUTION PREVENTION PLAN.

J. THE APPROXIMATE LOCATION OF KNOWN SURFACE AND SUBSURFACE STRUCTURES, PIPES, POWER, GAS, PHONE, ETC. ARE SHOWN ON THE DESIGN DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE EXACT LOCATION OF THE AFOREMENTIONED ITEMS, SHOWN AND NOT SHOWN.

K. CONTRACTOR TO ADHERE TO CURRENT OSHA REGULATIONS, INCLUDING EXCAVATION & TRENCH SAFETY.

L. CONTRACTOR SHALL CONTACT THE CITY OF BRYANT STREET DEPARTMENT IN REGARD TO MAINTENANCE OF TRAFFIC PRIOR TO COMMENCEMENT OF WORK WITHIN STREET RIGHT OF WAY.

M. CONTRACTOR SHALL CONTACT CITY OF BRYANT PRIOR TO COMMENCEMENT OF UTILITY CONSTRUCTION OR TIE–INS.

UTILITIES:

SANITARY SEWER:
BRYANT SEWER DEPARTMENT
1019 S.W. 2ND ST.
BRYANT, AR 72022
501–943–0469

WATER:
BRYANT WATER DEPARTMENT
210 S.W. 3RD ST.
BRYANT, AR 72022
501–943–0441

ELECTRIC:
ENTERGY
425 W. CAPITAL AVE.
LITTLE ROCK, AR 72201
1–800–368–3749

NATURAL GAS:
SUMMIT UTILITIES
400 WEST CAPITOL #600
LITTLE ROCK, ARKANSAS
888–498–0409

811 Know what's **below.** **Call** before you dig.

GRAPHIC SCALE
20 0 20 40 60
(IN FEET)
1 inch = 20 ft.

Legend

- Property Boundary
- Surveyed lines
- RW — Road Right of Way
- T — Telephone
- FO — FO — FO — Fiber Optic Line
- Road Center line
- X — Wire / Chainlink Fence
- S — Sanitary Sewer Main
- SS — Sanitary Sewer Service
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- OHE — Overhead Powerline
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- GS — Gas Service
- SF — Silt Fence
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- Existing Sewer Mnahole
- Existing Storm Box
- Existing Fire Hydrant
- Guy Wire
- Computed Corner
- Found Monument (Labeled)
- Utility Pole
- Gas Meter
- Sewer Stubup

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DEMOLITION PLAN
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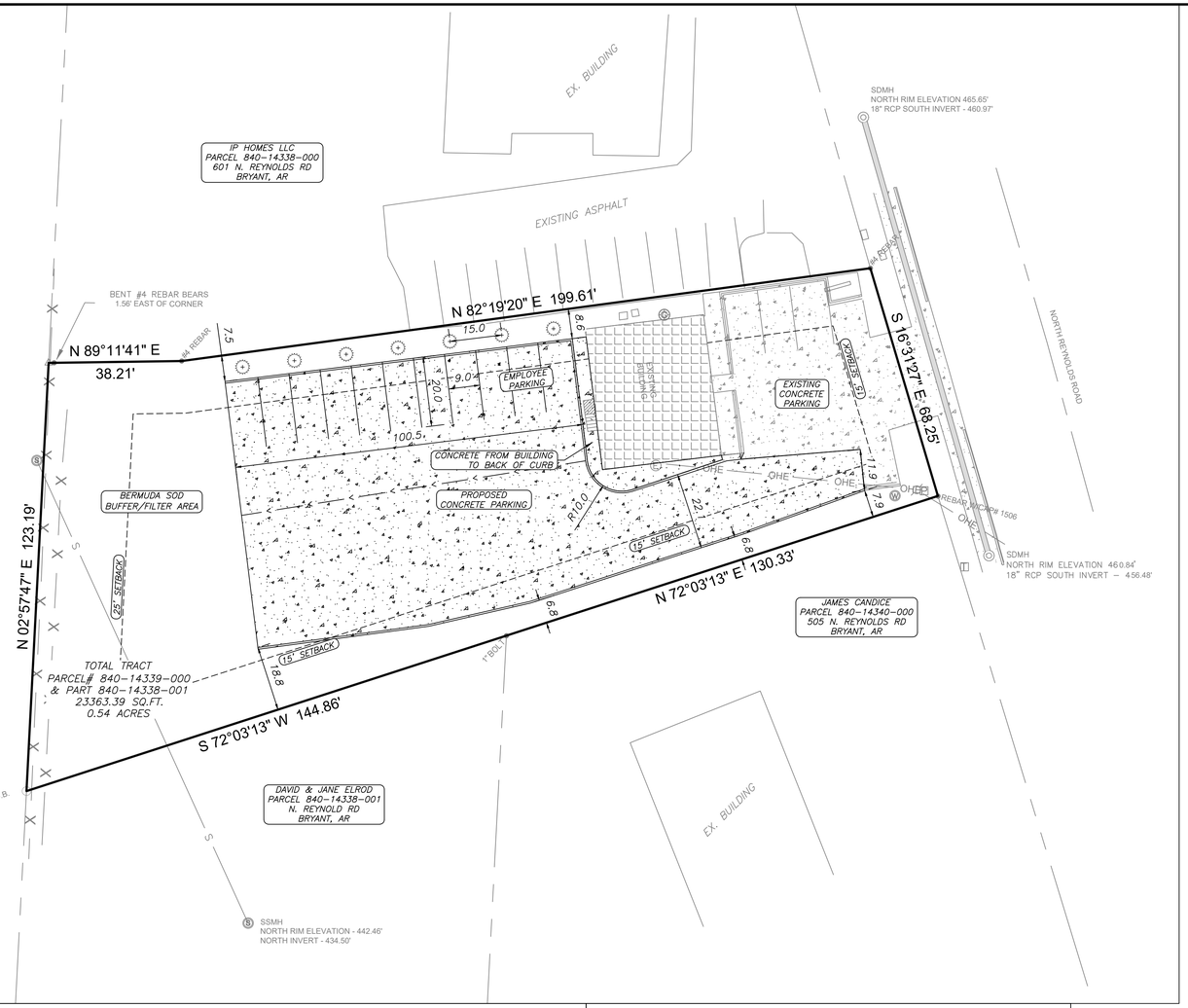

GENERAL NOTES:

1.) SURVEYING PROVIDED BY RASBURRY SURVEYING LLC.
2.) PAVEMENT SHALL BE CONSTRUCTED IN ACCORDANCE WITH THIS PLAN AND BASED ON GEOTECHNICAL ANALYSIS OF THE SOIL CONDITIONS.
3.) ATTENTION IS CALLED TO CONSTRUCTION DETAILS FOR ADDITIONAL INFORMATION.
4.) CONSTRUCTION SITE SHALL ADHERE TO BRYANT STORMWATER REQUIREMENTS, AND SHALL MEET ALL APPLICABLE ADEQ STANDARDS FOR EROSION CONTROL MEASURES.
5.) ALL UTILITIES TO BE LOCATED PRIOR TO CONSTRUCTION (ONE CALL, CITY, ETC.)
6.) CONTRACTOR TO ADHERE TO CURRENT OSHA REGULATIONS INCLUDING EXCAVATION & TRENCH SAFETY.
7.) THE APPROXIMATE LOCATION OF KNOWN SURFACE AND SUBSURFACE STRUCTURES, PIPES, POWER, GAS, PHONE, ETC. ARE SHOWN ON THE DESIGN DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE EXACT LOCATION OF THE AFOREMENTIONED ITEMS, SHOWN AND NOT SHOWN.

SITE NOTES

1.) PROJECT SITE AREA 0.54 ACRES (23363.39 SQUARE FEET).
2.) PROJECT PROVIDES 11 PARKING SPACES.
3.) CURRENT ZONING: C-2
4.) PROPOSED USE: PROPOSED PARKING LOT
5.) BUILDING LINES TO BE NOTED ON SITE PLAN.
6.) PROPOSED LOCATIONS OF TRAFFIC CONTROL MARKERS ARE APPROXIMATE. ACTUAL LOCATION AND INSTALLATION MUST MEET MUTCD AND BENTON ROAD DEPT. SPECS.
7.) CONTRACTOR SHALL INCLUDE IN BID THE COST FOR COMPACTION TESTS ON SUBGRADE & BASE. TEST TO BE CONDUCTED AS PER GEOTECHNICAL ENGINEERS SPECS.
8.) CONTRACTOR TO INCLUDE IN BID THE COST OF MATERIAL AND INSTALLATION OF STREET SIGNS & TRAFFIC CONTROL SIGNS.
9.) CONTRACTOR TO ADHERE TO CURRENT OSHA REGULATIONS INCLUDING EXCAVATION & TRENCH SAFETY.
10.) THIS PROPERTY (AS SHOWN) IS NOT IN THE 100 YEAR FLOOD HAZARD AREA. REFERENCE FIRM MAP 05125038OE EFFECTIVE 6/5/2020
11.) REFER TO LANDSCAPING PLAN FOR REQUIREMENTS/LIGHTING PLAN TO BE VERIFIED W/OWNER. REVISIONS TO PLAN TO BE SUBMITTED TO CITY.
12.) IRRIGATION SYSTEMS TO BE DESIGNED BY OTHERS.
13.) THERE ARE NO EROSION PROBLEMS ON THE SITE OR KNOWN EROSION PROBLEMS WITHIN 300' DOWNSTREAM.
14.) THERE IS NO SURFACE EVIDENCE OF EXISTING OR ABANDONED WATER WELLS, SUMPS, CESSPOOLS, SPRING WATER IMPOUNDMENTS, AND UNDERGROUND STRUCTURES WITHIN THE PROJECT.
15.) PAVEMENT SHALL BE CONSTRUCTED IN ACCORDANCE WITH AHTD SPECIFICATIONS, AND BASED ON GEOTECHNICAL ANALYSIS OF THE SOIL CONDITIONS. CONTRACTOR SHALL INCLUDE IN BID THE COST FOR COMPACTION TESTS ON SUBGRADE & BASE. TEST TO BE CONDUCTED AS PER GEOTECHNICAL SPECS.
16.) ALL SITE LIGHTING IS TO BE LOW LEVEL AND DIRECTIONAL, SHIELDED DOWNWARD AND INTO THE SITE. DESIGNED BY OTHER AS PER CITY OF BRYANT REQUIREMENTS.

UTILITIES:

SANITARY SEWER:
BRYANT SEWER DEPARTMENT
1019 S.W. 2ND ST.
BRYANT, AR 72022
501-943-0469

WATER:
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ENGINEER
RICHARDSON ENGINEERING, PLLC
ADDRESS: 325 W.SOUTH ST.
BENTON, AR. 72015
PHONE NO. (501) 315-7225
PROJECT REPRESENTATIVE:
TRISTIN PHILLIPS, P.E.

SURVEYOR
RASBURRY SURVEYING LLC
308 W. SOUTH STREET
BENTON, AR 72015
(501)860-6893

DEVELOPERS
JAMEY SOUTH
515 N. REYNOLDS ROAD
BRYANT, AR 72022

STATE OF ARKANSAS
LICENSED PROFESSIONAL ENGINEER
No. 21954
TRISTIN SCOTT PHILLIPS

LEGAL DESCRIPTION DOC 2022-002110

PART OF THE SOUTHWEST QUARTER OF THE SOUTHEAST QUARTER, SECTION 27, TOWNSHIP 1 SOUTH, RANGE 14 WEST, CITY OF BRYANT, SALINE COUNTY, ARKANSAS, AND DESCL'IBED AS FOLLOWS: FROM THE.SOUTHWEST CORNER OF SAID SOUTHWEST QUARTER OF SOUTHEAST QUARTER, RUN THENCE NORTH 87 DEG, 57 MIN. EAST ALONG THE SOUTH LINE OF SAID SOUTHWEST QUARTER OF SOUTHEAST QUARTER, 137.0 FEET; THENCE NORTH 00 DEG. 41 MIN. WEST, 198.1 FEET TO THE POINT OF BEGINNING; THENCE NORTH 66 DEG. 45 MIN, EAST, 129.9 FEET TO THE WEST RIGHT OF WAY LINE OF ARKANSAS STATE HIGHWAY #183; THENCE NORTH 21 DEG. 55 MIN. WEST, ALONG SAID RIGHT OF WAY LINE, 68.3 FEET; THENCE SOUTH 77 DEG. 14 MIN. 10 SEC. WEST, 198.19 FEET; THENCE SOUTH 00 DEG. 23 MIN. 30 SEC. WEST, 74.43 FEET; THENCE NORTH 87 DEG. 57 MIN. EAST, 100.0 FEET TO THE POINT OF BEGINNING.

AND

THAT PART OF THE SOUTHWEST QUARTER OF THE SOUTHEAST QUARTER OF SECTION 27, TOWNSHIP 1 SOUTH, RANGE 14 WEST, SALINE COUNTY, ARKANSAS, DESCRIBED AS FOLLOWS: COMMENCING AT THE SOUTHWEST CORNER OF SAID SOUTHWEST QUARTER OF THE SOUTHEAST QUARTER; THENCE NORTH 02°57'47" EAST ALONG THE WEST LINE THEREOF A DISTANCE OF 156.38 FEET TO THE POINT OF BEGINNING; THENCE NORTH 02°57'47" EAST CONTINUING ALONG SAID WEST LINE A DISTANCE OF 123.19 FEET; THENCE NORTH 89°11'41" EAST LEAVING SAID WEST LINE A DISTANCE OF 38.21 FEET; THENCE SOUTH 05°05'25" WEST A DISTANCE OF 73.89 FEET; THENCE SOUTH 86°56'40" EAST A DISTANCE OF 99.94 FEET; THENCE SOUTH 72°03'13" WEST A DISTANCE OF 144.86 FEET TO THE POINT OF BEGINNING. CONTAINING 0.14 ACRES, MORE OR LESS.

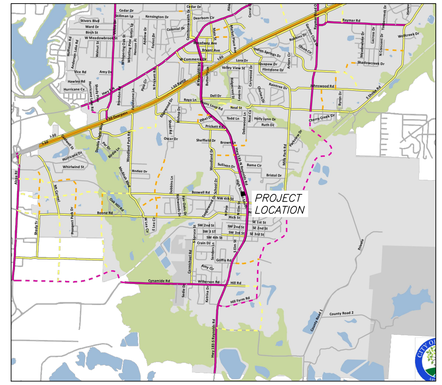
LANDSCAPE NOTES:

1) LANDSCAPING SHALL MEET CITY OF BRYANT LANDSCAPING REQUIREMENTS.
2) SPACING FOR SHRUBS IN BEDS AS NOTED.
3) ALL SHRUB MATERIAL MUST BE EIGHTEEN (18) INCHES IN HEIGHT AT INSTALLATION
4) ALL TREES MUST BE 2" CALIPER AT THE TIME OF INSTALLATION
5) AN AUTOMATIC IRRIGATION SYSTEM SHALL BE PROVIDED FOR ALL NEW LANDSCAPE AREAS. THIS SYSTEM WILL PROVIDE 100% COVERAGE FOR LANDSCAPED AREAS (DESIGNED BY OTHERS).
6) SHRUBS USED FOR SCREENING SHALL BE EVERGREEN IN NATURE AND BE AT LEAST 30 INCHES TALL AT THE TIME OF PLANTING. SPACING SHALL BE CLOSE ENOUGH SO AS TO CREATE A SEAMLESS ROW OF HEDGING.

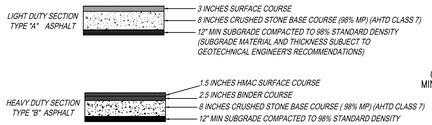
+ THUJA OCCIDENTALIS "EMERALD GREEN ARBORVITAE" (OR APPROVED EQUIVALENT)

VARIANCE REQUEST:
THE REAR SETBACK FROM RESIDENTIAL ZONING BE 25' AS SHOWN ON THIS PLAN.

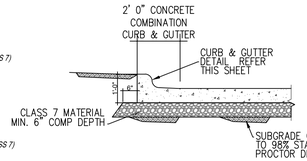
VICINITY MAP
1" = 2000'



PAVEMENT SECTION



* PAVEMENT SECTION SUBJECT TO GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.



TYPICAL CURB & GUTTER DETAIL
N.T.S.

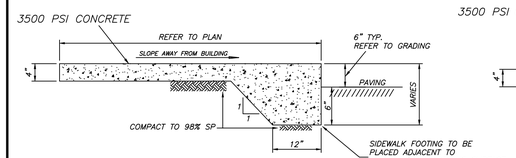
GRAPHIC SCALE

20 0 20 40 60
(IN FEET)
1 inch = 20 ft.

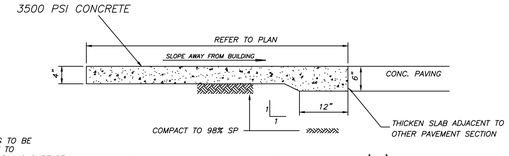
CONCRETE PAVEMENT NOTES:
1. CONCRETE TO BE 4000 PSI, SLUMP 4" +/- 1".
2. INSTALL 1" DIA. X 18" L SMOOTH DOWELS ON 18" CENTERS AT CONTROL JOINTS.
3. SAW CUT CONTROL JOINTS 15' MAX. EACH WAY AT A DEPTH OF 25% (¼" PER 1") OF CONCRETE SECTION THICKNESS.
4. AIR ENTRAINMENT 3-5%.
5. FINISH TO BE MEDIUM BROOM FINISH.
6. INSTALL EXPANSION JOINTS WHERE ABUTTING STRUCTURES AND AT A MAX. OF 200 LF OF PAVING. USE ¾" EXPANSION JOINT MATERIAL WHERE EXPANSION JOINTS ARE SPECIFIED. USE 1" DIA. X 18" L SMOOTH DOWELS ON 18" CENTERS IN EXPANSION JOINTS BETWEEN PAVEMENT SECTIONS.

Legend

- Property Boundary
- Easement/BSL (Labeled)
- RW — Road Right of Way
- T — Telephone
- FO — FO — FO — Fiber Optic Line
- G — Gas Line
- X — Wire / Chainlink Fence
- S — Sanitary Sewer
- W — Water Line
- OHE — Overhead Electric
- UGE — UGE — Underground Powerline
- PROPOSED CONCRETE
- Water Meter Box
- Sanitary Sewer Manhole
- Storm Drainage Manhole
- Telephone Pedistal
- Guy Wire
- Computed Corner
- Found Monument (Labeled)
- Utility Pole
- Gas Meter
- * APPROVED ADA SIDEWALK RAMP. REFER TO NOTES.

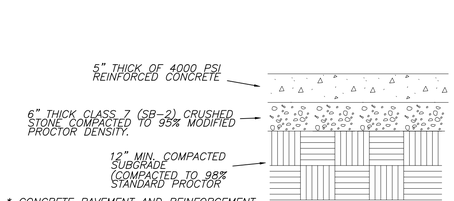


CONC. SIDEWALK - TYPE 'A'
NTS



CONC. SIDEWALK - TYPE 'B'
NTS

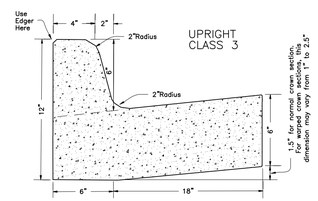
* NOTES:
1. PORTLAND CEMENT CONCRETE 3500 PSI. MIN. MIX AS PER GEOTECHICAL ENGINEER'S RECOMMENDATIONS.
2. USE WT.4 X WT.4 (6" X 6") WWF SHEET REINFORCEMENT. CUT ⅓ THE WIRES AT CONTROL JOINTS.
3. CONCRETE TO CONTAIN 3-5% AIR ENTRAINMENT.
4. FINISH TO BE LIGHT BROOM FINISH.
5. DETECTABLE WARNING DEVICES TO BE INSTALLED AT RAMPS PER ADA REQUIREMENTS.
6. MAX CROSS SLOPE 2%. MAX LONGITUDINAL SLOPE ON RAMPS 1:12 MAX. LONGITUDINAL SLOPE 5% ON SIDEWALKS.
7. CONTROL JOINTS PER CONC. WALK CONTROL JOINT DETAIL.



* CONCRETE PAVEMENT AND REINFORCEMENT TO BE VERIFIED PER ACI 330R-08-SPEC AND SUBJECT TO GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.

CONCRETE PAVEMENT SECTION
NTS

TYPICAL CURB SECTION



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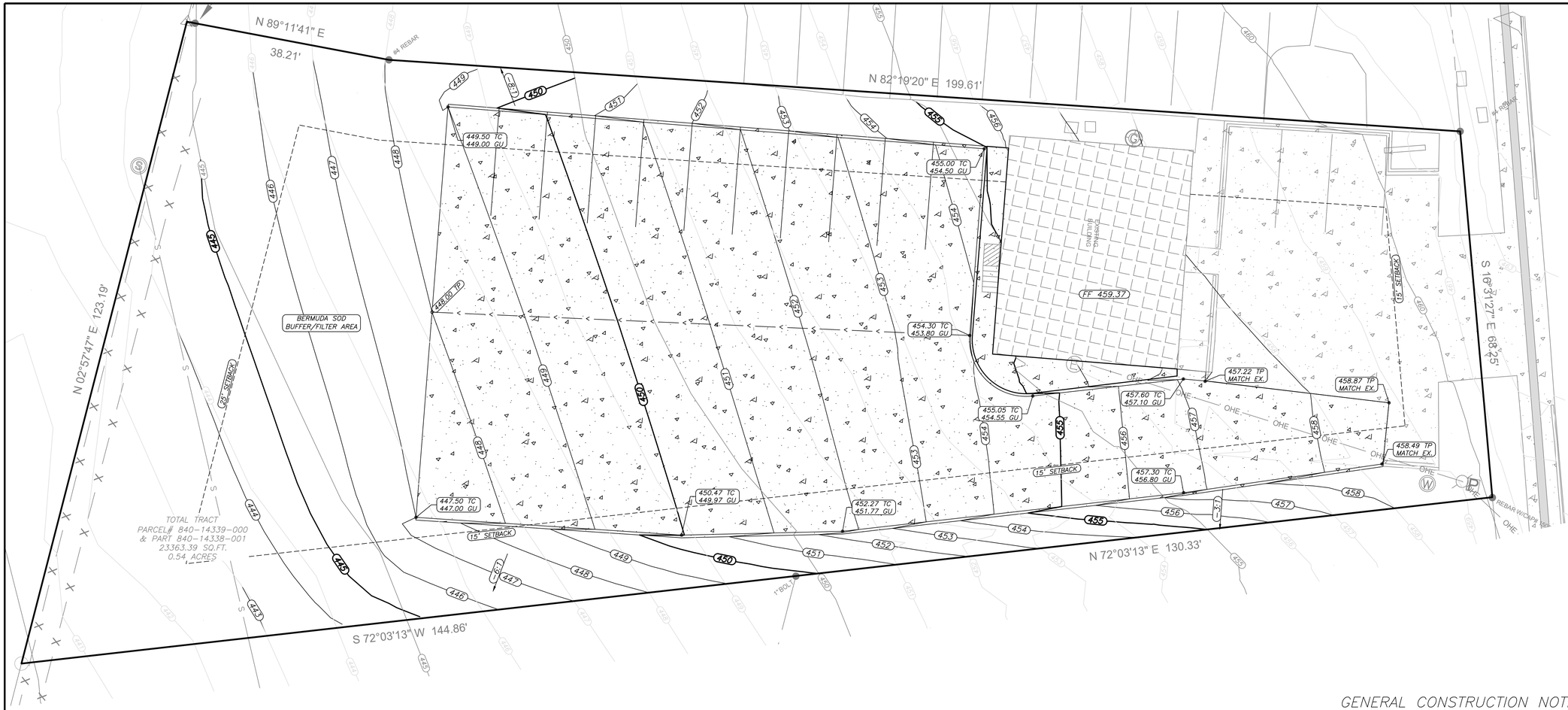

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BRYANT, AR 72022

OWNER OF RECORD
LM SOUTH INVESTMENTS LLC
515 N. REYNOLDS ROAD
BRYANT AR 72022
SOURCE OF TITLE: 2022-002110

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(IN FEET)
1 inch = 10 ft.

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- 422.00 TC — TOP OF CURB ELEVATION
- 422.00 GU — CURB GUTTER ELEVATION
- 422.00 TP — TOP OF PAVEMENT ELEVATION
- 422.00 SW — TOP OF SIDWALK
- PROPOSED CONTOUR
- EXISTING CONTOUR

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E. PRIOR TO INSTALLATION OF ANY UTILITIES, THE CONTRACTOR IS TO EXCAVATE, VERIFY, AND CALCULATE ALL CROSSINGS AND INFORM ANY IMPACTED UTILITY OWNERS OF ANY CONFLICTS PRIOR TO CONSTRUCTION.

F. CONSTRUCTION SHALL NOT START ON ANY WATER UTILITY TIE-INS UNTIL APPROVAL IS GIVEN BY CITY OF BRYANT. CONTRACTOR SHALL NOT OPERATE ANY VALVE, HYDRANT, OR WATER UTILITY APPURTENANCE NOR SHALL HE ATTACH TO OR TAP ANY WATER UTILITY MAIN WITHOUT APPROVAL FROM BRYANT. THE CONTRACTOR SHALL BEAR THE COST AND CONSEQUENCE OF ANY DISRUPTION OF UTILITY OPERATION CAUSED BY CONSTRUCTION.

G. FIBER OPTIC CABLE ON AND/OR ADJACENT TO THIS SITE WERE NOT LOCATED BY THE SURVEY AND ARE NOT SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ANY FIBER OPTIC CABLES AT TO NEAR THIS SITE AND TAKE ALL NECESSARY AND REQUIRED PRECAUTIONS TO PROTECT ANY EXISTING FIBER OPTIC CABLES. CONTRACTORS SHALL COORDINATE ALL EFFORTS WITH OWNER OF FIBER OPTIC CABLES OR THEIR DESIGNATED REPRESENTATIVE.

H. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING "ONECALL" SERVICE TO MARK ALL UTILITIES PRIOR TO ANY DEMOLITION, EARTHWORK, OR UTILITY WORK ON THIS SITE.

I. ANY LAND CLEARING, CONSTRUCTION, OR DEVELOPMENT INVOLVING THE MOVEMENT OF MATERIALS, UTILITIES OR GROUND EXCAVATION SHALL BE IN ACCORDANCE WITH THE STORMWATER POLLUTION PREVENTION PLAN.

J. THE APPROXIMATE LOCATION OF KNOWN SURFACE AND SUBSURFACE STRUCTURES, PIPES, POWER, GAS, PHONE, ETC. ARE SHOWN ON THE DESIGN DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE EXACT LOCATION OF THE AFOREMENTIONED ITEMS, SHOWN AND NOT SHOWN.

K. CONTRACTOR TO ADHERE TO CURRENT OSHA REGULATIONS, INCLUDING EXCAVATION & TRENCH SAFETY.

L. CONTRACTOR SHALL CONTACT THE CITY OF BRYANT STREET DEPARTMENT IN REGARD TO MAINTENANCE OF TRAFFIC PRIOR TO COMMENCEMENT OF WORK WITHIN STREET RIGHT OF WAY.

M. CONTRACTOR SHALL CONTACT CITY OF BRYANT PRIOR TO COMMENCEMENT OF UTILITY CONSTRUCTION OR TIE-INS.

RICHARDSON ENGINEERING
Planning • Engineering • Development Consulting
325 W. SOUTH STREET, BENTON, AR 72015 (501)315-7225

CERTIFICATE OF AUTHORIZATION
Richardson Engineering, PLLC
No. 2519
ARKANSAS

GRADING PLAN
STATE FARM - JAMEY SOUTH
PROPOSED PARKING LOT
515 N. REYNOLDS ROAD
BRYANT, ARKANSAS

Prepared For: JAMEY SOUTH
515 N. REYNOLDS ROAD
BRYANT, AR 72022

No.	Revisions	Date
1	BRYANT DRC COMMENTS	5/16/2025
2	REMOVED BUILDING	7/30/2025
3	REMOVED DETENTION BASIN	8/13/2025

PROJECT #: 025-007
Date: 4/22/2025 REV: 8/13/2025
Scale: 1" = 10'
Sheet: 5 of 5

GAS METER	STORM DRAIN MANHOLE		ASPHALT
WATER METER	SEWER CLEANOUT		CONCRETE
GUY WIRE	N NORTH		
POWER/UTILITY POLE	S SOUTH		
TELEPHONE PEDESTAL	E EAST		
SEWER MANHOLE	W WEST		
WATER VALVE	(M) AS MEASURED		SET 1/2" REBAR w/ CAP #1853
FIRE HYDRANT	(D) PER DEED		SET COTTON-PICKER SPINDLE
SIGNS	(R) RECORDED		FOUND MONUMENT (DESC. NOTED)
LIGHT POLE	R/W RIGHT-OF-WAY		COMPUTED CORNER (NOT SET)
TELEPHONE MANHOLE	L.A. LANDSCAPED AREA		CORRESPONDS TO DRAWING NOTE
SS SANITARY SEWER LINE	CR4 CAPPED 1/2" REBAR		
W WATER LINE	CONC. CONCRETE		
STORM SEWER PIPE	P.O.C. POINT OF COMMENCEMENT		
ROADWAY CENTERLINE	P.O.B. POINT OF BEGINNING		
UTILITY EASEMENT	CMP CORRUGATED METAL PIPE		
BUILDING SETBACK LINE	RCP REINFORCED CONCRETE PIPE		
R/W ROADWAY RIGHT-OF-WAY	ESMT EASEMENT		
OHE OVERHEAD ELECTRIC LINES	HDPE HIGH DENSITY POLYETHYLENE		
UGT UNDERGROUND TELEPHONE	SUBD SUBDIVISION		
G UNDERGROUND GAS	FDC FIRE DEPARTMENT CONNECTION		
F.O. UNDERGROUND FIBER OPTIC	CPS COTTON PICKER SPINDLE		
	P5 5/8" PIPE		

GENERAL CONSTRUCTION NOTES

A. THE CONTRACTOR WILL BE HELD SOLELY RESPONSIBLE FOR DAMAGES OCCURRING TO ANY PROPERTY DURING THE CONSTRUCTION OF THIS PROJECT. SAID CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PREVENT PROPERTY DAMAGE.

B. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL SOLELY AND COMPLETELY BE RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND WILL NOT BE LIMITED TO NORMAL WORKING HOURS.

C. THE DUTY OF THE LOCAL UTILITY PROVIDER TO CONDUCT CONSTRUCTION INSPECTION REVIEWS OF THE CONTRACTOR'S PERFORMANCE IS NOT AN INSPECTION OR REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES IN, ON, OR NEAR THE CONSTRUCTION SITE.

D. ALL WATER AND SEWER IMPROVEMENTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST REVISION TO THE LOCAL PROVIDER'S WATER AND WASTEWATER (SANITARY SEWER) STANDARD SPECIFICATIONS.

E. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES AND, WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANY AT LEAST 48 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF ALL UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH PROPOSED IMPROVEMENTS SHOWN ON THE PLAN.

F. CONTRACTOR IS TO REMOVE AND DISPOSE OF ALL DEBRIS, RUBBISH, AND OTHER MATERIALS RESULTING FROM PREVIOUS AND CURRENT DEMOLITION OPERATIONS. DISPOSAL WILL BE IN ACCORDANCE WITH ALL LOCAL, STATE AND/OR FEDERAL REGULATIONS GOVERNING SUCH OPERATIONS.

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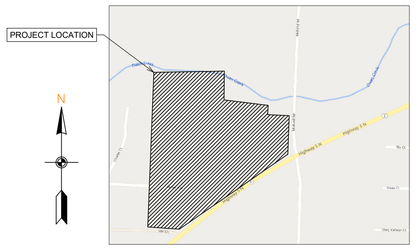
NEW BEGINNINGS

HIGHWAY 5
BRYANT, AR

Sheet List Table	
Sheet Number	Sheet Title
C1.0	COVER SHEET
C1.1	OVERALL SITE PLAN
C1.2	ENLARGED SITE PLAN
C1.3	SITE DETAILS
C1.4	GRADING PLAN
C1.5	ENLARGED GRADING PLAN
C1.6	UTILITY PLAN
C1.7	UTILITY PROFILES
C1.8	UTILITY DETAILS I
C1.9	UTILITY DETAILS II
C1.10	PRE-DEV DRAINAGE
C1.11	POST-DEV DRAINAGE
C1.12	LANDSCAPE PLAN
C1.13	SWPPP



Vicinity Map

SCALE 1" = 500'

811 Know what's **below.** **Call** before you dig.

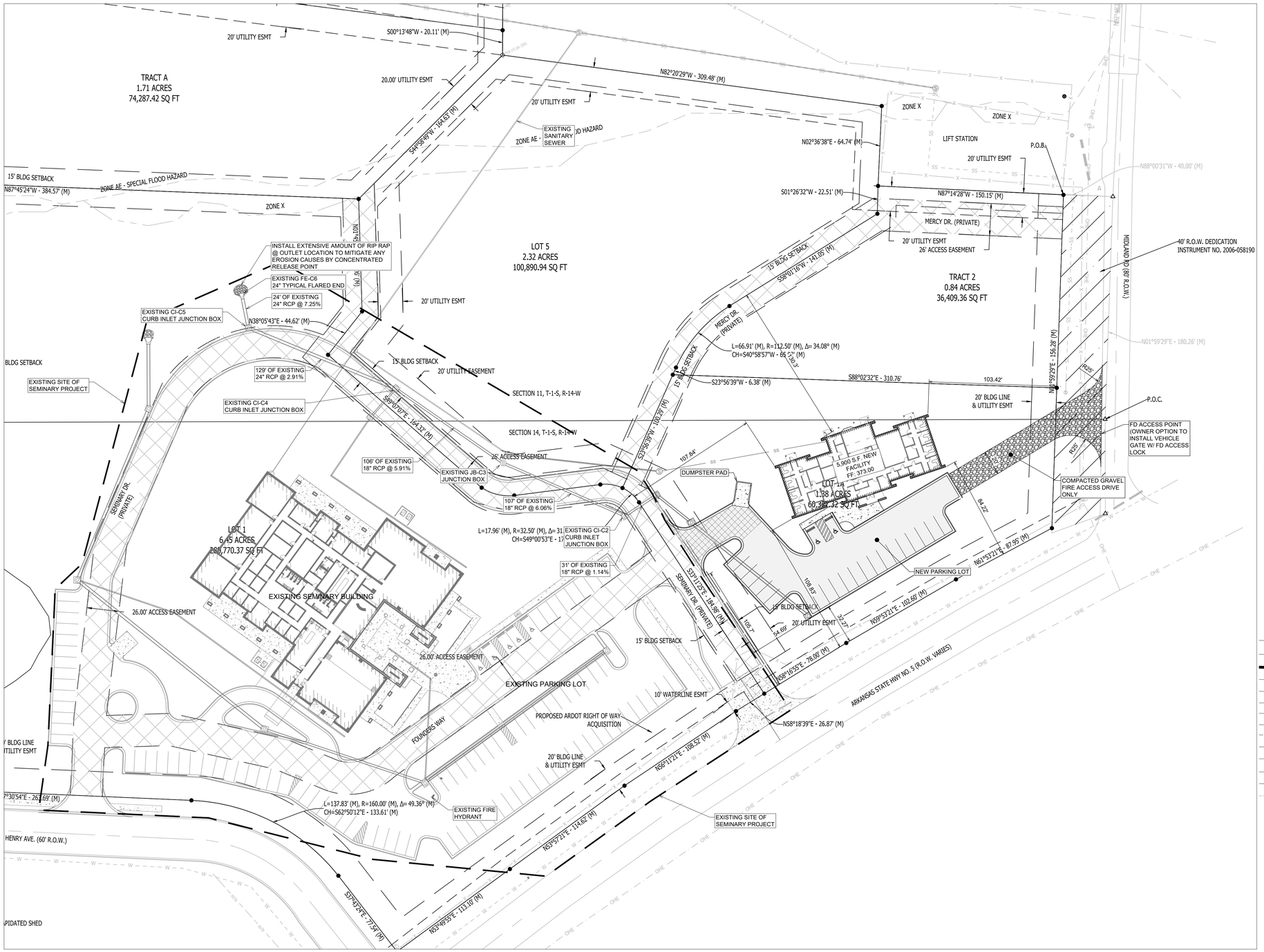
DEVIATIONS/VARIANCES

CERTIFICATE OF AUTHORIZATION PHILLIP LEWIS ENGINEERING, INC. No. 566 ARKANSAS-ENGINEER

STATE OF ARKANSAS REGISTERED PROFESSIONAL ENGINEER No. 9540 PHILLIP A. LEWIS

PHILLIP LEWIS ENGINEERING, INC.
Structural + Civil Consultants

23620 Interstate 30 | Bryant, Arkansas
PH: 501-350-9840



OVERALL SITE PLAN

SCALE 1" = 40'

LEGEND

- LIGHT DUTY HMAC ASPHALT SURFACE COURSE
- MEDIUM DUTY HMAC ASPHALT SURFACE COURSE
- MEDIUM DUTY HMAC ASPHALT SURFACE COURSE

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LEGEND OF SYMBOLS & ABBREVIATIONS

GAS METER		STORM DRAIN MANHOLE	
WATER METER		SEWER CLEANOUT	
GUY WIRE	N	NORTH	
POWER/UTILITY POLE	S	SOUTH	
TELEPHONE PEDESTAL	E	EAST	
SEWER MANHOLE	W	WEST	
WATER VALVE	(M)	AS MEASURED	SET 1/2" REBAR w/ CAP #1853
FIRE HYDRANT	(D)	PER DEED	SET COTTON-PICKER SPINDLE
SIGNS	(R)	RECORDED	FOUND MONUMENT (DESC. NOTED)
LIGHT POLE	R/W	RIGHT-OF-WAY	COMPUTED CORNER (NOT SET)
TELEPHONE MANHOLE	L.A.	LANDSCAPED AREA	
GAS VALVE	CR4	CAPPED 1/2" REBAR	
SANITARY SEWER LINE	CONC.	CONCRETE	
WATER LINE	P.O.C.	POINT OF COMMENCEMENT	
STORM SEWER PIPE	P.O.B.	POINT OF BEGINNING	
ROADWAY CENTERLINE	CMP	CORRUGATED METAL PIPE	
UTILITY EASEMENT	RCP	REINFORCED CONCRETE PIPE	
BUILDING SETBACK LINE	ESMT	EASEMENT	
ROADWAY RIGHT-OF-WAY	HDPE	HIGH DENSITY POLYETHYLENE	
OVERHEAD ELECTRIC LINES	SUBD	SUBDIVISION	
UNDERGROUND TELEPHONE	FDC	FIRE DEPARTMENT CONNECTION	
UNDERGROUND GAS	CPS	COTTON PICKER SPINDLE	
UNDERGROUND FIBER OPTIC	P5	5/8" PIPE	
FENCE	R5	5/8" REBAR	
STEEL GUARD RAIL	R4	1/2" REBAR	
SURFACE CONTOUR LINE & ELEVATION	PK	SURVEY NAIL	
	PB	TELEPHONE PULL BOX	

PHILLIP LEWIS ENGINEERING, INC.
Structural + Civil Consultants
23620 Interstate 30 | Bryant, Arkansas
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REVISION:

NEW BEGINNINGS
HIGHWAY 5
BRYANT, ARKANSAS

STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 9540
PHILLIP LEWIS

CERTIFICATE OF AUTHORIZATION
PHILLIP LEWIS ENGINEERING, INC.
No. 566
ARKANSAS—ENGINEER

N

0 40'

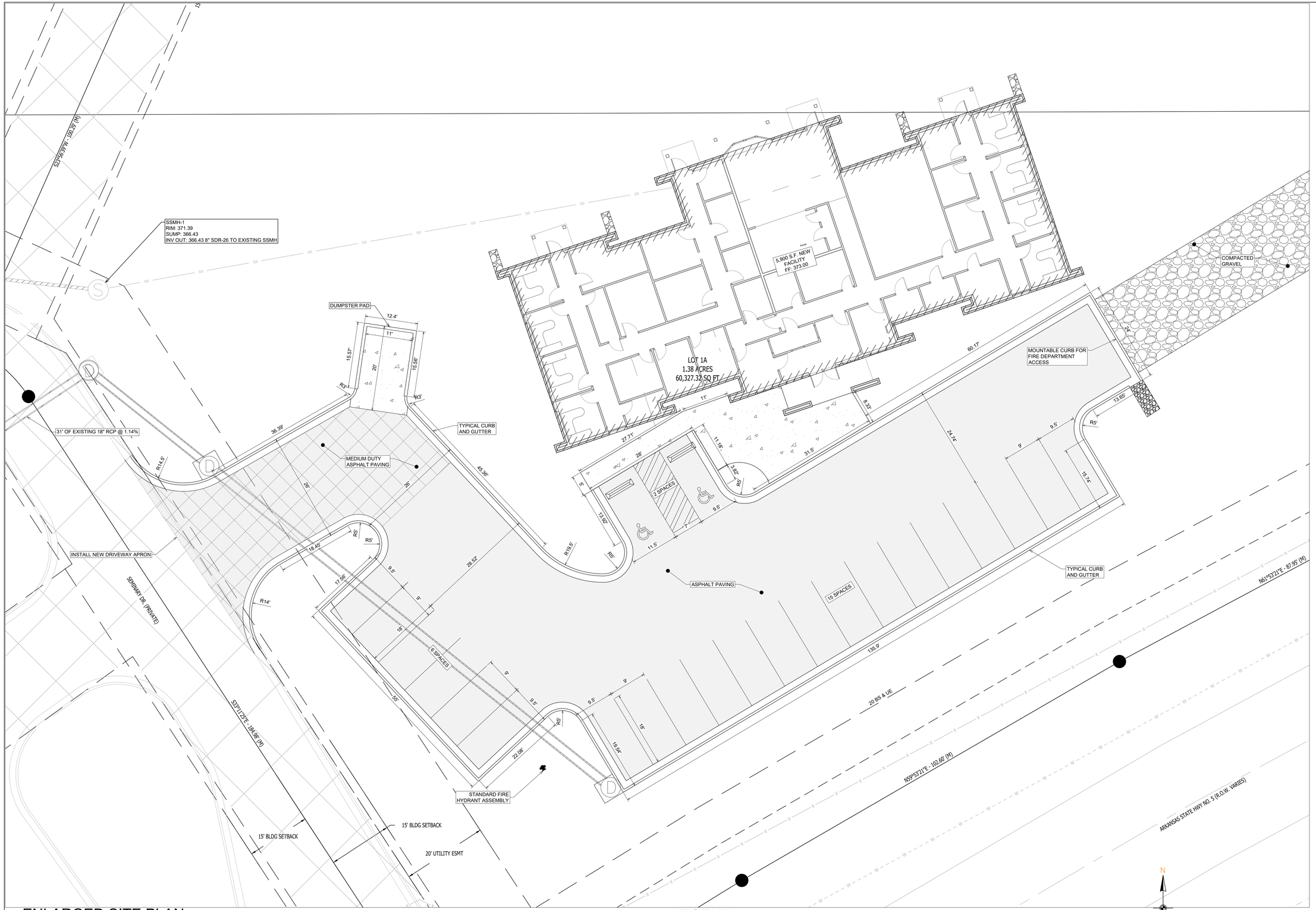
1" = 40'-0"

PROJECT NUMBER:

SHEET ISSUE DATE:
08-06-2025

PAGE TITLE:
OVERALL SITE PLAN

SHEET NUMBER:
C1.1



ENLARGED SITE PLAN

SCALE 1" = 10'

1. TOTAL NEW DEVELOPMENT AREA = (+/-) 1.8 ACRES
2. PROPERTY IS ZONED C-2
3. 23 PARKING SPACES PROVIDED INCLUDING 2 ADA ACCESSIBLE PARKING SPACES
4. ALL DIMENSIONS ARE TO THE BACK OF CURB AND/OR EDGE OF PAVEMENT
5. DAMAGE TO PUBLIC AND PRIVATE PROPERTY DUE TO HAULING OPERATIONS OR OPERATIONS OF CONSTRUCTION RELATED EQUIPMENT FROM A CONSTRUCTION SITE SHALL BE REPAIRED BY THE THE RESPONSIBLE PARTY PRIOR TO THE ISSUANCE OF A CERTIFICATE OF OCCUPANCY.
6. REPAIR, REPLACE, OR EXTEND EXISTING DAMAGED OR MISSING CURB AND GUTTER, SIDEWALK OR RAMPS WITHIN THE PUBLIC RIGHT OF WAY.
7. ALL SIGNAGE, PAVEMENT MARKING AND PARKING LOT STRIPING SHALL CONFORM TO REQUIREMENTS GIVEN IN MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD). MUTCD REQUIRES THAT PARKING SPACES BE MARKED IN WHITE.

LEGEND

- LIGHT DUTY HMAC ASPHALT SURFACE COURSE
- MEDIUM DUTY HMAC ASPHALT SURFACE COURSE
- COMPACTED GRAVEL
- CONCRETE PAVING OR SIDEWALK

N

0 10'

1" = 10'-0"

PHILLIP LEWIS ENGINEERING, INC.
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REVISION:

NEW BEGININGS
HIGHWAY 5
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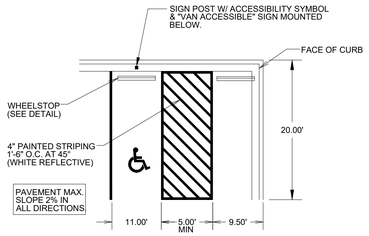
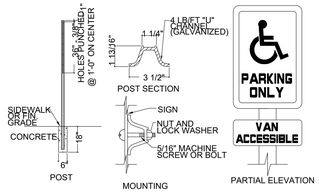
PAGE TITLE:
ENLARGED SITE PLAN

SHEET NUMBER:
C1.2



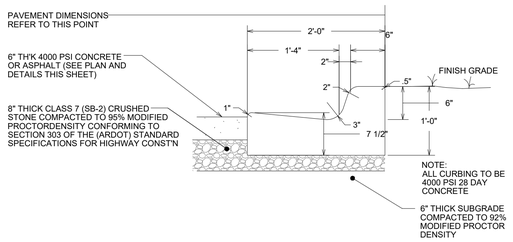
TYPICAL ACCESSIBLE PARKING STALLS



HANDICAP SIGN DETAIL

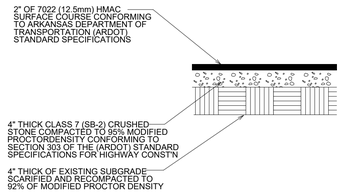
NOT TO SCALE

NOTE:
HANDICAP SIGNAGE SHALL BE IN STRICT COMPLIANCE WITH CURRENT FEDERAL AND LOCAL LAW REQUIREMENTS



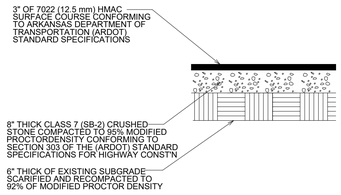
2'-0" CONCRETE CURB & GUTTER

NOT TO SCALE



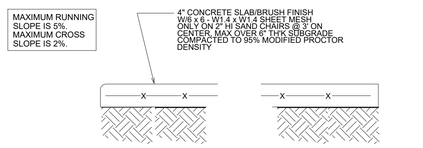
LIGHT DUTY HMAC ASPHALT SURFACE COURSE

NOT TO SCALE



MEDIUM DUTY HMAC ASPHALT SURFACE COURSE

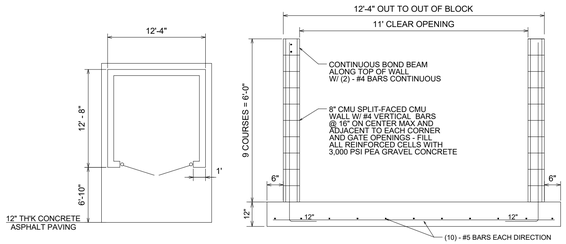
NOT TO SCALE



CONCRETE WALK SECTION

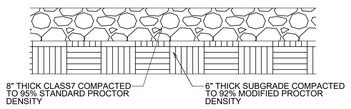
NOT TO SCALE

NOTE:
ALL SIDEWALK JOINTS TO BE STEEL TROWELED. ALL JOINT EDGES AND SHALL BE SPACED AT 5 FEET ON CENTER MAXIMUM IN ALL DIRECTIONS AND SHALLBE STEEL TROWELED ON A RADIUS NOT TO EXCEED ONE-HALF INCH.



DUMPSTER PAD/ENCLOSURE DETAIL

NOT TO SCALE



COMPACTED GRAVEL PAVEMENT SECTION

NOT TO SCALE

PHILLIP LEWIS ENGINEERING, INC.
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REVISION:

NEW BEGININGS
HIGHWAY 5
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ARKANSAS ENGINEER

STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 9540
PHILLIP A. LEWIS

PROJECT NUMBER:

SHEET ISSUE DATE:
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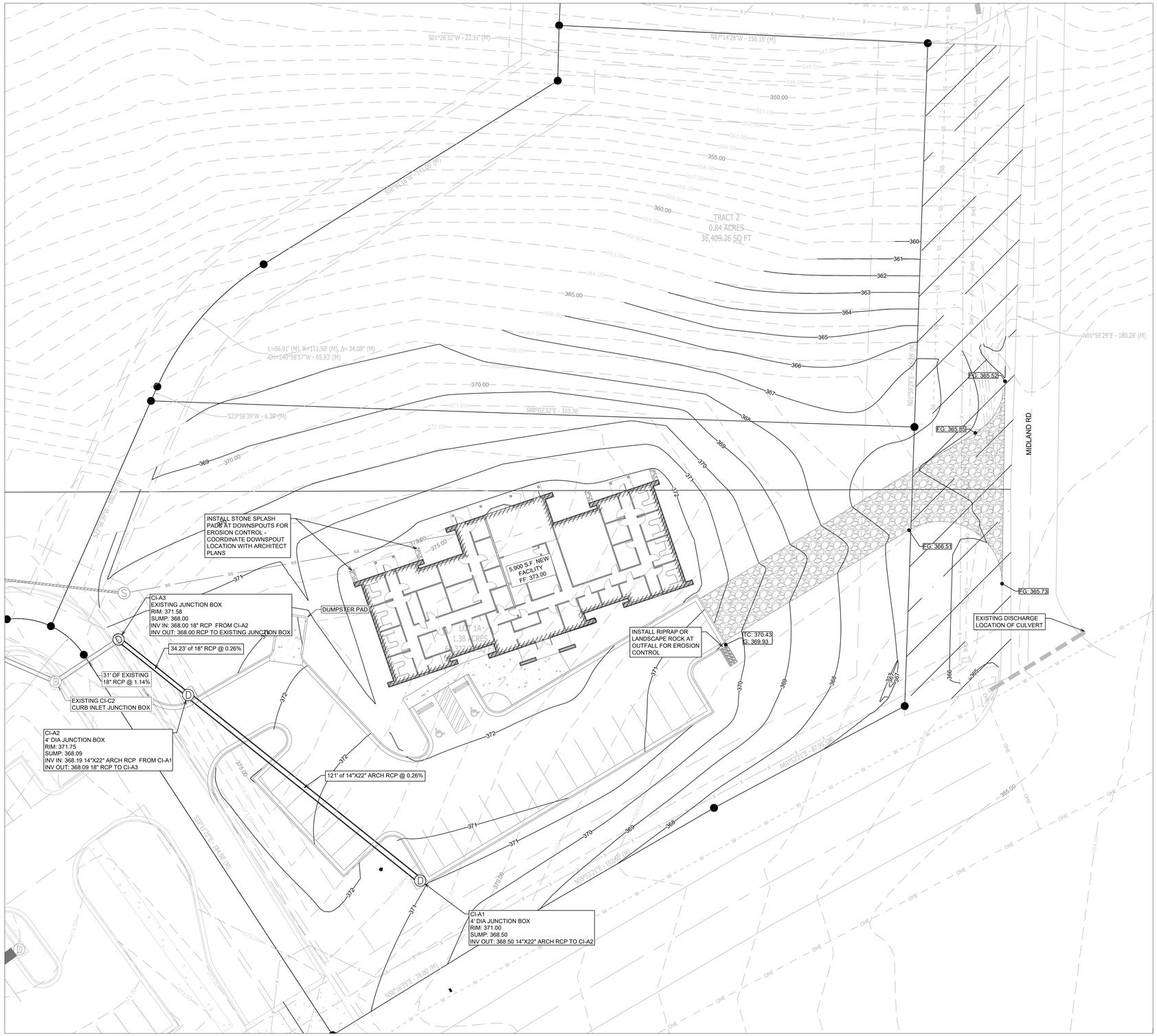
PAGE TITLE:
SITE DETAILS

SHEET NUMBER:
C1.3



OVERALL GRADING PLAN

SCALE 1" = 20'

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N

0 20'

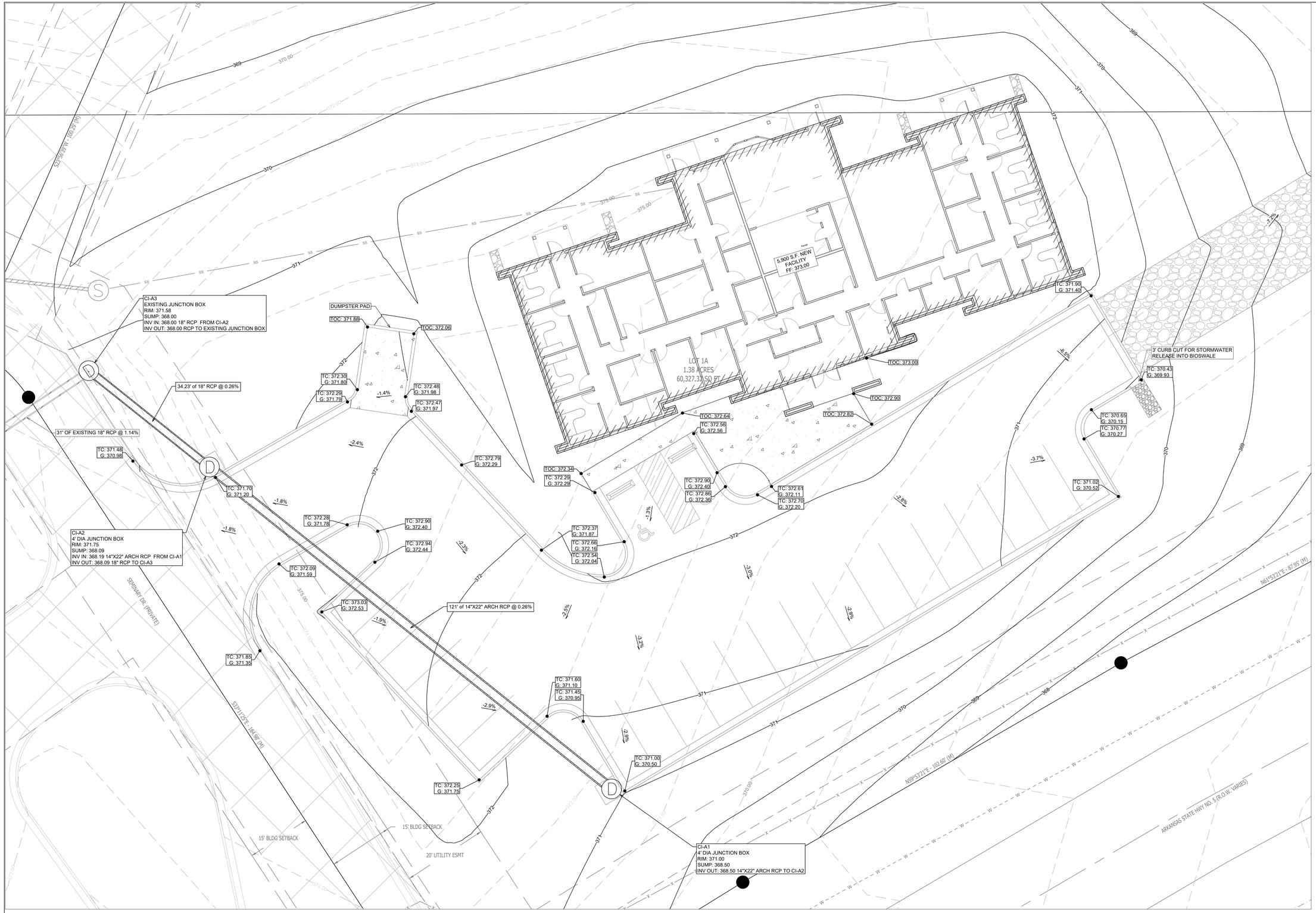
1" = 20'-0"

PROJECT NUMBER:

SHEET ISSUE DATE:
08-06-2025

PAGE TITLE:
GRADING PLAN

SHEET NUMBER:
C1.4



ENLARGED GRADING PLAN

SCALE 1" = 10'

TC = TOP OF CURB ELEVATION
G = GUTTER ELEVATION
TOC = TOP OF CONCRETE ELEVATION
FG = FINAL GRADE ELEVATION
TP = TOP OF PAVEMENT ELEVATION
EG = EXISTING GRADE ELEVATION

N

0 10'

1" = 10'-0"

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PAGE TITLE:
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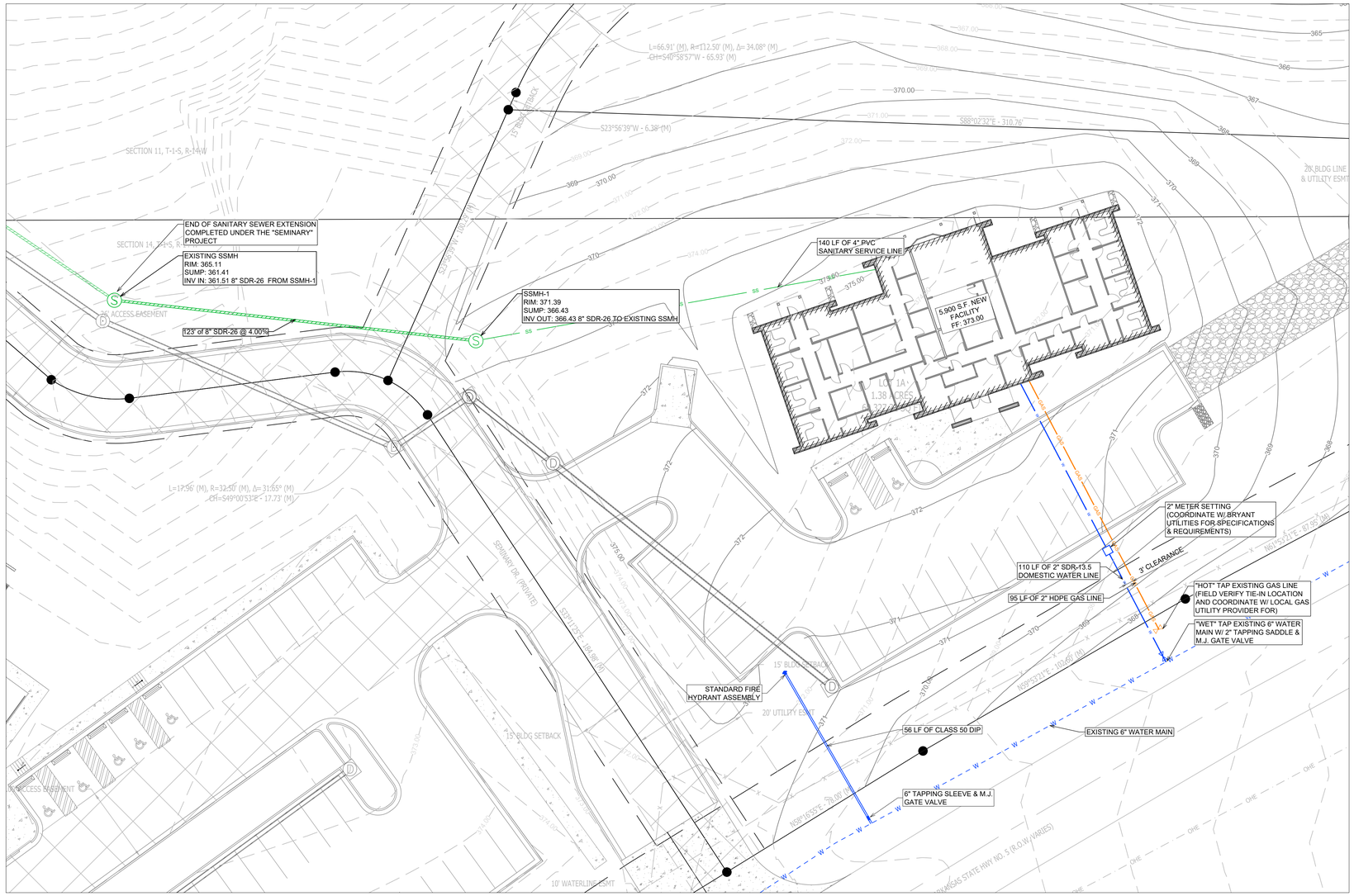

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UTILITY PLAN

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B. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL SOLELY AND COMPLETELY BE RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND WILL NOT BE LIMITED TO NORMAL WORKING HOURS.

THE DUTY OF BRYANT TO CONDUCT CONSTRUCTION INSPECTION REVIEWS OF THE CONTRACTOR'S PERFORMANCE IS NOT AN INSPECTION OR REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES IN, ON, OR NEAR THE CONSTRUCTION SITE.

C. ALL WATER AND SEWER IMPROVEMENTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST REVISION TO THE CITY OF BRYANT'S WATER AND WASTEWATER (SANITARY SEWER) STANDARD SPECIFICATIONS.

D. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES AND, WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANY AT LEAST 48 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF ALL UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH PROPOSED IMPROVEMENTS SHOWN ON THE PLAN.

E. CONTRACTOR IS TO REMOVE AND DISPOSE OF ALL DEBRIS, RUBBISH, AND OTHER MATERIALS RESULTING FROM PREVIOUS AND CURRENT DEMOLITION OPERATIONS. DISPOSAL WILL BE IN ACCORDANCE WITH ALL LOCAL, STATE AND/OR FEDERAL REGULATIONS GOVERNING SUCH OPERATIONS.

F. PRIOR TO INSTALLATION OF ANY UTILITIES, THE CONTRACTOR IS TO EXCAVATE, VERIFY AND CALCULATE ALL CROSSINGS AND INFORM ANY AND ALL UTILITIES OF ANY CONFLICTS PRIOR TO CONSTRUCTION.

G. CONSTRUCTION SHALL NOT START ON ANY WATER UTILITY TIE-INS UNTIL APPROVAL IS GIVEN BY BRYANT WATER. SAID CONTRACTOR SHALL NOT OPERATE ANY VALVE, HYDRANT, OR WATER UTILITY APPURTENANCE NOR SHALL HE ATTACH TO OR TAP ANY WATER UTILITY MAIN WITHOUT APPROVAL. THE CONTRACTOR SHALL BEAR THE COST AND CONSEQUENCE OF ANY DISRUPTION OF UTILITY OPERATION CAUSED BY CONSTRUCTION.

H. FIBER OPTIC CABLE ON AND/OR ADJACENT TO THIS SITE WERE NOT LOCATED BY THE SURVEY AND ARE NOT SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ANY FIBER OPTIC CABLES ASSOCIATED WITH THIS SITE AND TAKE ALL NECESSARY AND REQUIRED PRECAUTIONS TO PROTECT ANY EXISTING FIBER OPTIC CABLES. CONTRACTORS SHALL COORDINATE ALL EFFORTS WITH OWNER OF FIBER OPTIC CABLES OR THEIR DESIGNATED REPRESENTATIVE.

J. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING "ONECALL" SERVICE TO MARK ALL UTILITIES PRIOR TO ANY DEMOLITION, EARTHWORK, OR UTILITY WORK ON THIS SITE.

PHILLIP LEWIS ENGINEERING, INC.
Structural + Civil Consultants
23620 Interstate 30 | Bryant, Arkansas
PH: 501-350-9840

REVISION:

NEW BEGININGS
HIGHWAY 5
BRYANT, ARKANSAS

STATE OF ARKANSAS
PHILLIP LEWIS ENGINEERING, INC.
No. 566
REGISTERED PROFESSIONAL ENGINEER
No. 9540
PHILLIP A. LEWIS

CERTIFICATE OF AUTHORIZATION
ARKANSAS-ENGINEER

Know what's **below.**
Call before you dig.

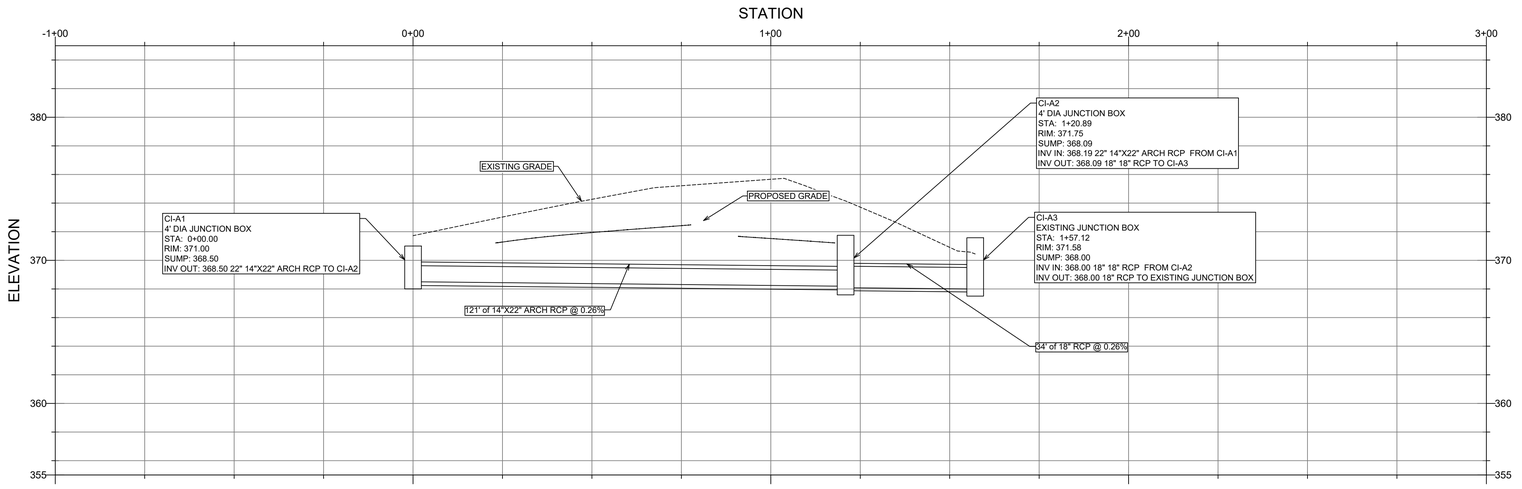
PROJECT NUMBER:

SHEET ISSUE DATE:
08-06-2025

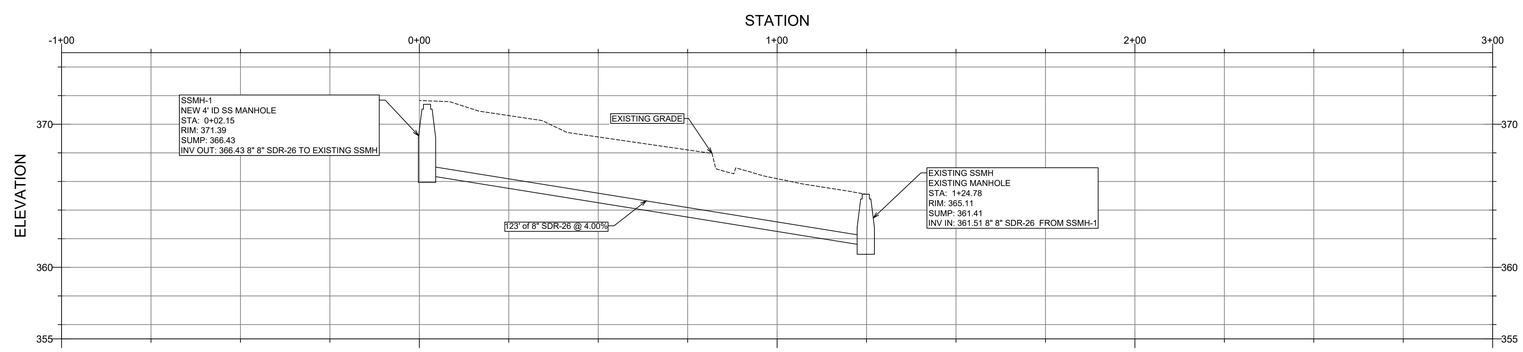
PAGE TITLE:
UTILITY PLAN

SHEET NUMBER:
C1.6

STORM SEWER



SANITARY SEWER



PHILLIP LEWIS ENGINEERING, INC.
Structural + Civil Consultants
23620 Interstate 30 | Bryant, Arkansas
PH: 501-350-9840

REVISION:

NEW BEGINGS
HIGHWAY 5
BRYANT, ARKANSAS

CERTIFICATE OF AUTHORIZATION
PHILLIP LEWIS ENGINEERING, INC.
No. 566
ARKANSAS ENGINEER

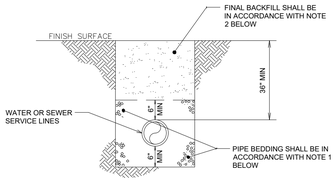
STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 9540
PHILLIP A. LEWIS

PROJECT NUMBER:

SHEET ISSUE DATE:
C1.7

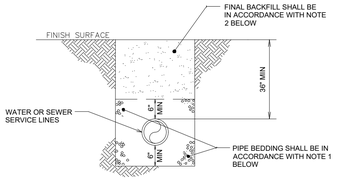
PAGE TITLE:
UTILITY PROFILES

SHEET NUMBER:
C1.7



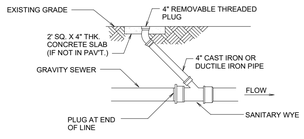
GAS LINE BEDDING DETAIL

NOT TO SCALE



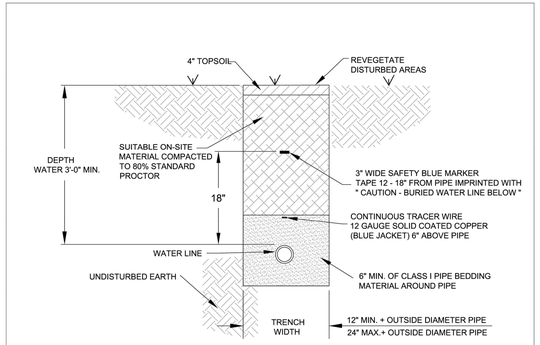
WATER AND SEWER LINES BEDDING DETAIL

NOT TO SCALE



THROUGH FLOW CLEANOUT

NOT TO SCALE



NOTES:

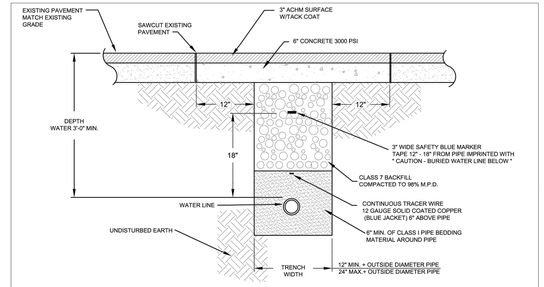
1. ALL VALVES, BENDS, ETC. SHALL BE RESTRAINED.
2. THE CONTRACTOR SHALL PROVIDE ALL ITEMS NECESSARY TO CONNECT WITH ANY PART OF THE EXISTING WATER SYSTEM THAT WILL REMAIN IN ORDER TO ESTABLISH A SATISFACTORY AND ACCEPTABLE WATER SYSTEM.
3. CONTRACTOR TO CONSTRUCT ALL TRENCH EXCAVATION IN ACCORDANCE WITH ALL OSHA REGULATIONS (29 CFR CH.XVII, SUBPART B)
4. TRENCH SHALL BE EXCAVATED BELOW GRADE REQUIRED TO PROVIDE A MINIMUM 36" OF PIPE COVER.
5. MAXIMUM PIPE COVER SHALL BE 60" UNDER NORMAL CONDITIONS. COVER GREATER THAN 60" MAY BE ALLOWED FOR SHORT DISTANCES WHERE REQUIRED BY FIELD CONDITIONS. NO HYDRANT SHALL BE ALLOWED WHERE LINES ARE GREATER THAN 60" DEEP.

WATER MAIN TRENCH

(NON-PAVED AREA)

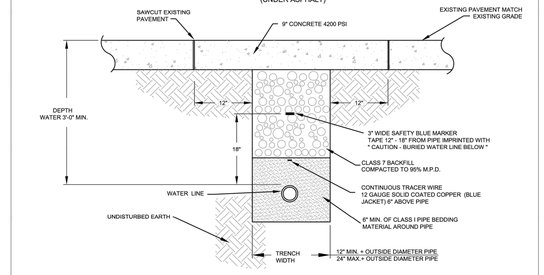
CITY OF BRYANT, AR
WATER UTILITIES
210 S.W. 3rd. STREET
BRYANT, AR
PHONE: (501) 943-0468

TITLE:	WATER DETAILS	DATE: APRIL 2015	SHEET:
DESCRIPTION:	WATER MAIN TRENCH (NON-PAVED AREA)	REVISED ----- ----- ----- -----	W1
DRAWN BY:	CHECKED BY:	FILE: W1-Water Trench (Non-Paved Area).dwg	-----



WATER MAIN TRENCH

(UNDER ASPHALT)



NOTES:

1. ALL VALVES, BENDS, ETC. SHALL BE RESTRAINED.
2. THE CONTRACTOR SHALL PROVIDE ALL ITEMS NECESSARY TO CONNECT WITH ANY PART OF THE EXISTING WATER SYSTEM THAT WILL REMAIN IN ORDER TO ESTABLISH A SATISFACTORY AND ACCEPTABLE WATER SYSTEM.
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WATER MAIN TRENCH

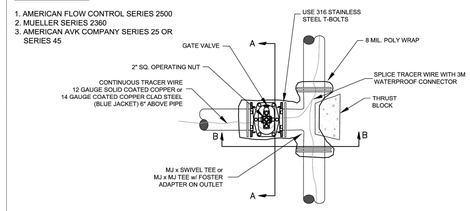
(UNDER CONCRETE)

CITY OF BRYANT, AR
WATER UTILITIES
210 S.W. 3rd. STREET
BRYANT, AR
PHONE: (501) 943-0468

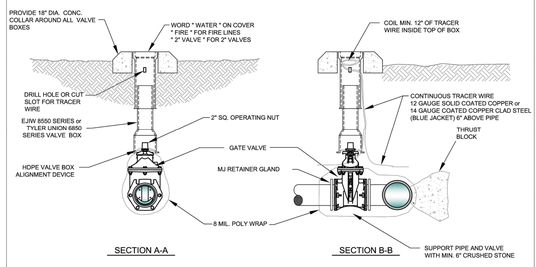
TITLE:	WATER DETAILS	DATE: APRIL 2015	SHEET:
DESCRIPTION:	WATER MAIN TRENCH (UNDER PAVEMENT)	REVISED ----- ----- ----- -----	W2
DRAWN BY:	CHECKED BY:	FILE: W2-Water Trench (Under Pavement).dwg	-----

PERMITTED VALVES

1. AMERICAN FLOW CONTROL SERIES 2500
2. MUELLER SERIES 2360
3. AMERICAN AVK COMPANY SERIES 25 OR SERIES 45



PLAN



SECTION A-A

SECTION B-B

NOTES:

1. ALL VALVES SHALL BE SECURELY ANCHORED TO THE TEE.
2. ALL HARDWARE SHALL BE 316 STAINLESS STEEL.
3. IF DEPTH OF BURY EXCEEDS 4 FT., A VALVE STEM EXTENSION SHALL BE REQUIRED. THE VALVE STEM EXTENSION NUT SHALL BE WITHIN 24-INCHES TO 12-INCHES OF THE FINISHED SURFACE.

GATE VALVE DETAIL

2" - 12"

CITY OF BRYANT, AR
WATER UTILITIES
210 S.W. 3rd. STREET
BRYANT, AR
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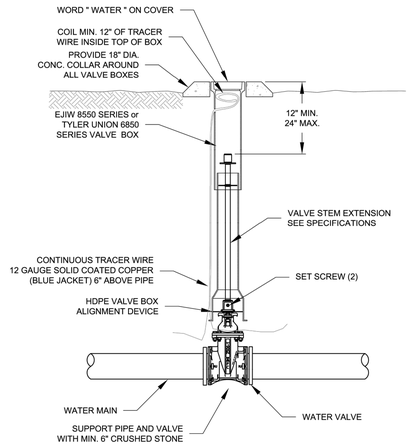
TITLE:	WATER DETAILS	DATE: APRIL 2015	SHEET:
DESCRIPTION:	GATE VALVE	REVISED ----- ----- ----- -----	W4
DRAWN BY:	CHECKED BY:	FILE: W4-Gate Valve.dwg	-----

NOTES:

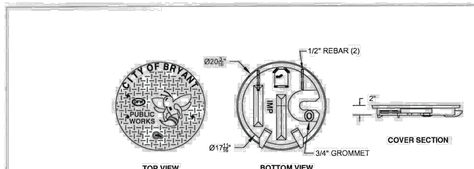
1. OPERATING NUT EXTENSIONS SHALL BE USED WHEN THE TOP OF THE OPERATING NUT IS GREATER THAN 4 FEET FROM THE TOP OF FINISHED SURFACE.
2. THE STEM EXTENSION SHALL BE OF ADEQUATE LENGTH TO REACH FROM THE VALVE OPERATING NUT TO A POINT WITHIN 24-INCHES TO 12-INCHES OF THE FINISHED SURFACE.



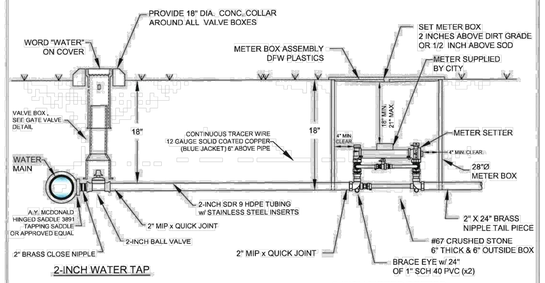
VALVE WITH STEM EXTENSION

CITY OF BRYANT, AR
WATER UTILITIES
210 S.W. 3rd. STREET
BRYANT, AR
PHONE: (501) 943-0468

TITLE:	WATER DETAILS	DATE: APRIL 2015	SHEET:
DESCRIPTION:	VALVE STEM EXTENSION	REVISED ----- ----- ----- -----	W7
DRAWN BY:	CHECKED BY:	FILE: W7-Valve Stem Extension.dwg	-----



WATER METER LID



2-INCH WATER TAP

2-INCH METER SETTER

NOTES:

1. COORDINATE INSTALLATION AND CONFIGURATIONOF ALL 1-1/2" AND LARGER METER SETTINGS WITH THE CITY.
2. ALL METER BOXES SHALL BE INSTALLED IN NON-PAVED AREAS. ANY METER BOX THAT GETS PLACED IN A PAVED AREA SHALL BE RELOCATED AT THE OWNER'S EXPENSE BEFORE A WATER METER WILL BE INSTALLED.
3. SUPPORT METER INSTALLATION WITH 1"x24" SCH40 PVC THROUGH EACH BRACE EYE.

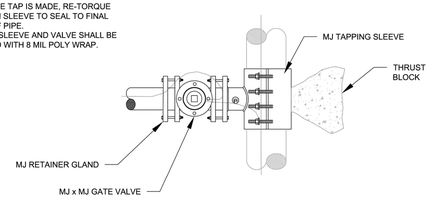
2" WATER SERVICE AND WATER METER

CITY OF BRYANT, AR
WATER UTILITIES
210 S.W. 3rd. STREET
BRYANT, AR
PHONE: (501) 943-0468

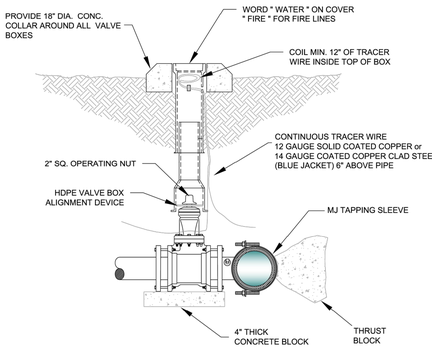
TITLE:	WATER DETAILS	DATE: APRIL 2015	SHEET:
DESCRIPTION:	2" WATER METER	REVISED ----- ----- ----- -----	W12
DRAWN BY:	CHECKED BY:	FILE: W12-2 Inch Water Meter.dwg	-----

NOTES:

1. TAPPING SLEEVE SHALL BE SMITH-BLAIR 662 OR FORD FAST.
2. AFTER THE TAP IS MADE, RE-TORQUE BOLTS ON SLEEVE TO SEAL TO FINAL SHAPE OF PIPE.
3. TAPPING SLEEVE AND VALVE SHALL BE WRAPPED WITH 8 MIL POLY WRAP.



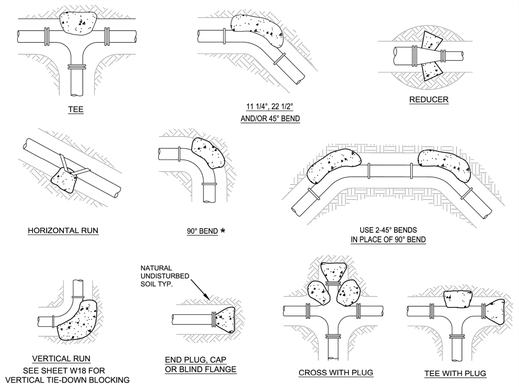
PLAN VIEW



PROFILE VIEW

CITY OF BRYANT, AR
WATER UTILITIES
210 S.W. 3rd. STREET
BRYANT, AR
PHONE: (501) 943-0468

TITLE:	WATER DETAILS	DATE: APRIL 2015	SHEET:
DESCRIPTION:	TAPPING SLEEVE AND VALVE	REVISED ----- ----- ----- -----	W13
DRAWN BY:	CHECKED BY:	FILE: W13-Tapping Sleeve and Valve.dwg	-----



THRUST BLOCK SCHEDULE

(2000 PSF SOIL, 200 PSI WATER PRESSURE, 100 PSI WATER HAMMER)

BEARING AREA OF THRUST BLOCKS IN SQ. FT. (HORIZONTAL BENDS)						VOLUME OF THRUST BLOCK IN CU. FEET (VERTICAL BENDS)						
FITTING SIZE	PLUG TEE OR CAP	BEND ANGLE 90°	45°	22 1/2°	11 1/4°	FITTING SIZE	BEND ANGLE 90°	45°	22 1/2°	11 1/4°	ROD DIAM.	EMBED MENT
2, 3 & 4	1.9	2.7	1.4	0.7	0.4	2, 3 & 4	35.5	19.2	9.9	4.9	50"	30"
6	5.6	7.9	4.3	2.2	1.1	6	105.8	57.2	29.2	14.7	75"	30"
8	9.6	13.6	7.4	3.8	1.9	8	181.9	98.5	50.2	25.2	75"	30"
12	20.5	29.0	15.7	8.0	4.0	12	387.1	209.5	106.8	53.7	1.0"	30"
16	35.7	50.4	27.3	13.9	7.0	16	672.6	364.0	185.6	93.2	1.0"	36"
18	44.8	63.4	34.3	17.5	18.8	18	844.7	457.2	233.1	117.1	1.0"	36"
24	78.4	110.9	60.0	30.6	15.4	24	1478.7	800.3	408.0	205.0	1.5"	36"

★ 90 DEGREE BENDS SHALL ONLY BE USED ON A BY-EXCEPTION BASIS, AND MUST EACH BE SPECIFICALLY APPROVED BY CITY.

THRUST BLOCK NOTES:

1. CONCRETE FOR THRUST BLOCKS - CLASS A CONCRETE SHALL DEVELOP NOT LESS THAN 3000 P.S.I. COMPRESSIVE STRENGTH AT 28 DAYS AND BE PLACED AGAINST UNDISTURBED SOIL.
2. ALL BENDS, BOTH HORIZONTAL AND VERTICAL, SHALL BE BACKED WITH CONCRETE. VERTICAL BENDS SHALL BE PLACED ON CONCRETE PADS WHERE BENDS TURN UP, OR LOADED WHERE BENDS TURN DOWN.
3. WRAP PIPE JOINTS IN 8 MIL "POLYETHLENE" BEFORE PLACING CONCRETE.
4. BEARING AREA SHOWN IN TABLE IS BASED UPON A 2000 LBS/F SOIL BEARING, AND UPON A PIPELINE PRESSURE OF 200 psi. PLUS 100 psi. WATER HAMMER. AREAS SHOWN SHALL BE ADJUSTED, SHOULD FIELD CONDITIONS VARY.
5. MJ RESTRAINTS ARE REQUIRED FOR ALL FITTINGS.
6. USE LONG-RADIUS FITTINGS WHEREVER POSSIBLE.
7. ALL BOLTS FOR FITTINGS SHALL BE 316 STAINLESS STEEL.
8. ALL DUCTILE IRON FITTINGS SHALL BE FUSION-BONDED EPOXY COATED INSIDE AND OUTSIDE IN ACCORDANCE WITH ANSI/AWWA C116/A21.16.
9. UNIT WEIGHT OF CONCRETE FOR VERTICAL THRUST BLOCKS IS 150 LBS/CU. FT.

THRUST BLOCKING

CITY OF BRYANT, AR
WATER UTILITIES
210 S.W. 3rd. STREET
BRYANT, AR
PHONE: (501) 943-0468

TITLE:	WATER DETAILS	DATE: APRIL 2015	SHEET:
DESCRIPTION:	THRUST BLOCKING	REVISED ----- ----- ----- -----	W16
DRAWN BY:	CHECKED BY:	FILE: W16-Thrust Blocking.dwg	-----

PHILLIP LEWIS ENGINEERING, INC.
Structural + Civil Consultants
23620 Interstate 30 | Bryant, Arkansas
PH: 501-350-9840

REVISION:

NEW BEGINNINGS
HIGHWAY 5
BRYANT, ARKANSAS

STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 9540
PHILLIP A. LEWIS

CERTIFICATE OF AUTHORIZATION
PHILLIP LEWIS ENGINEERING, INC.
No. 566
ARKANSAS ENGINEER

PROJECT NUMBER:

SHEET ISSUE DATE:
08-06-2025

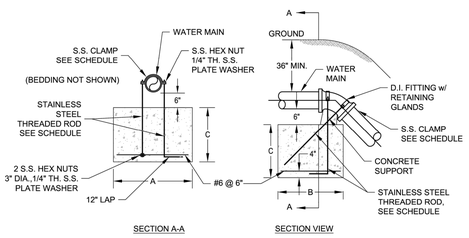
PAGE TITLE:
UTILITY DETAILS I

SHEET NUMBER:
C1.8

BLOCKING SCHEDULE					
PIPE SIZE	BENDS	45°	22 1/2°	11 1/4°	ROD DIA.
8"	VOLUME REQ'D (CU. FT.)	98.5	50.2	25.2	3/4 IN.
	A (FT.)	5.00'	4.00'	3.00'	
	B (FT.)	4.00'	3.20'	2.80'	
	C (FT.)	5.00'	4.00'	3.00'	
	MIN. CLAMP (2 EA.)	3/8 IN. x 2 IN.			
12"	VOLUME REQ'D (CU. FT.)	209.5	106.8	53.7	3/4 IN.
	A (FT.)	6.00'	5.00'	4.00'	
	B (FT.)	6.00'	4.25'	3.50'	
	C (FT.)	6.00'	5.00'	4.00'	
	MIN. CLAMP (2 EA.)	1/2 IN. x 2 IN.			
18"	VOLUME REQ'D (CU. FT.)	457.2	233.1	117.1	1 IN.
	A (FT.)	8.00'	6.50'	5.00'	
	B (FT.)	7.25'	5.50'	4.75'	
	C (FT.)	8.00'	6.50'	5.00'	
	MIN. CLAMP (2 EA.)	5/8 IN. x 3 IN.			
24"	VOLUME REQ'D (CU. FT.)	800.3	408.0	205.0	1 1/4 IN.
	A (FT.)	9.50'	7.50'	6.00'	
	B (FT.)	9.00'	7.25'	5.75'	
	C (FT.)	9.50'	7.50'	6.00'	
	MIN. CLAMP (2 EA.)	5/8 IN. x 3 IN.			

VOLUME CALCULATED ON THE BASIS OF CONCRETE REACTING THRUST ON THE RESPECTIVE BENDS UNDER AN INTERNAL PRESSURE OF 250 PSI, 50 PSI SURGE AND THE WEIGHT OF CONCRETE IS 150 POUNDS PER CU. FT.

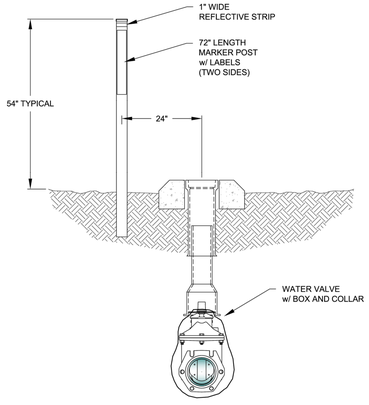
ALL FITTINGS SHALL BE MECHANICAL JOINTS WITH RETAINING GLANDS. BEDDING NOT SHOWN



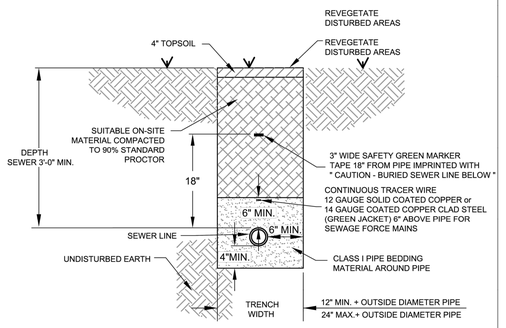
VERTICAL TIE-DOWN BLOCKING

NOTES:

1. WATER LINE MARKERS SHALL BE TRI-VIEW MARKING SYSTEM BY RHINO MARKING AND PROTECTION SYSTEMS OR CARSONITE INTERNATIONAL DUAL-SIDED UTILITY MARKER (CIB-380).
2. THE UPPERMOST PORTION OF THE CARSONITE MARKER SHALL BE MADE OF VISIBILITY ENHANCER (CVE-360).
3. TRI-VIEW MARKERS DO NOT REQUIRE VISIBILITY ENHANCERS.
4. AN ADDITIONAL WHITE 1" WIDE REFLECTIVE TAPE (3M OR EQUAL) SHALL BE PLACED AROUND THE FULL CIRCUMFERENCE OF THE TOP OF THE MARKER.



VALVE MARKER

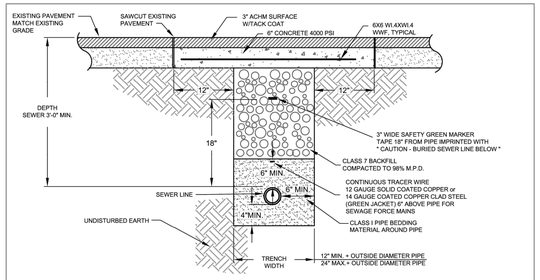


NOTES:

1. THE CONTRACTOR SHALL PROVIDE ALL ITEMS NECESSARY TO CONNECT WITH ANY PART OF THE EXISTING SEWER SYSTEM THAT WILL REMAIN IN ORDER TO ESTABLISH A SATISFACTORY AND ACCEPTABLE SEWER SYSTEM.
2. CONTRACTOR TO CONSTRUCT ALL TRENCH EXCAVATION IN ACCORDANCE WITH ALL OSHA REGULATIONS (29 CFR CH.XVII, SUBPART B)
3. TRENCH SHALL BE EXCAVATED BELOW GRADE REQUIRED TO PROVIDE A MINIMUM 36" OF PIPE COVER.

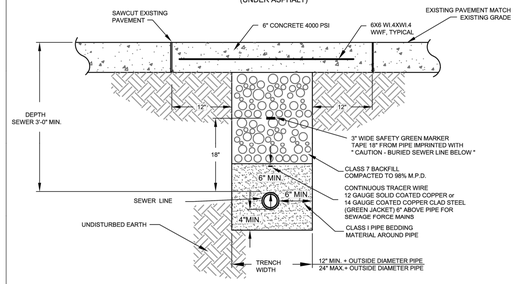
SEWER TRENCH

(NON-PAVED AREA)



SEWER TRENCH

(UNDER ASPHALT)



NOTES:

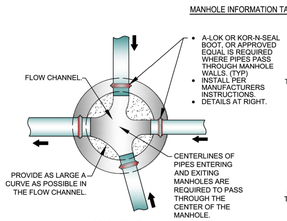
1. THE CONTRACTOR SHALL PROVIDE ALL ITEMS NECESSARY TO CONNECT WITH ANY PART OF THE EXISTING SEWER SYSTEM THAT WILL REMAIN IN ORDER TO ESTABLISH A SATISFACTORY AND ACCEPTABLE SEWER SYSTEM.
2. CONTRACTOR TO CONSTRUCT ALL TRENCH EXCAVATION IN ACCORDANCE WITH ALL OSHA REGULATIONS (29 CFR CH.XVII, SUBPART B)
3. TRENCH SHALL BE EXCAVATED BELOW GRADE REQUIRED TO PROVIDE A MINIMUM 36" OF PIPE COVER.

SEWER TRENCH

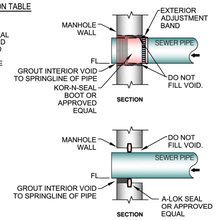
(UNDER CONCRETE)

Inside Diameter of Manhole	Minimum Wall Thickness	Base Thickness	Minimum Ring & Cover Size
4' DIA	5"	6"	24" (< or Equal to 24' Pipes)
5' DIA	7"	8"	
6' DIA	7"	8"	36" (> 24" Pipes)

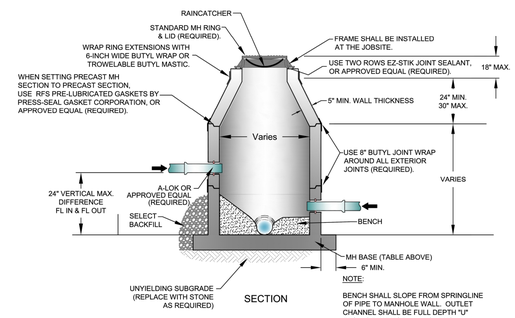
MANHOLE INFORMATION TABLE



MANHOLE FLOW CHANNEL

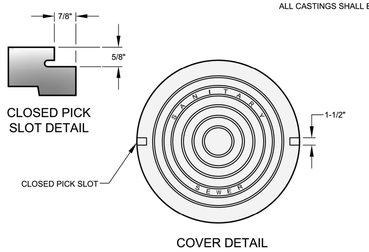


CONNECTION DETAILS



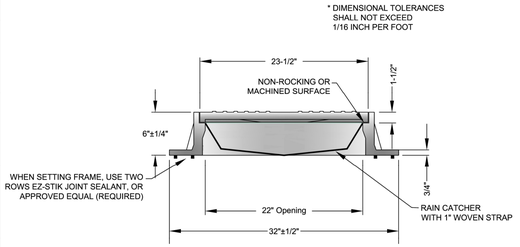
PRECAST MANHOLE

ALL CASTINGS SHALL BE "MADE IN USA"

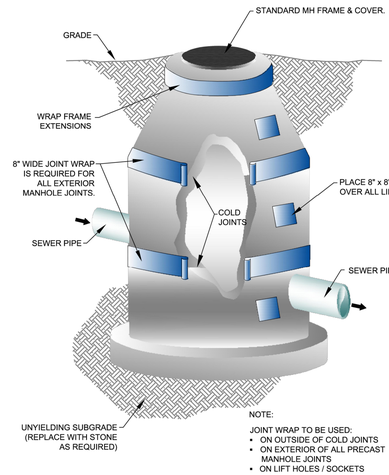


COVER DETAIL

1. MINIMUM WEIGHT OF RING: 100 POUNDS
2. MINIMUM WEIGHT OF COVER: 110 POUNDS
3. COVERS ARE FURNISHED WITH TWO CLOSED PICK SLOTS.
4. CASTINGS SHALL BE "MADE IN USA"



FRAME AND COVER DETAIL



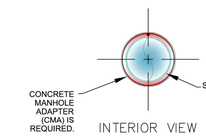
NOTE:

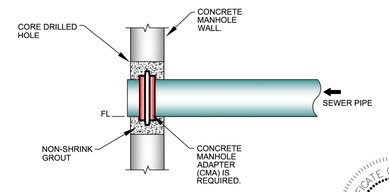
JOINT WRAP TO BE USED:

- ON OUTSIDE OF COLD JOINTS
- ON EXTERIOR OF ALL PRECAST MANHOLE JOINTS
- ON LIFT HOLES / SOCKETS

MANHOLE JOINT WRAP

THE INSTALLATION SHALL BE DYE TESTED FOR ACCEPTANCE.





MANHOLE CORING DETAILS

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PH: 501-350-9840

REVISION:

NEW BEGINNINGS
HIGHWAY 5
BRYANT, ARKANSAS

STATE OF ARKANSAS — REGISTERED PROFESSIONAL ENGINEER — No. 9540 — PHILLIP LEWIS

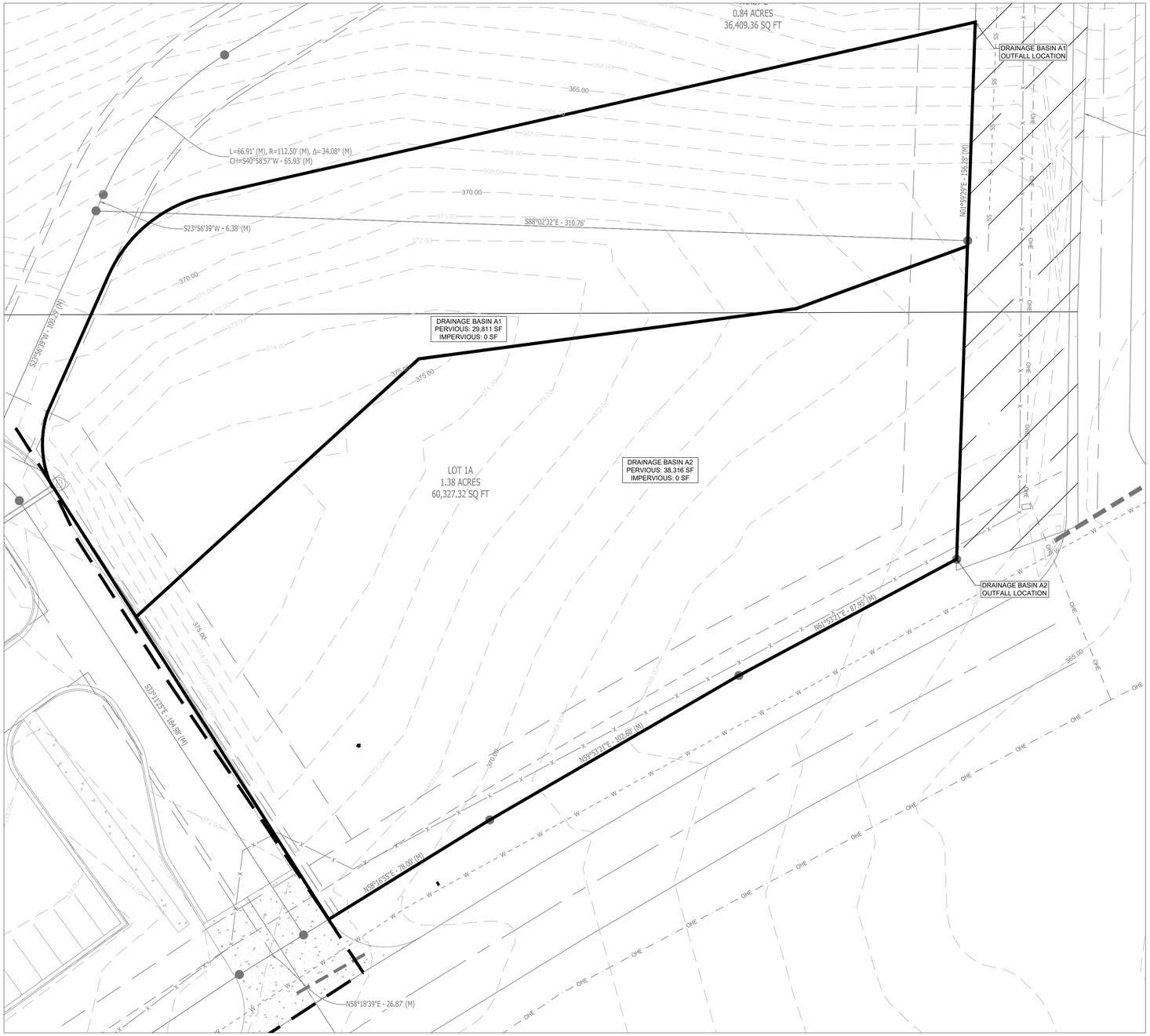
CERTIFICATE OF AUTHORIZATION — PHILLIP LEWIS ENGINEERING, INC. — No. 566 — ARKANSAS ENGINEER

PROJECT NUMBER:

SHEET ISSUE DATE: 08-06-2025

PAGE TITLE: UTILITY DETAILS II

SHEET NUMBER: C1.9



PRE-DEVELOPMENT DRAINAGE BASIN PLAN

SCALE 1" = 20'

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Structural + Civil Consultants
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NEW BEGINGS
HIGHWAY 5
BRYANT, ARKANSAS

CERTIFICATE OF AUTHORIZATION
PHILLIP LEWIS ENGINEERING, INC.
No. 566
ARKANSAS ENGINEER

STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 9540
PHILLIP A. LEWIS

N

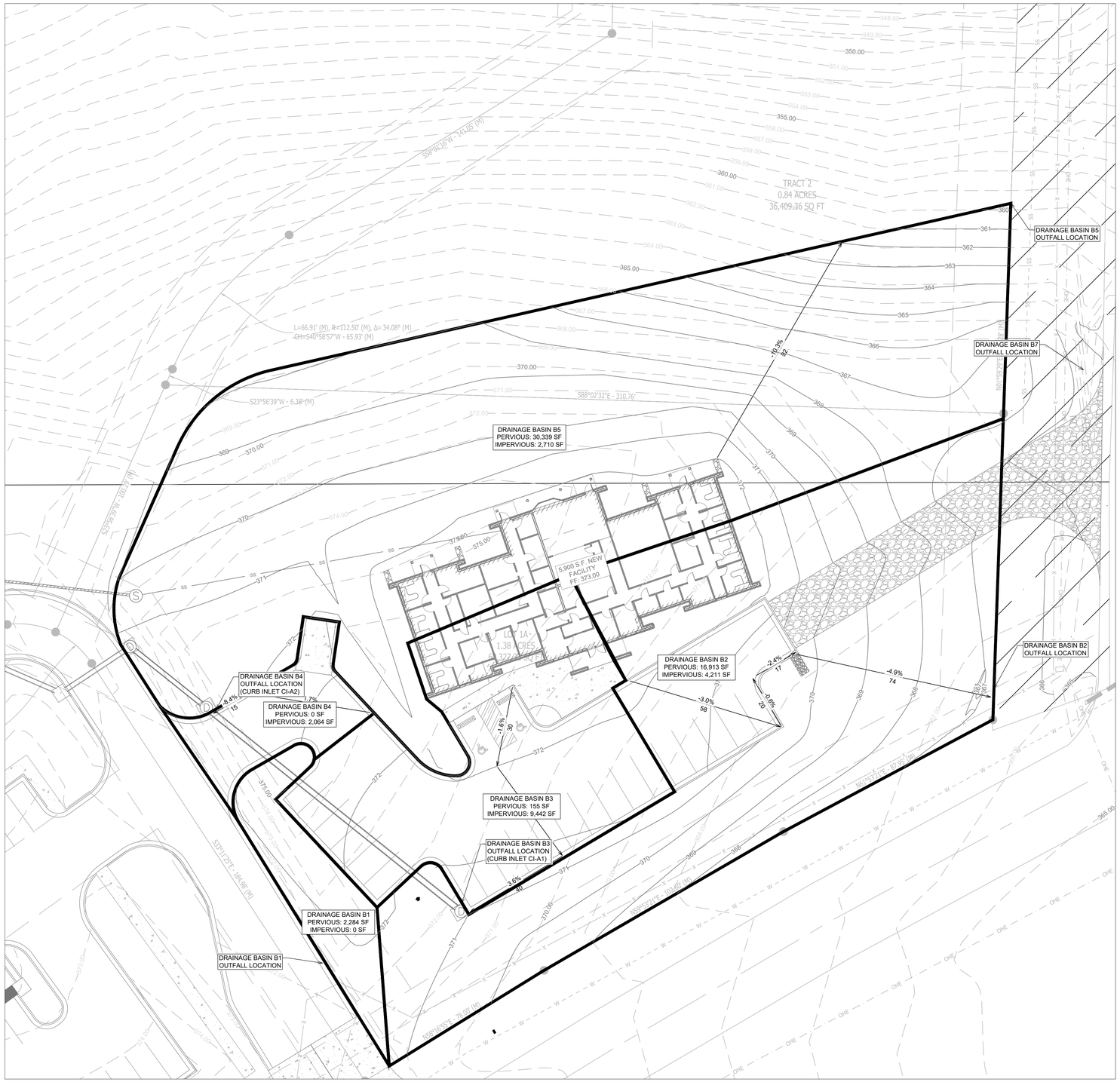
0 20'
1" = 20'-0"

PROJECT NUMBER:

SHEET ISSUE DATE:
08-06-2025

PAGE TITLE:
PRE-DEV DRAINAGE

SHEET NUMBER:
C1.10



POST-DEV DRAINAGE

SCALE 1" = 20'

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REVISION:

NEW BEGININGS
HIGHWAY 5
BRYANT, ARKANSAS

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PHILLIP LEWIS ENGINEERING, INC.
No. 566
ARKANSAS ENGINEER

STATE OF
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REGISTERED
PROFESSIONAL
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No. 9540
PHILLIP LEWIS

PROJECT NUMBER:

SHEET ISSUE DATE:
08-06-2025

PAGE TITLE:
POST-DEV DRAINAGE

SHEET NUMBER:
C1.11

N

0 10'
1" = 10'-0"

LANDSCAPE PLAN

SCALE 1" = 60'

NOTES:

1. LANDSCAPED AREAS TO BE AMENDED WITH 4" OF TOPSOIL, SCARIFY SOIL 3" PRIOR TO APPLICATION. ALL TOP SOIL SHALL BE PLACED IN COORDINATION WITH GRADING AND DRAINAGE PLANS TO ENSURE THAT THE GRADING AND DRAINAGE DESIGN FOR THE SITE IS MAINTAINED AFTER BEING SODDED OR SEEDED. EXISTING SOIL FROM THE SITE CAN BE STOCK PILED AND REUSED AS LONG AS IT IS OF QUALITY THAT ENCOURAGES ADEQUATE GROWTH OF PLANTING MATERIAL. THE CONTRACTOR IS RESPONSIBLE FOR ANY SOIL TESTING THAT MAY BE REQUIRED.
2. LANDSCAPE PLAN REPRESENTS RECOMMENDED SPECIES, SIZES, & LOCATIONS. OWNER SHALL CHANGE THE ITEMS TO EQUAL OR GREATER VALUE.

GENERAL CONSTRUCTION NOTES

A. THE CONTRACTOR WILL BE HELD SOLELY RESPONSIBLE FOR DAMAGES OCCURRING TO ANY PROPERTY DURING THE CONSTRUCTION OF THIS PROJECT. SAID CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PREVENT PROPERTY DAMAGE.

B. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL SOLELY AND COMPLETELY BE RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND WILL NOT BE LIMITED TO NORMAL WORKING HOURS.

THE DUTY OF BRYANT TO CONDUCT CONSTRUCTION INSPECTION REVIEWS OF THE CONTRACTOR'S PERFORMANCE IS NOT AN INSPECTION OR REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES IN, ON, OR NEAR THE CONSTRUCTION SITE.

C. ALL WATER AND SEWER IMPROVEMENTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST REVISION TO THE CITY OF BRYANT'S WATER AND WASTEWATER (SANITARY SEWER) STANDARD SPECIFICATIONS.

D. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES AND, WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANY AT LEAST 48 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF ALL UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH PROPOSED IMPROVEMENTS SHOWN ON THE PLAN.

E. CONTRACTOR IS TO REMOVE AND DISPOSE OF ALL DEBRIS, RUBBISH, AND OTHER MATERIALS RESULTING FROM PREVIOUS AND CURRENT DEMOLITION OPERATIONS. DISPOSAL WILL BE IN ACCORDANCE WITH ALL LOCAL, STATE AND/OR FEDERAL REGULATIONS GOVERNING SUCH OPERATIONS.

F. PRIOR TO INSTALLATION OF ANY UTILITIES, THE CONTRACTOR IS TO EXCAVATE, VERIFY AND CALCULATE ALL CROSSINGS AND INFORM ANY AND ALL UTILITIES OF ANY CONFLICTS PRIOR TO CONSTRUCTION.

G. CONSTRUCTION SHALL NOT START ON ANY WATER UTILITY TIE-INS UNTIL APPROVAL IS GIVEN BY BRYANT WATER. SAID CONTRACTOR SHALL NOT OPERATE ANY VALVE, HYDRANT, OR WATER UTILITY APPURTENANCE NOR SHALL HE ATTACH TO OR TAP ANY WATER UTILITY MAIN WITHOUT APPROVAL. THE CONTRACTOR SHALL BEAR THE COST AND CONSEQUENCE OF ANY DISRUPTION OF UTILITY OPERATION CAUSED BY CONSTRUCTION.

H. FIBER OPTIC CABLE ON AND/OR ADJACENT TO THIS SITE WERE NOT LOCATED BY THE SURVEY AND ARE NOT SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ANY FIBER OPTIC CABLES ASSOCIATED WITH THIS SITE AND TAKE ALL NECESSARY AND REQUIRED PRECAUTIONS TO PROTECT ANY EXISTING FIBER OPTIC CABLES. CONTRACTORS SHALL COORDINATE ALL EFFORTS WITH OWNER OF FIBER OPTIC CABLES OR THEIR DESIGNATED REPRESENTATIVE.

J. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING "ONECALL" SERVICE TO MARK ALL UTILITIES PRIOR TO ANY DEMOLITION, EARTHWORK, OR UTILITY WORK ON THIS SITE.

PLANT SCHEDULE

PLANT TYPE	SYMBOL	CODE	QTY	COMMON SPECIES	SCIENTIFIC NAME	CAL / SIZE
TREES		WO	6	WILLOW OAK	QUERCUS PHELLOS	MIN. 3" DIAMETER @ BASE AND 12' TALL
SHRUBS		BW	40	DWARF NANDINA	N. DOMESTICA "HARBOUR DWARF"	3 GAL
GROUND COVER		SO	25,587 SF	BERMUDA SOD		
		GS	24,140 SF	GRASS SEED		
			1,828 SF	LANDSCAPE BEDDING (TBD BY OWNER)		

CERTIFICATE OF AUTHORIZATION
PHILLIP LEWIS ENGINEERING, INC.
No. 566
ARKANSAS-ENGINEER

N

0 60'

1" = 60'-0"

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Structural + Civil Consultants
23620 Interstate 30 | Bryant, Arkansas
PH: 501-350-9840

REVISION:

NEW BEGINNINGS
HIGHWAY 5
BRYANT, ARKANSAS

STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 9540
PHILLIP W. LEWIS

PROJECT NUMBER:

SHEET ISSUE DATE:
08-06-2025

PAGE TITLE:
LANDSCAPE PLAN

SHEET NUMBER:
C1.12

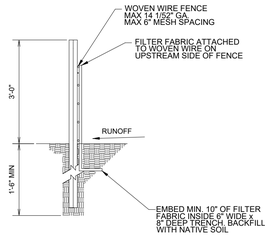
STORMWATER POLLUTION PREVENTION PLAN

SCALE 1" = 60'

INSTALL FILTER SOCKS AT ALL INLETS; MAINTAIN THROUGHOUT CONSTRUCTION.
DISTURBED AREA: 85,405 AC.

LEGEND

- DISTURBED AREA
- UNDISTURBED AREA
- SEDIMENT FENCE WITH WIRE BACKING
- DRAINAGE DIRECTION



SILT FENCE

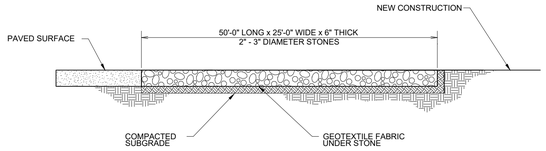
NOT TO SCALE

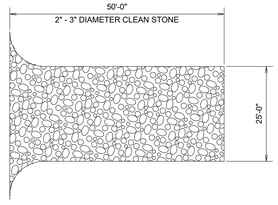
NOTES AND SPECIFICATIONS:

1. POSTS SHALL BE A MINIMUM OF 36 INCHES CONSTRUCTED OF EITHER OF THE FOLLOWING MATERIALS: STEEL "T" OR "U" TYPE, OR 2" x 2" HARDWOOD.
2. WOVEN WIRE USED AS ADDITIONAL FENCE SUPPORT SHALL BE MINIMUM 14.5 GA. WITH 6" MAXIMUM SPACING.
3. WOVEN WIRE SHALL BE PLACED ALONG THE UPHILL SIDE OF THE FENCE AND FASTENED WITH WIRE TIES OR 1" STAPLES ALONG THE UPHILL SIDE OF THE POSTS.
4. FILTER FABRIC SHALL BE FASTENED TO WOVEN WIRE ACCORDING TO MANUFACTURER'S RECOMMENDATION, OR WITH TIES EVERY 24" AT THE TOP AND MID-SECTIONS.
5. WHERE TWO PIECES OF FILTER FABRIC ADJOIN EACH OTHER THEY SHALL BE OVERLAPPED BY 6 INCHES AND FOLDED TOGETHER.
6. WHERE TWO POSTS MEET TO JOIN FENCE SECTIONS, THE TOPS OF THE POSTS SHALL BE SECURED TOGETHER WITH WIRE.
7. THE FENCE SHALL BE CONSTRUCTED ALONG THE CONTOUR AS MUCH AS POSSIBLE.
8. ENDS OF FENCES SHALL BE EXTENDED UP THE SLOPE TO PRVENT RUNOFF FROM MIGRATING AROUND THE END OF THE FENCE.
9. INSPECTION OF THE FENCE SHALL BE PERFORMED WEEKLY, OR IMMEDIATELY AFTER A RAIN EVENT, OR WHEN BULGES APPEAR IN THE FENCE. ACCUMULATED SILT SHALL NOT BE ALLOWED TO EXCEED HALF THE HEIGHT OF THE FABRIC. REPAIR AND OR REPLACMENT OF DAMAGED FENCE SHALL BE COMPLETED PROMPTLY.
10. ACCUMULATED SILT SHALL BE REMOVED AND DISPOSED OF IN AN APPROVED SITE IN SUCH A MANNER THAT IT WILL NOT CONTRIBUTE TO OFF-SITE SILTATION.
11. ALL FENCING SHALL BE REMOVED WITH THE CONSTRUCTION SITE IS FULLY STABLIZED SO AS TO NOT IMPEDE STORM FLOW OR DRAINAGE.
12. PRE-FRABRICATED UNITS DO NOT REQUIRE THE USE OF WOVEN WIRE FENCE.

NOTES (GENERAL):

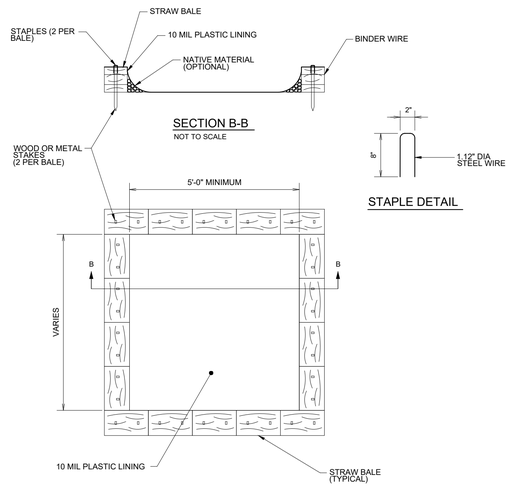
1. SEE EROSION CONTROL DETAILS IN SWPPP FOR EROSION CONTROL FACILITIES.
2. SEE SWPPP FOR INSTALLATION, MAINTENANCE, INSPECTION, AND RECORD KEEPING REQUIREMENTS.
3. CONTRACTOR SHALL SHOW EROSION CONTROL MEASURE ON SITE MAP.
4. EROSION AND SEDIMENT CONTROL STRUCTURES TO MEET SWPPP DETAILS - APPENDIX D
5. INSTALL ROCK DITCH, CHECK, OR SAND BAG CHECKS AS NECESSARY TO PREVENT SCOUR UNTIL LANDSCAPING IS ESTABLISHED.
6. CONTRACTOR MUST PLACE SEDIMENT BASIN WITH SEDIMENT FENCE OUTLET FOR ANY SEDIMENT CONTAMINATED DEWATERING DISCHARGE.
7. FINAL SLOPE WILL BE SAME DIRECTION AS EXISTING SLOPE.
8. TEMPORARY STABILIZATION PRACTICES WILL NOT BE REQUIRED. WORK WILL BE CONTINUOUS AND DISTURBED AREA REVEGITATED IN A TIMELY MANNER. SEE SWPPP FOR SEEDING MIXES.
9. PERMANENT STABILIZATION OF ALL DISTURBED AREAS ARE TO BE SEEDED, FERTILIZED, WATERED AND COVERED WITH STRAW UNLESS OTHERWISE NOTED ON PLANS TO BE HYDROSEEDED.
10. CONTRACTOR TO SHOW CONCRETE WASH OUT SUMP, ENTRANCE/EXIT PAD AND OTHER CONTROLS AS REQUIRED/NEEDED AS SWPPP SITE MAP IS UPDATED THROUGHOUT THE DURATION OF THE PROJECT.
11. STOCKPILING OF CONSTRUCTION SPOIL MATERIAL AT PARTICULAR LOCATIONS SHALL ONLY BE ALLOWED FOR A LIMITED TIME PERIOD, NOT TO EXCEED (6) MONTHS. PRIOR TO A FINAL INSPECTION OF THE GRADING PERMIT, THE FOLLOWING STANDARDS SHALL BE ACHIEVED FOR COMPLETION OF CONSTRUCTION:
    a) DEVELOPMENT AND GRADING WITHIN THE DISTURBED AREA IS COMPLETE AND MATCHES PLANS AS APPROVED BY THE PLANNING COMMISSION, AND
    b) THE DISTURBED SOIL AREA IS OBSERVED TO HAVE 80% GRASS COVERAGE AND 100% STABILITY, AND
    c) NO SLOPES STEEPER THAN A 3:1 PITCH UNLESS OTHERWISE APPROVED IN WRITING BY THE DIRECTOR OF ENGINEERING, AND
    d) NOTICE OF VIOLATIONS ISSUED HAVE ALL CORRECTIVE ACTIONS APPROVED WITH AN INSPECTION REPORT SIGNED BY A REPRESENTATIVE OF THE DIRECTOR OF ENGINEERING, AND
    e) ALL HEAVY EQUIPMENT, STOCKPILES, AND CONSTRUCTION SITE MATERIALS HAVE BEEN REMOVED FROM THE CONSTRUCTION SITE.





CONSTRUCTION ENTRANCE

NOT TO SCALE



CONCRETE WASHOUT

NOT TO SCALE

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No. 566
ARKANSAS ENGINEER
PHILLIP LEWIS

N

0 60'

1" = 60'-0"

PHILLIP LEWIS ENGINEERING, INC.
Structural + Civil Consultants
23620 Interstate 30 | Bryant, Arkansas
PH: 501-350-9840

REVISION:

NEW BEGINNINGS
HIGHWAY 5
BRYANT, ARKANSAS

STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 9540
PHILLIP LEWIS
PRELIMINARY - NOT FOR CONSTRUCTION

PROJECT NUMBER:

SHEET ISSUE DATE:
08-06-2025

PAGE TITLE:
SWPPP

SHEET NUMBER:
C1.13

NEW BEGINNINGS

DRAINAGE REPORT

Date: **08-07-2025**

Located in: **Bryant, Arkansas**

Prepared for:
City of Bryant, Arkansas

Prepared by:

PHILLIP LEWIS ENGINEERING

Structural + Civil Consultants

23620 Interstate 30 | Bryant, AR
PH: 501-350-9840

Table of Contents

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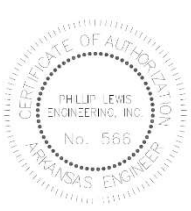
STORM SEWER SIZING 95
DRAINAGE BASIN MAPS 105
SOIL CLASSIFICATION MAPS 108
FEMA FLOOD INSURANCE RATE MAP 120

CERTIFICATION

I hereby state that this Drainage Analysis has been prepared by me or under my supervision and meets the standard of care and expertise which is usual and customary in this community of professional engineers. The analysis has been prepared utilizing procedures and practices by the City of Bryant and within the standard accepted practices.



DATE: 08-07-2024

Phillip A. Lewis, PE.

DESCRIPTION OF PROPERTY

The proposed project is for the construction of a new pregnancy center located along Highway 5, directly adjacent to the current ongoing seminary project. The proposed development is a 5,900 sq. ft. building and parking lot.

The intent of this drainage analysis is to adequately size the storm sewer system and summarize pre and post runoff conditions.

The existing ground coverage for the entire development drainage basin consists of natural vegetation (2%-7% slope), hydrologic soil group B/C.

According to FEMA Flood Insurance Rate Map, Panel 05125C0240E, this property lies within Zone X, areas determined to be outside the 0.2% annual chance floodplain. A copy of the map can be found in the appendix.

PROJECT LOCATION MAP

DRAINAGE CRITERIA

In accordance with the requirements of the City of Bryant, the proposed developments drainage plan and this drainage report were developed with the criteria established in the Bryant Stormwater Management & Drainage Manual provided on cityofbryant.com.

All drainage calculations were performed using HydroCAD software to determine and analyze the changes in storm runoff volume, flow rates, and design the outlet release structure. Hydraflow Express software was used to appropriately design and size all storm sewer inlets, pipes and channels.

Calculations were performed using the Rational Method, using NOAA rainfall data, Runoff Coefficient table (Bryant Stormwater Management & Drainage Manual, Table 400-2) and the pipe and inlet structure sizes were determined by the 25-year storm event.

PROPOSED DRAINAGE SYSTEM

This development is designed to capture the majority of runoff within the parking lot curb and gutter. A portion of the site will discharge into a standard storm sewer system, releasing into the existing adjacent storm sewer. Other portions of the new development will either be captured by gutter/downspouts or curb/gutter and released to vegetated greenspace surrounding the project. These release points are similar to the pre-development conditions of this site.

The storm sewer system will consist of standard concrete curb inlets. These inlets were sized based on their independent drainage basin flow rate and the slope that the inlets will be placed at. The New Beginnings storm sewer system will tie-into the Bryant Seminary existing storm sewer system. The stormwater will ultimately be discharged to the north side of the property into the floodplain.

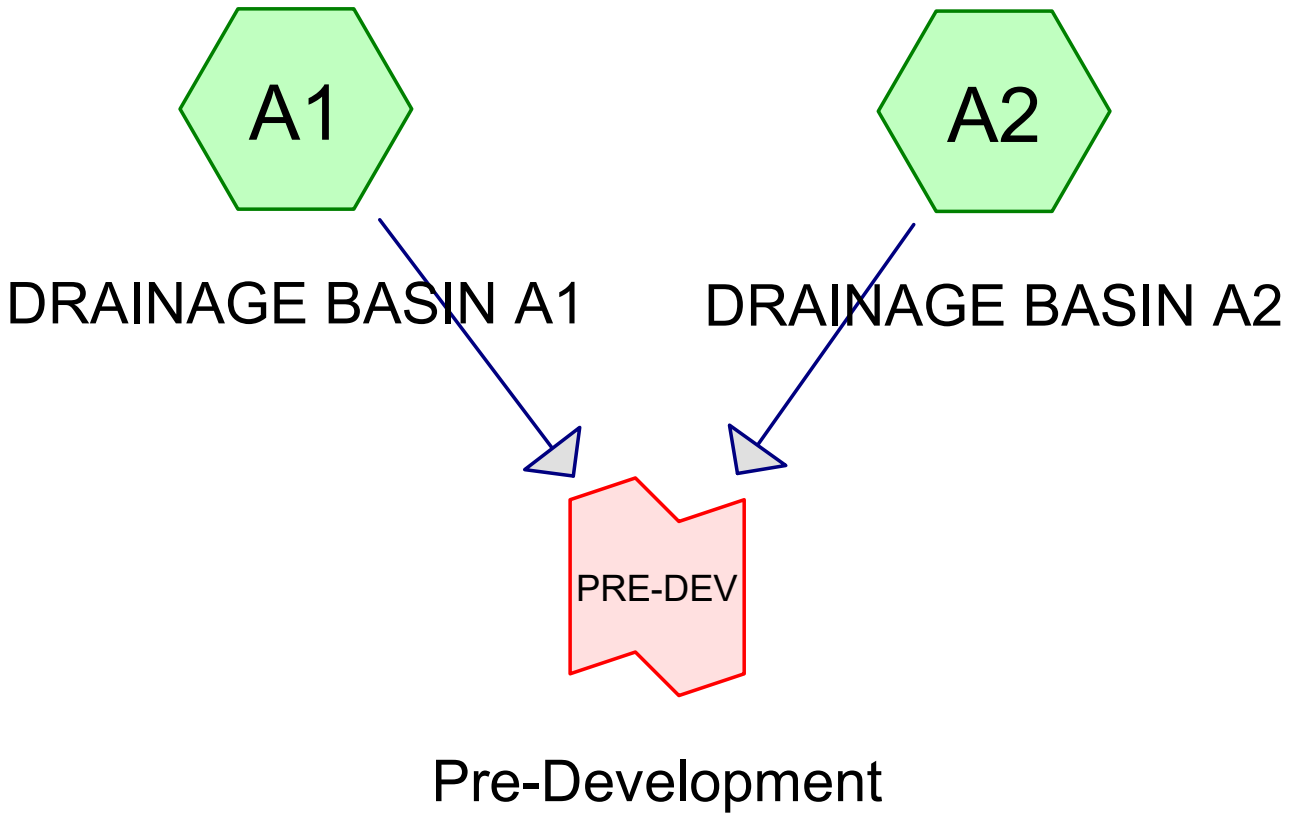
Overall Pre-development and Post-development runoff/discharge rates are compared below:

Storm Event	Pre-development Discharge (cfs)	Post-development Discharge (cfs) Without Detention
2-yr	1.51	4.38
10-yr	2.02	5.86
25-yr	2.33	6.82
50-yr	2.56	7.41
100-yr	2.77	8.04

Hydraulic grade elevations for the inlets are shown below:

Inlet	Peak Elevation (25-yr Storm Event)
CI – A1	369.08’
CI – A2	368.89’

PRE DEVELOPMENT HYDROGRAPHS



Pre-Development

Subcat Reach Pond Link

Routing Diagram for New Beginnings Drainage
Prepared by Phillip Lewis Engineering, Printed 7/24/2025
HydroCAD® 10.20-6a s/n 12520 © 2024 HydroCAD Software Solutions LLC

Summary for Subcatchment A1: DRAINAGE BASIN A1

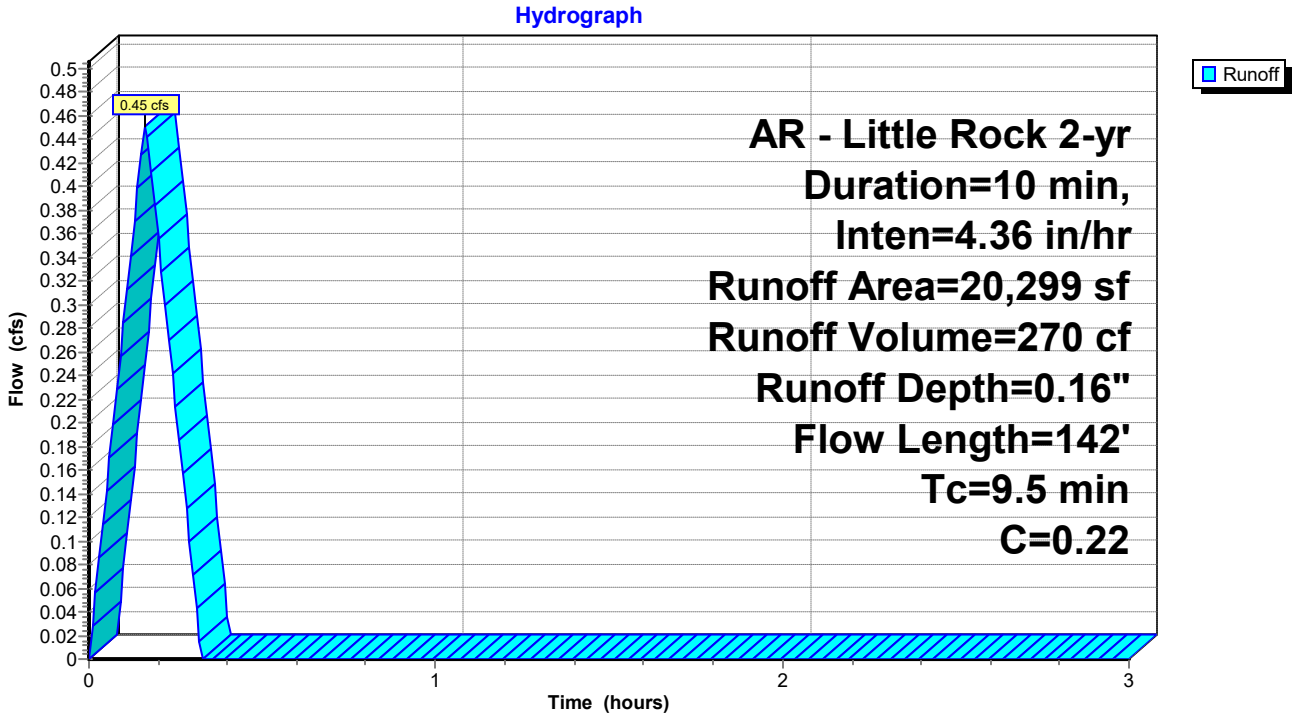
Runoff = 0.45 cfs @ 0.16 hrs, Volume= 270 cf, Depth= 0.16"
Routed to Link PRE-DEV : Pre-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 2-yr Duration=10 min, Inten=4.36 in/hr

Area (sf)	C	Description
20,299	0.22	Sandy Soil 2-7% per manual (undeveloped)
20,299		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.2	100	0.0430	0.18		**Sheet Flow, Overland Sheet flow** Grass: Dense n= 0.240 P2= 4.20"
0.3	42	0.1410	2.63		**Shallow Concentrated Flow, Overland Concentrated Flow** Short Grass Pasture Kv= 7.0 fps
9.5	142	Total			

Subcatchment A1: DRAINAGE BASIN A1



Summary for Subcatchment A2: DRAINAGE BASIN A2

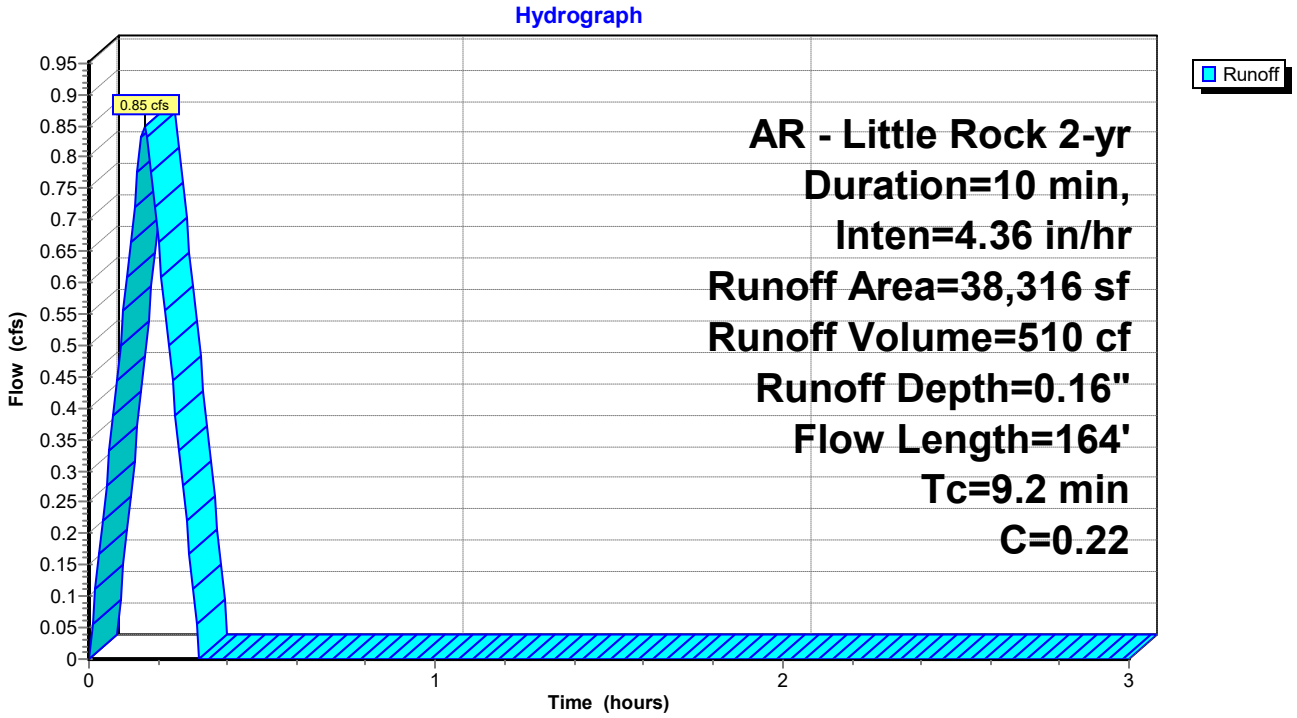
Runoff = 0.85 cfs @ 0.16 hrs, Volume= 510 cf, Depth= 0.16"
Routed to Link PRE-DEV : Pre-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 2-yr Duration=10 min, Inten=4.36 in/hr

Area (sf)	C	Description
38,316	0.22	Sandy Soil 2-7% per manual (undeveloped)
38,316		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.3	100	0.0560	0.20		**Sheet Flow, Overland Sheet flow** Grass: Dense n= 0.240 P2= 4.20"
0.9	64	0.0320	1.25		**Shallow Concentrated Flow, Overland Concentrated Flow** Short Grass Pasture Kv= 7.0 fps
9.2	164	Total			

Subcatchment A2: DRAINAGE BASIN A2

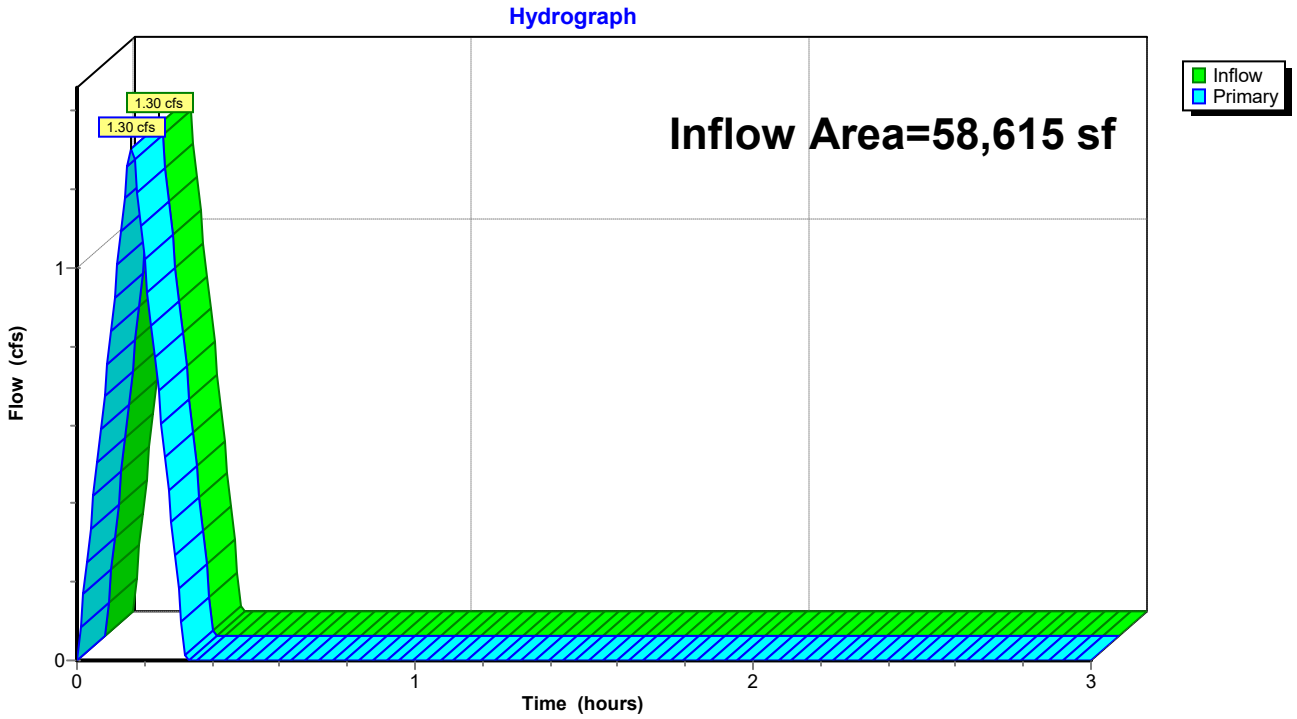


Summary for Link PRE-DEV: Pre-Development

Inflow Area = 58,615 sf, 0.00% Impervious, Inflow Depth = 0.16" for 2-yr event
Inflow = 1.30 cfs @ 0.16 hrs, Volume= 780 cf
Primary = 1.30 cfs @ 0.16 hrs, Volume= 780 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs

Link PRE-DEV: Pre-Development



Summary for Subcatchment A1: DRAINAGE BASIN A1

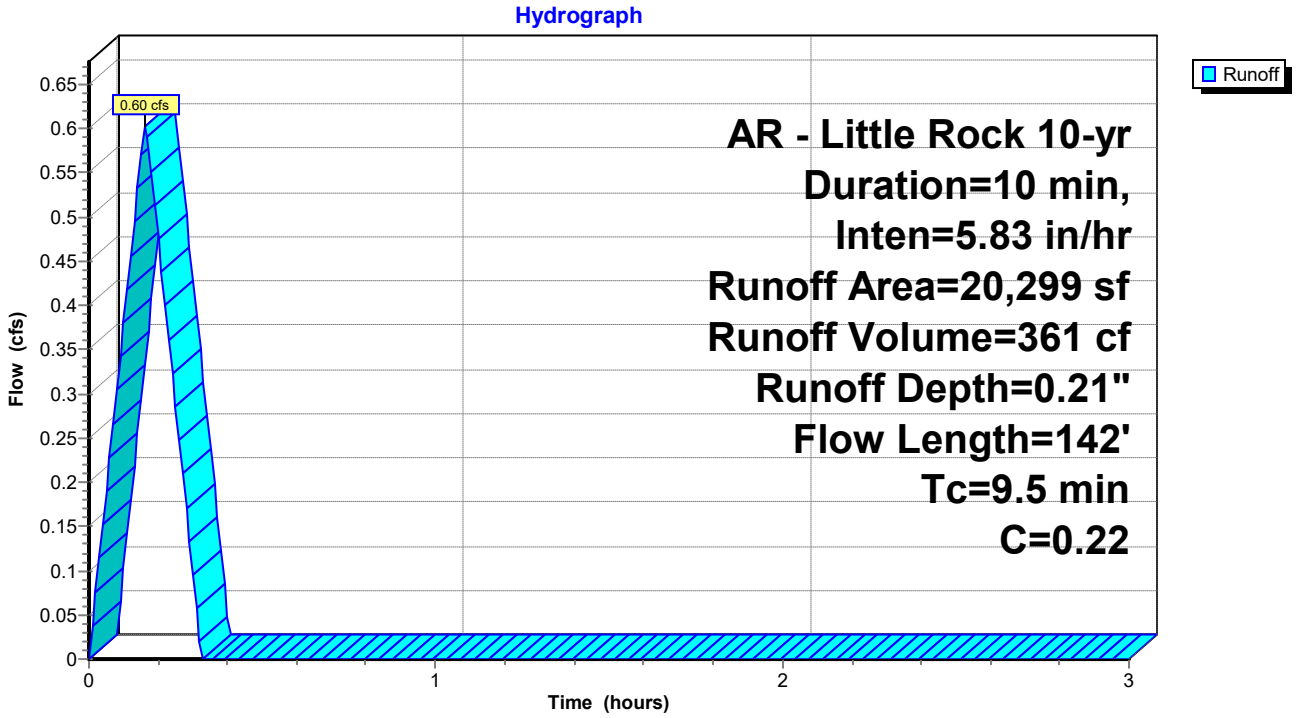
Runoff = 0.60 cfs @ 0.16 hrs, Volume= 361 cf, Depth= 0.21"
Routed to Link PRE-DEV : Pre-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 10-yr Duration=10 min, Inten=5.83 in/hr

Area (sf)	C	Description
20,299	0.22	Sandy Soil 2-7% per manual (undeveloped)
20,299		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.2	100	0.0430	0.18		**Sheet Flow, Overland Sheet flow** Grass: Dense n= 0.240 P2= 4.20"
0.3	42	0.1410	2.63		**Shallow Concentrated Flow, Overland Concentrated Flow** Short Grass Pasture Kv= 7.0 fps
9.5	142	Total			

Subcatchment A1: DRAINAGE BASIN A1



Summary for Subcatchment A2: DRAINAGE BASIN A2

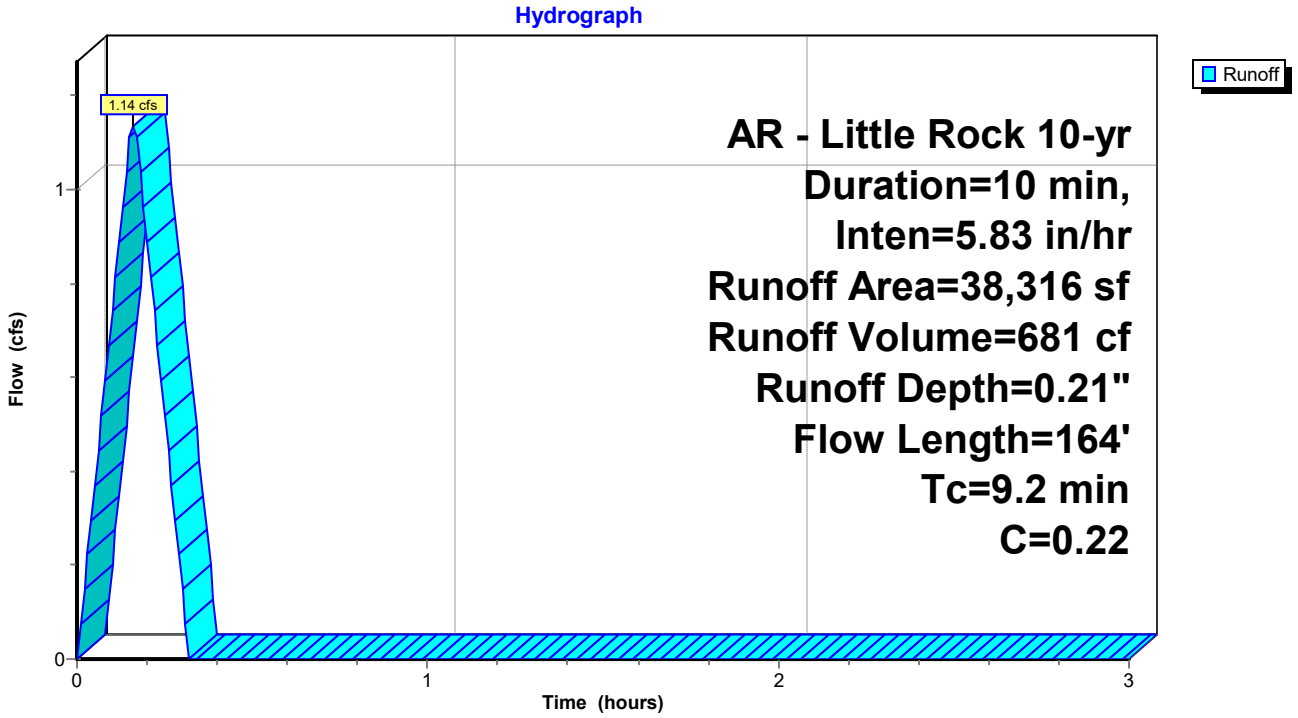
Runoff = 1.14 cfs @ 0.16 hrs, Volume= 681 cf, Depth= 0.21"
Routed to Link PRE-DEV : Pre-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 10-yr Duration=10 min, Inten=5.83 in/hr

Area (sf)	C	Description
38,316	0.22	Sandy Soil 2-7% per manual (undeveloped)
38,316		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.3	100	0.0560	0.20		**Sheet Flow, Overland Sheet flow** Grass: Dense n= 0.240 P2= 4.20"
0.9	64	0.0320	1.25		**Shallow Concentrated Flow, Overland Concentrated Flow** Short Grass Pasture Kv= 7.0 fps
9.2	164	Total			

Subcatchment A2: DRAINAGE BASIN A2

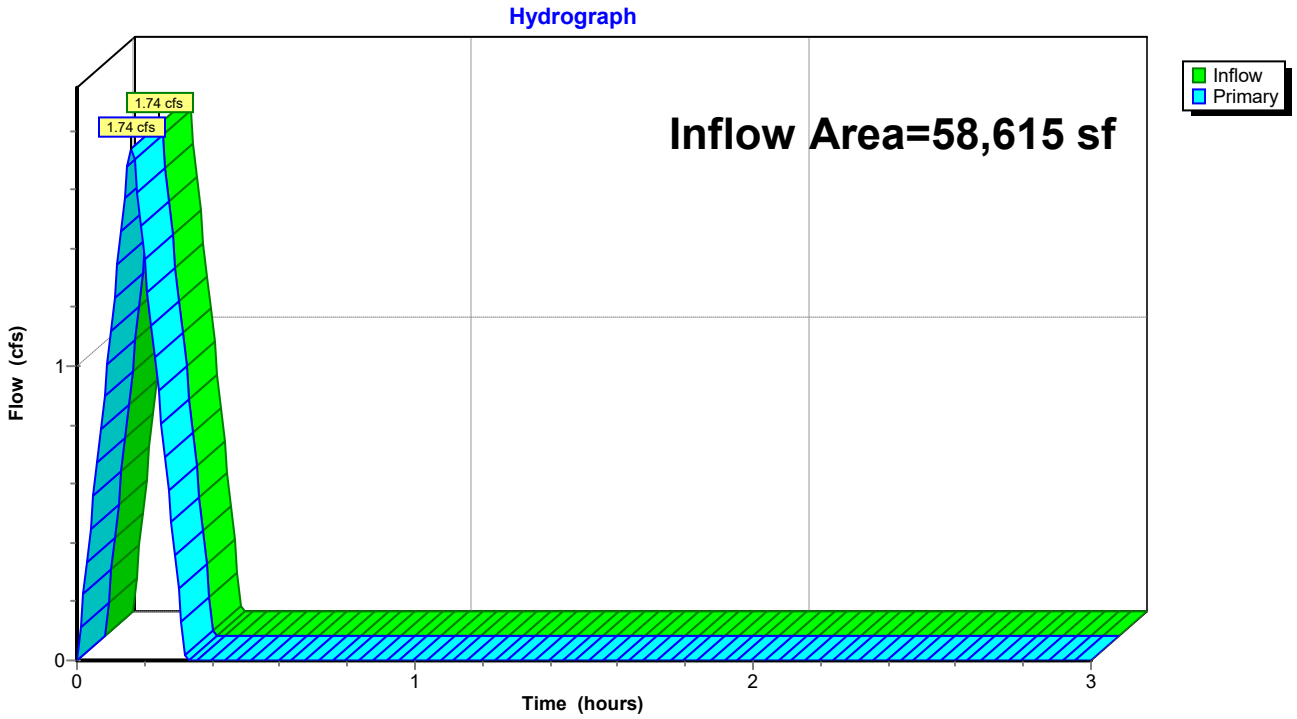


Summary for Link PRE-DEV: Pre-Development

Inflow Area = 58,615 sf, 0.00% Impervious, Inflow Depth = 0.21" for 10-yr event
Inflow = 1.74 cfs @ 0.16 hrs, Volume= 1,043 cf
Primary = 1.74 cfs @ 0.16 hrs, Volume= 1,043 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs

Link PRE-DEV: Pre-Development



Summary for Subcatchment A1: DRAINAGE BASIN A1

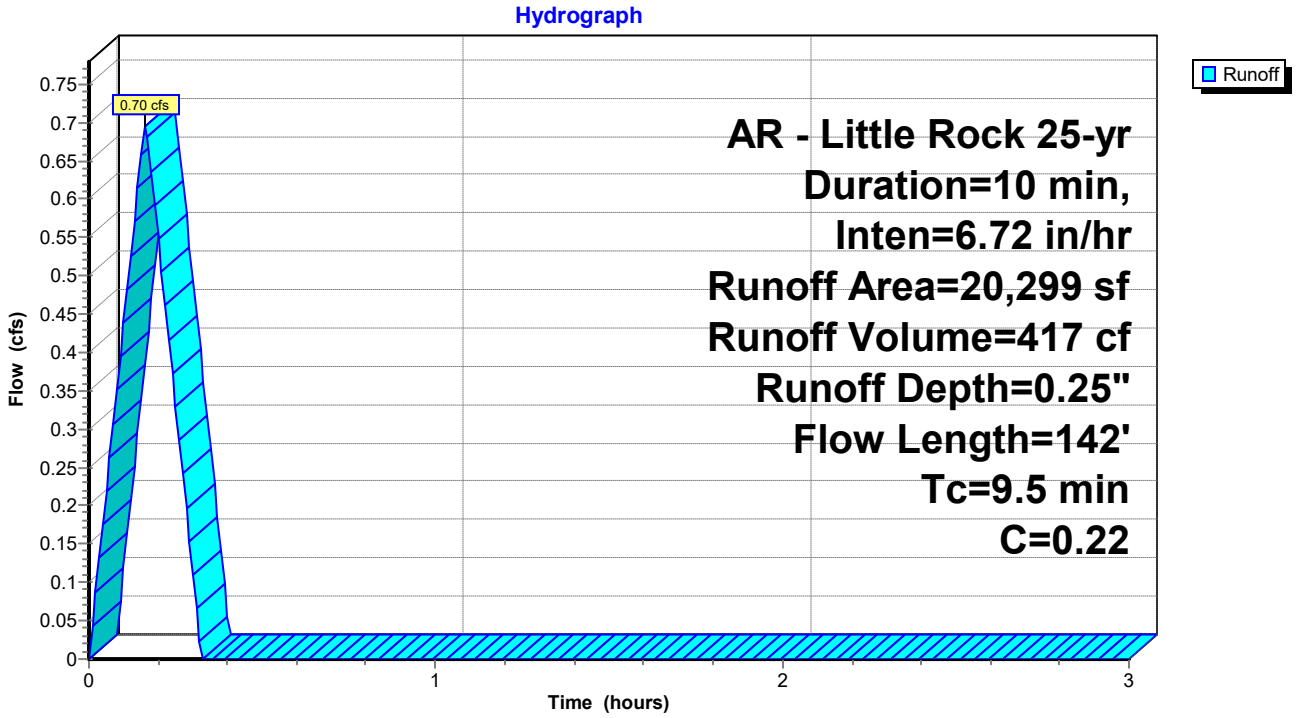
Runoff = 0.70 cfs @ 0.16 hrs, Volume= 417 cf, Depth= 0.25"
Routed to Link PRE-DEV : Pre-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 25-yr Duration=10 min, Inten=6.72 in/hr

Area (sf)	C	Description
20,299	0.22	Sandy Soil 2-7% per manual (undeveloped)
20,299		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.2	100	0.0430	0.18		**Sheet Flow, Overland Sheet flow** Grass: Dense n= 0.240 P2= 4.20"
0.3	42	0.1410	2.63		**Shallow Concentrated Flow, Overland Concentrated Flow** Short Grass Pasture Kv= 7.0 fps
9.5	142	Total			

Subcatchment A1: DRAINAGE BASIN A1



Summary for Subcatchment A2: DRAINAGE BASIN A2

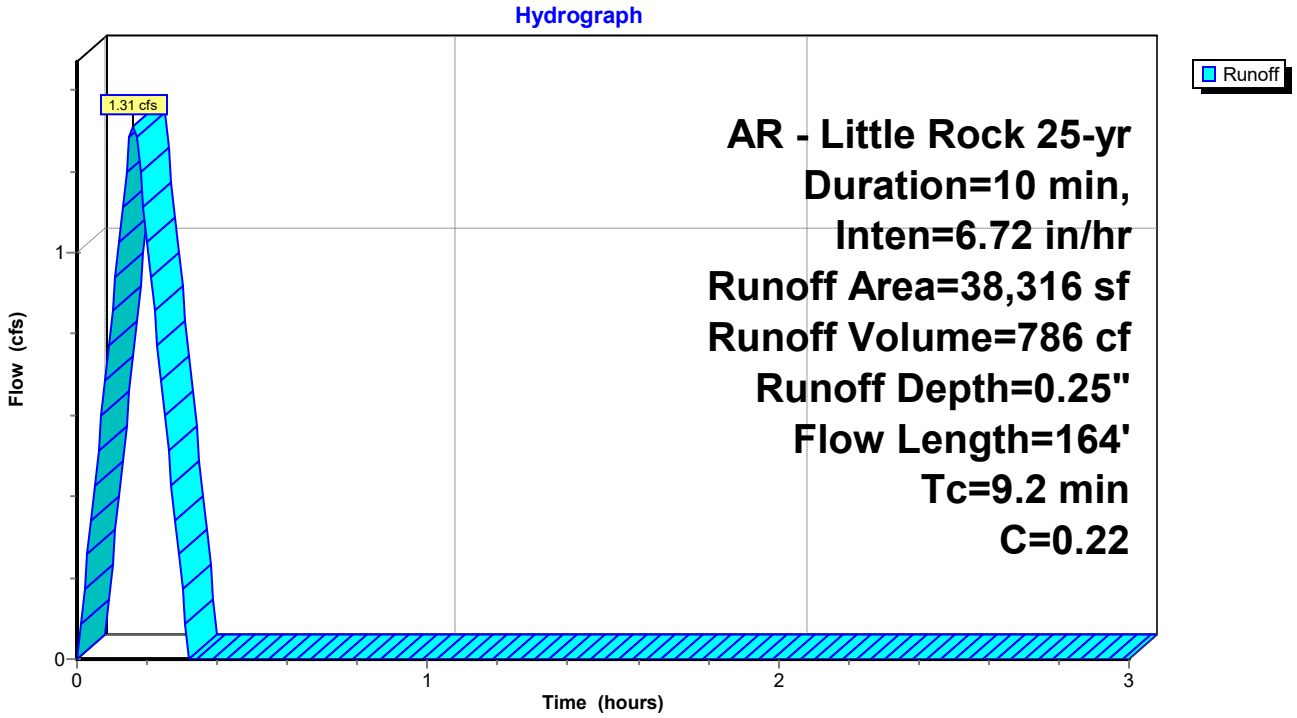
Runoff = 1.31 cfs @ 0.16 hrs, Volume= 786 cf, Depth= 0.25"
Routed to Link PRE-DEV : Pre-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 25-yr Duration=10 min, Inten=6.72 in/hr

Area (sf)	C	Description
38,316	0.22	Sandy Soil 2-7% per manual (undeveloped)
38,316		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.3	100	0.0560	0.20		**Sheet Flow, Overland Sheet flow** Grass: Dense n= 0.240 P2= 4.20"
0.9	64	0.0320	1.25		**Shallow Concentrated Flow, Overland Concentrated Flow** Short Grass Pasture Kv= 7.0 fps
9.2	164	Total			

Subcatchment A2: DRAINAGE BASIN A2

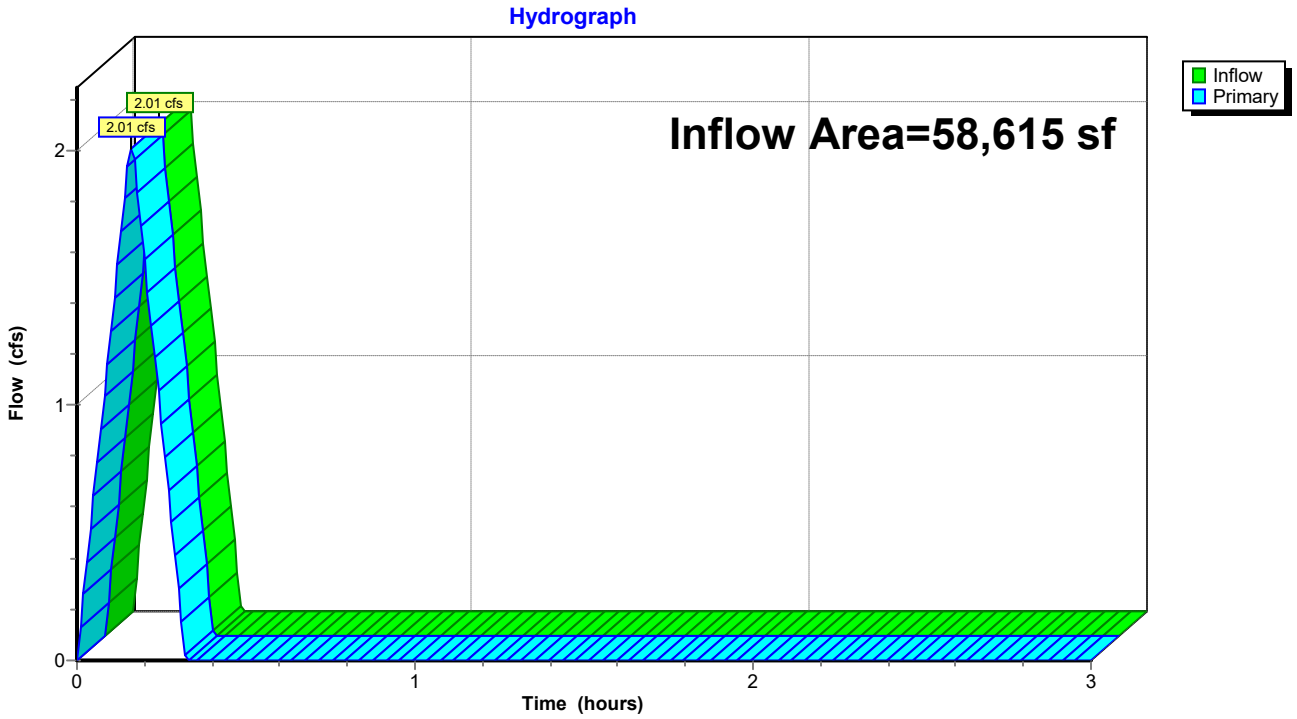


Summary for Link PRE-DEV: Pre-Development

Inflow Area = 58,615 sf, 0.00% Impervious, Inflow Depth = 0.25" for 25-yr event
Inflow = 2.01 cfs @ 0.16 hrs, Volume= 1,203 cf
Primary = 2.01 cfs @ 0.16 hrs, Volume= 1,203 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs

Link PRE-DEV: Pre-Development



Summary for Subcatchment A1: DRAINAGE BASIN A1

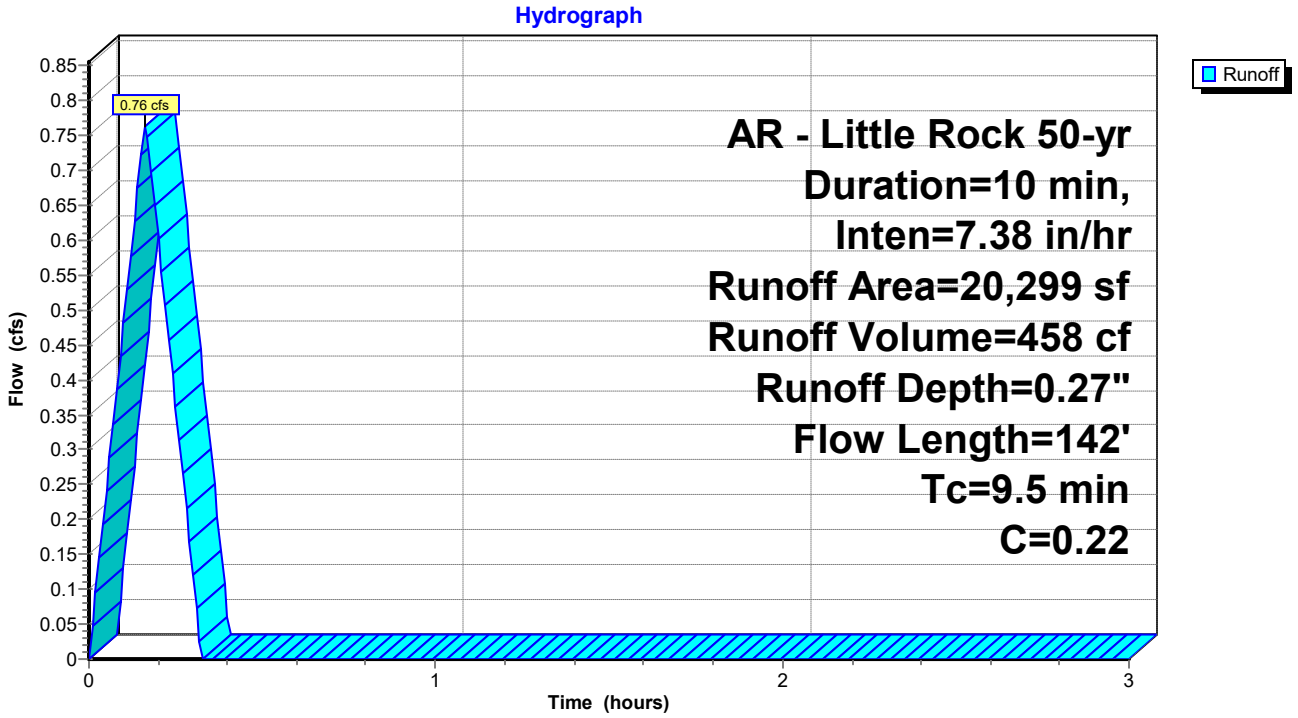
Runoff = 0.76 cfs @ 0.16 hrs, Volume= 458 cf, Depth= 0.27"
Routed to Link PRE-DEV : Pre-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 50-yr Duration=10 min, Inten=7.38 in/hr

Area (sf)	C	Description
20,299	0.22	Sandy Soil 2-7% per manual (undeveloped)
20,299		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.2	100	0.0430	0.18		**Sheet Flow, Overland Sheet flow** Grass: Dense n= 0.240 P2= 4.20"
0.3	42	0.1410	2.63		**Shallow Concentrated Flow, Overland Concentrated Flow** Short Grass Pasture Kv= 7.0 fps
9.5	142	Total			

Subcatchment A1: DRAINAGE BASIN A1



Summary for Subcatchment A2: DRAINAGE BASIN A2

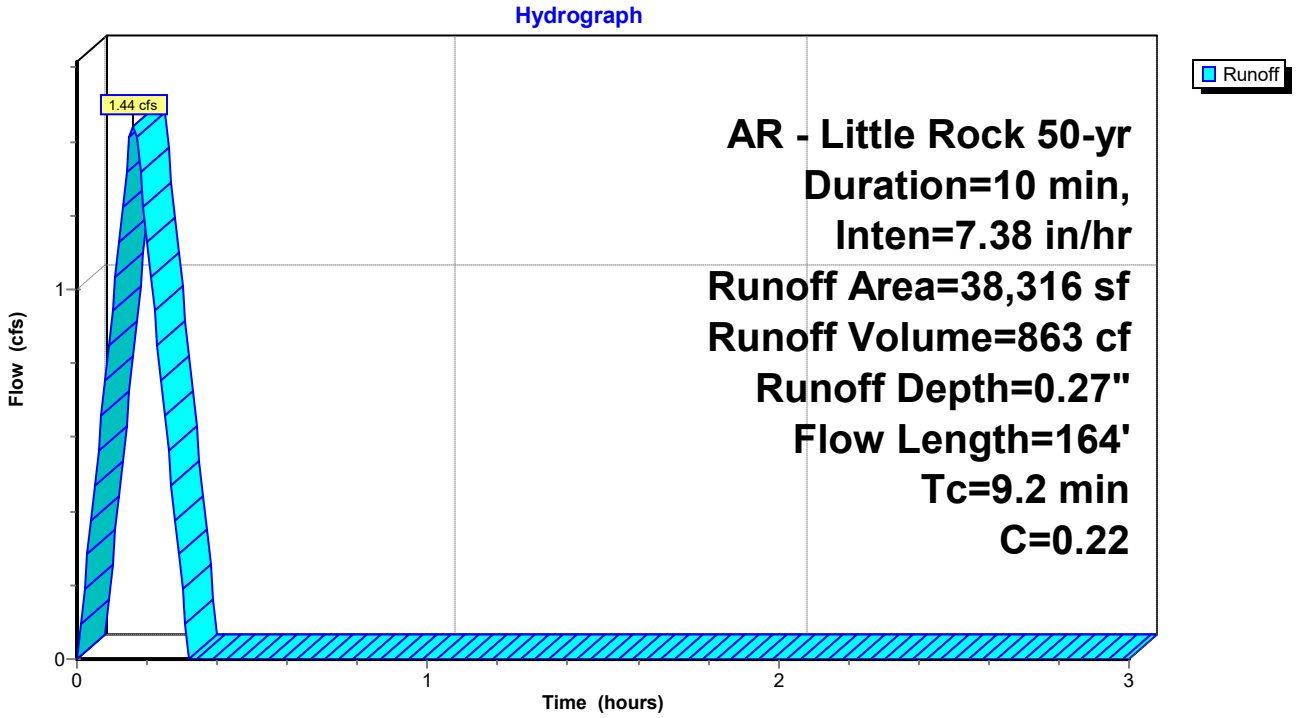
Runoff = 1.44 cfs @ 0.16 hrs, Volume= 863 cf, Depth= 0.27"
Routed to Link PRE-DEV : Pre-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 50-yr Duration=10 min, Inten=7.38 in/hr

Area (sf)	C	Description
38,316	0.22	Sandy Soil 2-7% per manual (undeveloped)
38,316		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.3	100	0.0560	0.20		**Sheet Flow, Overland Sheet flow** Grass: Dense n= 0.240 P2= 4.20"
0.9	64	0.0320	1.25		**Shallow Concentrated Flow, Overland Concentrated Flow** Short Grass Pasture Kv= 7.0 fps
9.2	164	Total			

Subcatchment A2: DRAINAGE BASIN A2

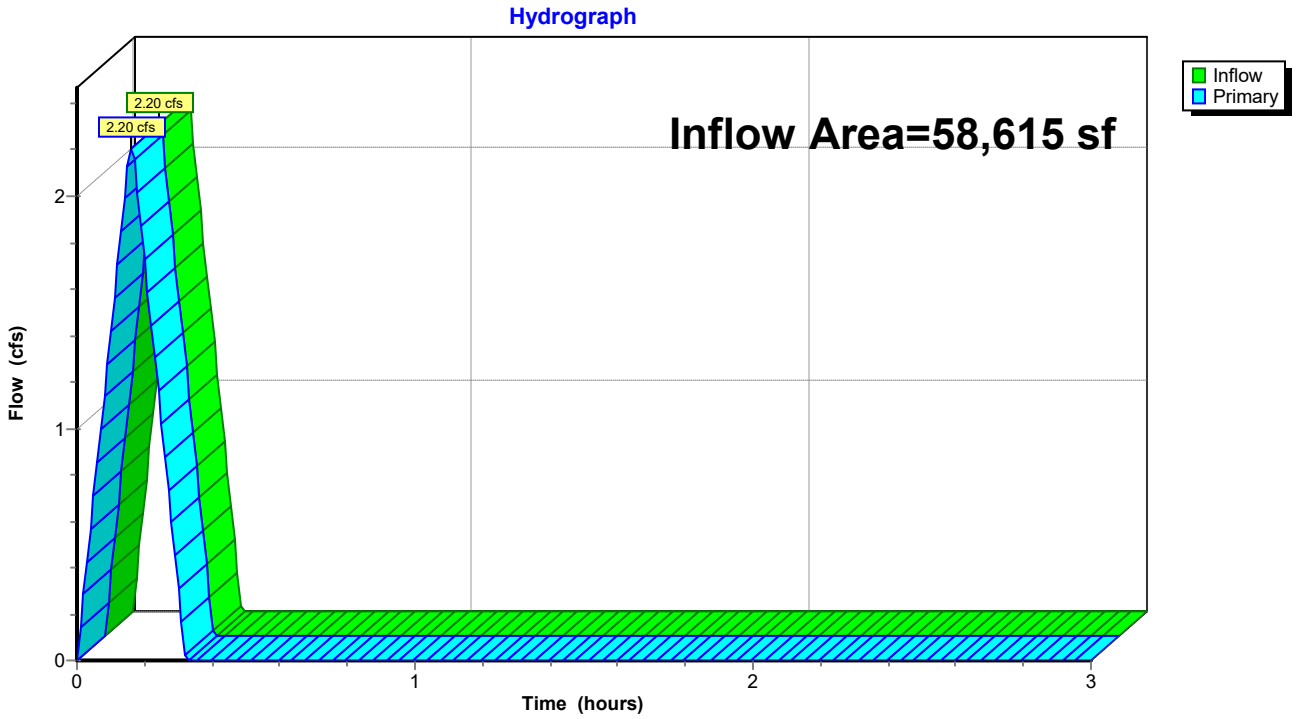


Summary for Link PRE-DEV: Pre-Development

Inflow Area = 58,615 sf, 0.00% Impervious, Inflow Depth = 0.27" for 50-yr event
Inflow = 2.20 cfs @ 0.16 hrs, Volume= 1,321 cf
Primary = 2.20 cfs @ 0.16 hrs, Volume= 1,321 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs

Link PRE-DEV: Pre-Development



Summary for Subcatchment A1: DRAINAGE BASIN A1

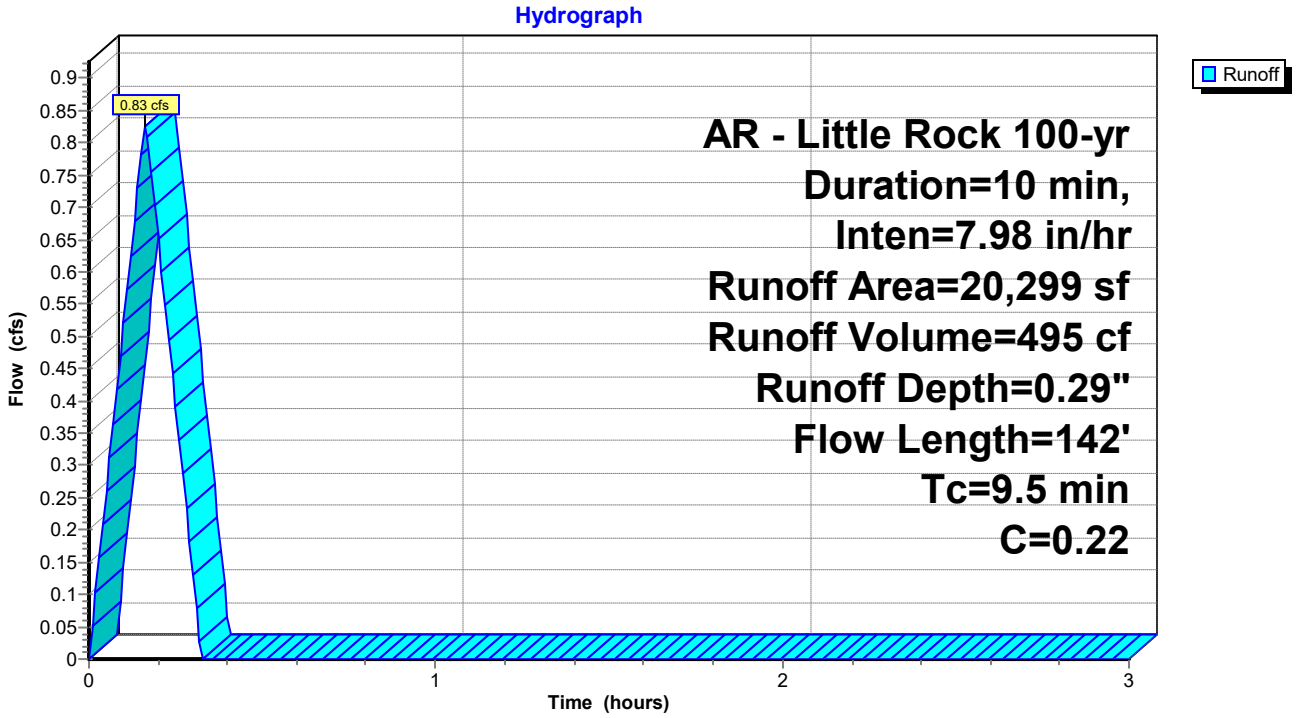
Runoff = 0.83 cfs @ 0.16 hrs, Volume= 495 cf, Depth= 0.29"
Routed to Link PRE-DEV : Pre-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 100-yr Duration=10 min, Inten=7.98 in/hr

Area (sf)	C	Description
20,299	0.22	Sandy Soil 2-7% per manual (undeveloped)
20,299		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.2	100	0.0430	0.18		**Sheet Flow, Overland Sheet flow** Grass: Dense n= 0.240 P2= 4.20"
0.3	42	0.1410	2.63		**Shallow Concentrated Flow, Overland Concentrated Flow** Short Grass Pasture Kv= 7.0 fps
9.5	142	Total			

Subcatchment A1: DRAINAGE BASIN A1



Summary for Subcatchment A2: DRAINAGE BASIN A2

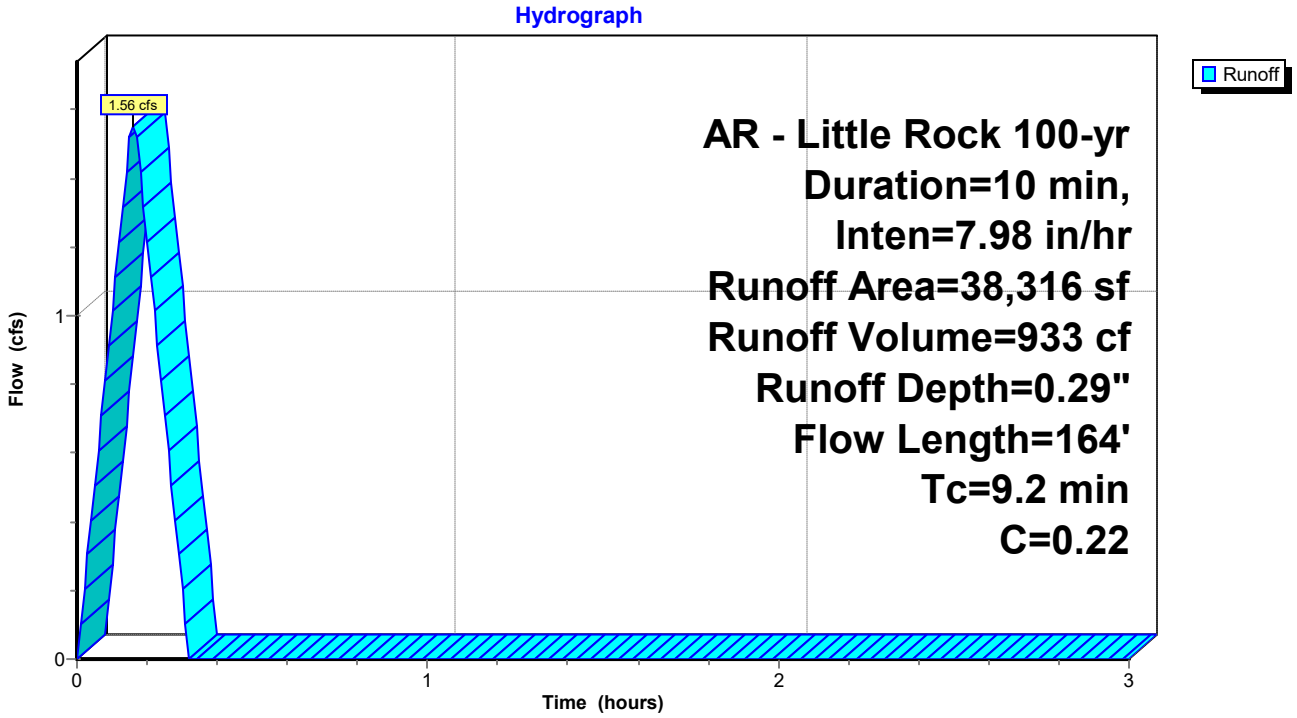
Runoff = 1.56 cfs @ 0.16 hrs, Volume= 933 cf, Depth= 0.29"
Routed to Link PRE-DEV : Pre-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 100-yr Duration=10 min, Inten=7.98 in/hr

Area (sf)	C	Description
38,316	0.22	Sandy Soil 2-7% per manual (undeveloped)
38,316		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.3	100	0.0560	0.20		**Sheet Flow, Overland Sheet flow** Grass: Dense n= 0.240 P2= 4.20"
0.9	64	0.0320	1.25		**Shallow Concentrated Flow, Overland Concentrated Flow** Short Grass Pasture Kv= 7.0 fps
9.2	164	Total			

Subcatchment A2: DRAINAGE BASIN A2

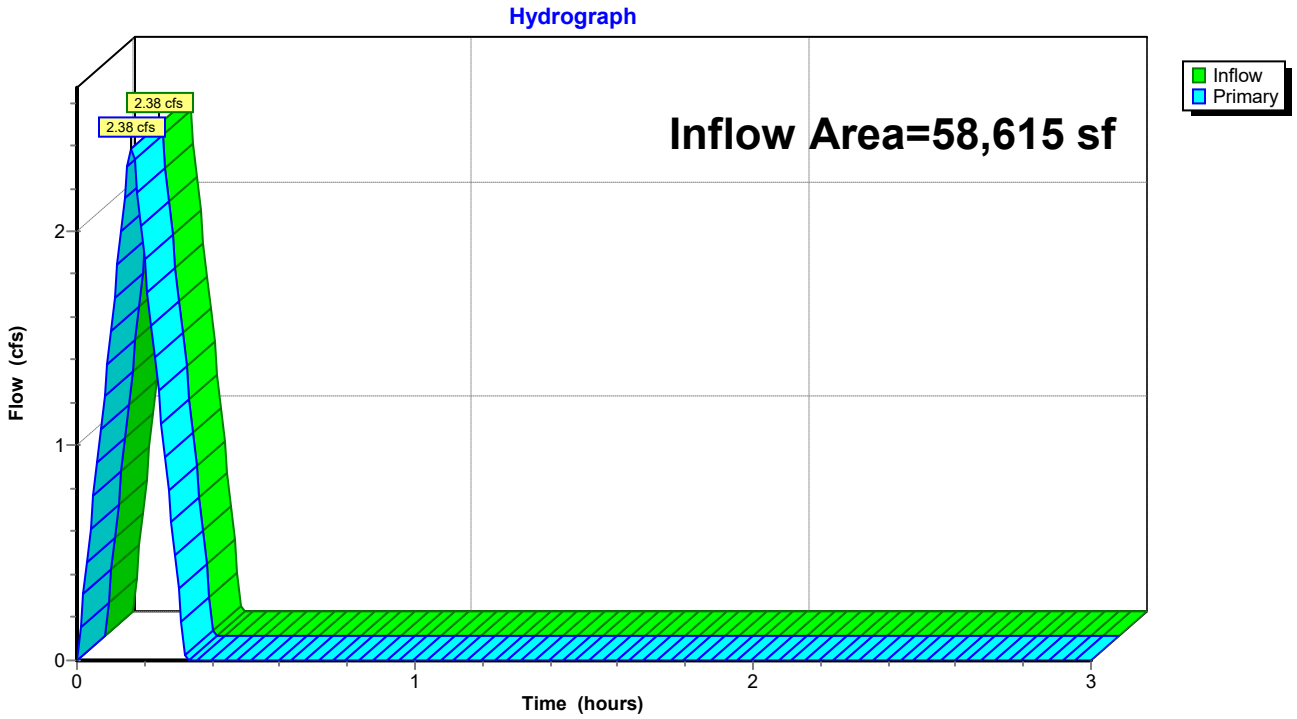


Summary for Link PRE-DEV: Pre-Development

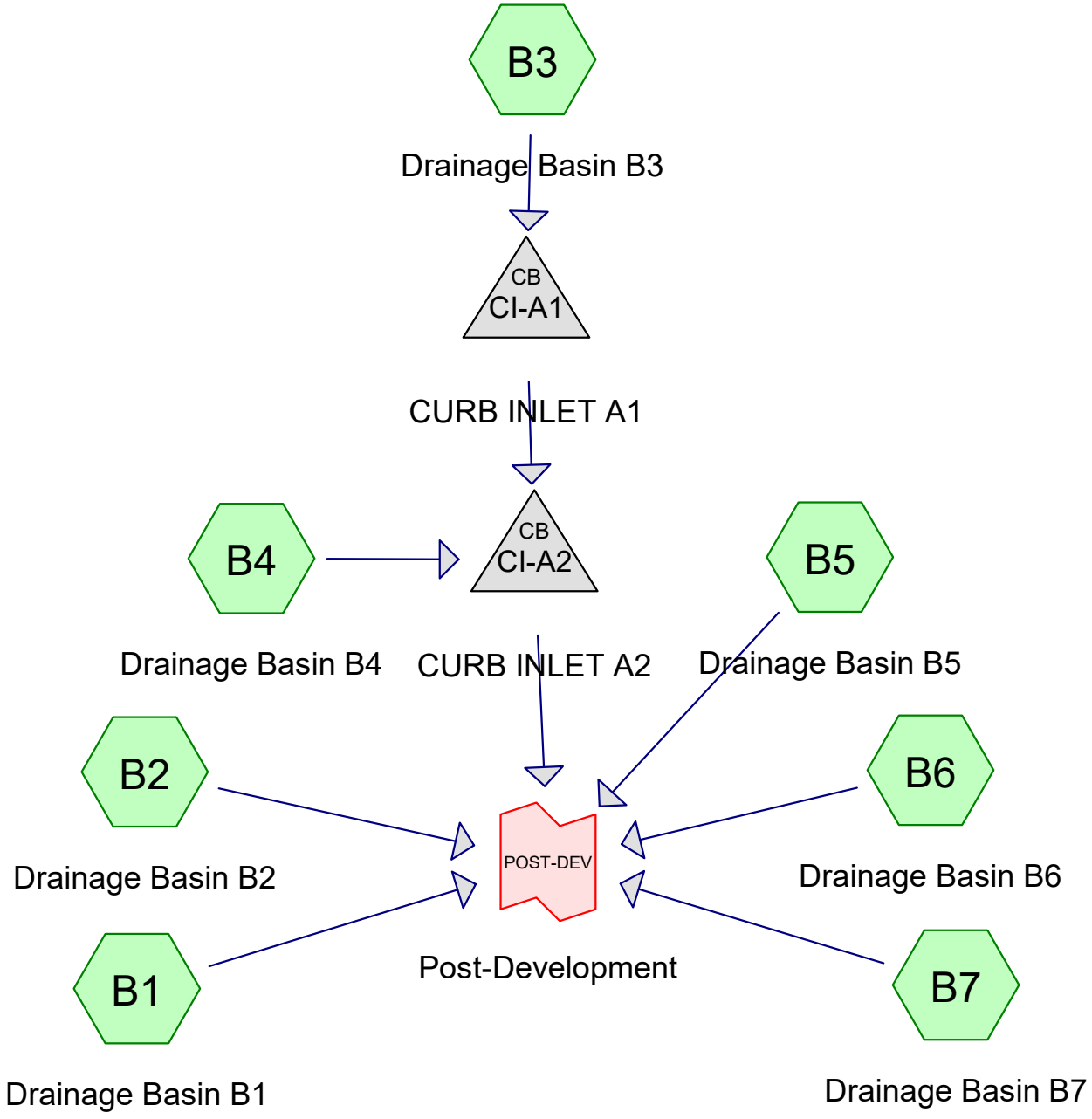
Inflow Area = 58,615 sf, 0.00% Impervious, Inflow Depth = 0.29" for 100-yr event
Inflow = 2.38 cfs @ 0.16 hrs, Volume= 1,428 cf
Primary = 2.38 cfs @ 0.16 hrs, Volume= 1,428 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs

Link PRE-DEV: Pre-Development



POST DEVELOPMENT HYDROGRAPHS



Subcat Reach Pond Link

Routing Diagram for New Beginnings Drainage
Prepared by Phillip Lewis Engineering, Printed 7/24/2025
HydroCAD® 10.20-6a s/n 12520 © 2024 HydroCAD Software Solutions LLC

Summary for Subcatchment B1: Drainage Basin B1

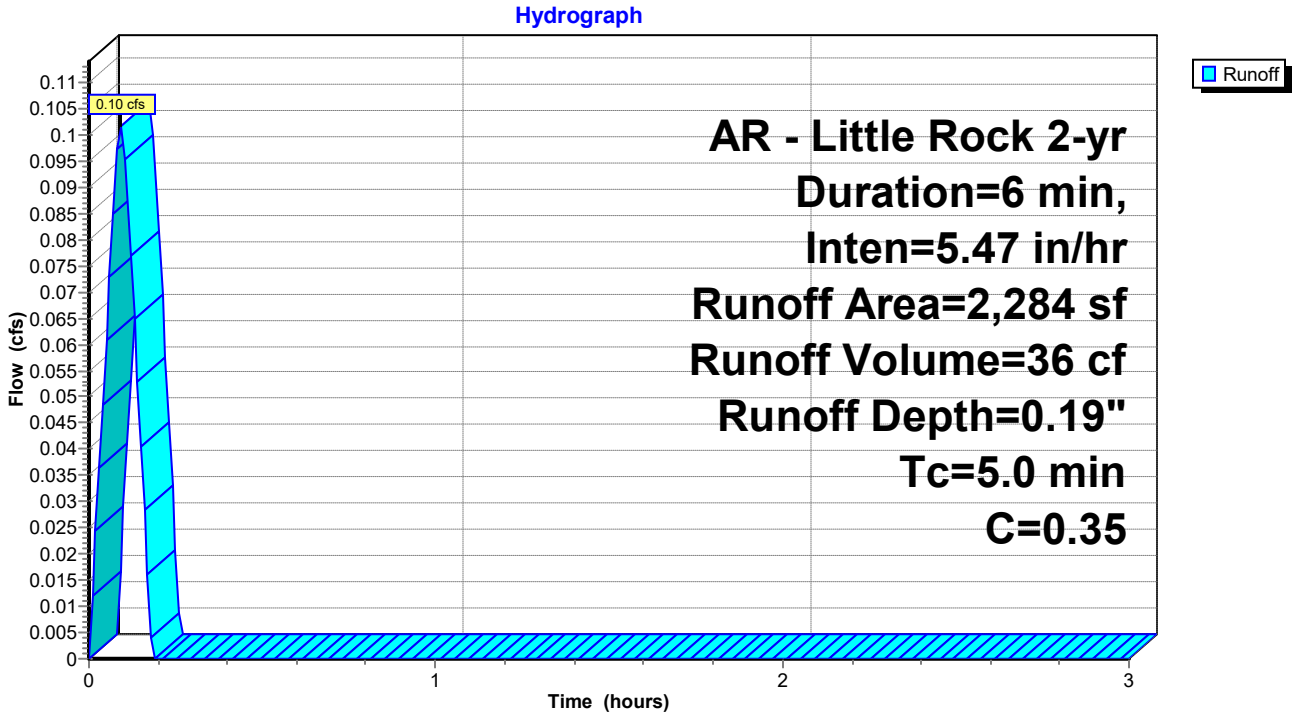
Runoff = 0.10 cfs @ 0.09 hrs, Volume= 36 cf, Depth= 0.19"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 2-yr Duration=6 min, Inten=5.47 in/hr

Area (sf)	C	Description
2,284	0.35	Sandy Soil 2-7% per manual
0	0.92	Paved Areas
2,284	0.35	Weighted Average
2,284		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry, Minimum Adjustment**

Subcatchment B1: Drainage Basin B1



Summary for Subcatchment B2: Drainage Basin B2

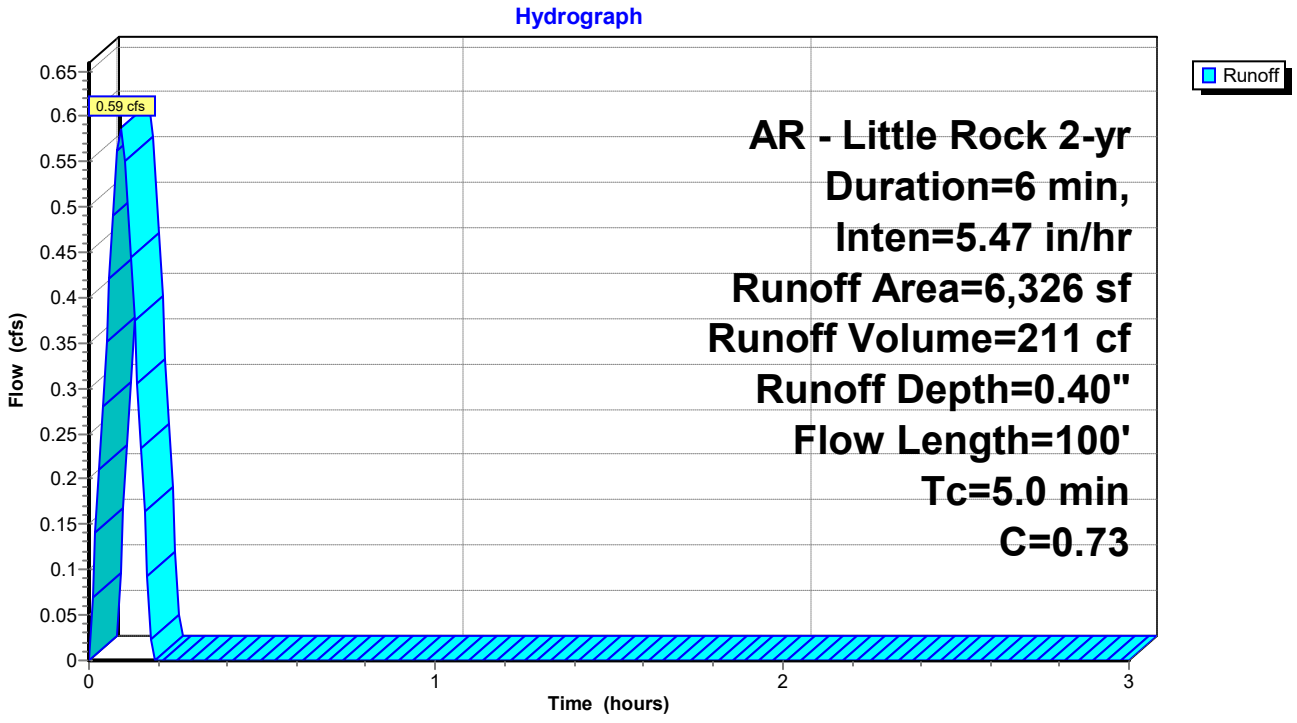
Runoff = 0.59 cfs @ 0.09 hrs, Volume= 211 cf, Depth= 0.40"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 2-yr Duration=6 min, Inten=5.47 in/hr

Area (sf)	C	Description
2,115	0.35	Sandy Soil 2-7% per manual
4,211	0.92	Paved Areas
6,326	0.73	Weighted Average
6,326		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	42	0.1667	3.09		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.5	58	0.0500	2.04		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
4.3					**Direct Entry, Minimum Adjustment**
5.0	100	Total			

Subcatchment B2: Drainage Basin B2



Summary for Subcatchment B3: Drainage Basin B3

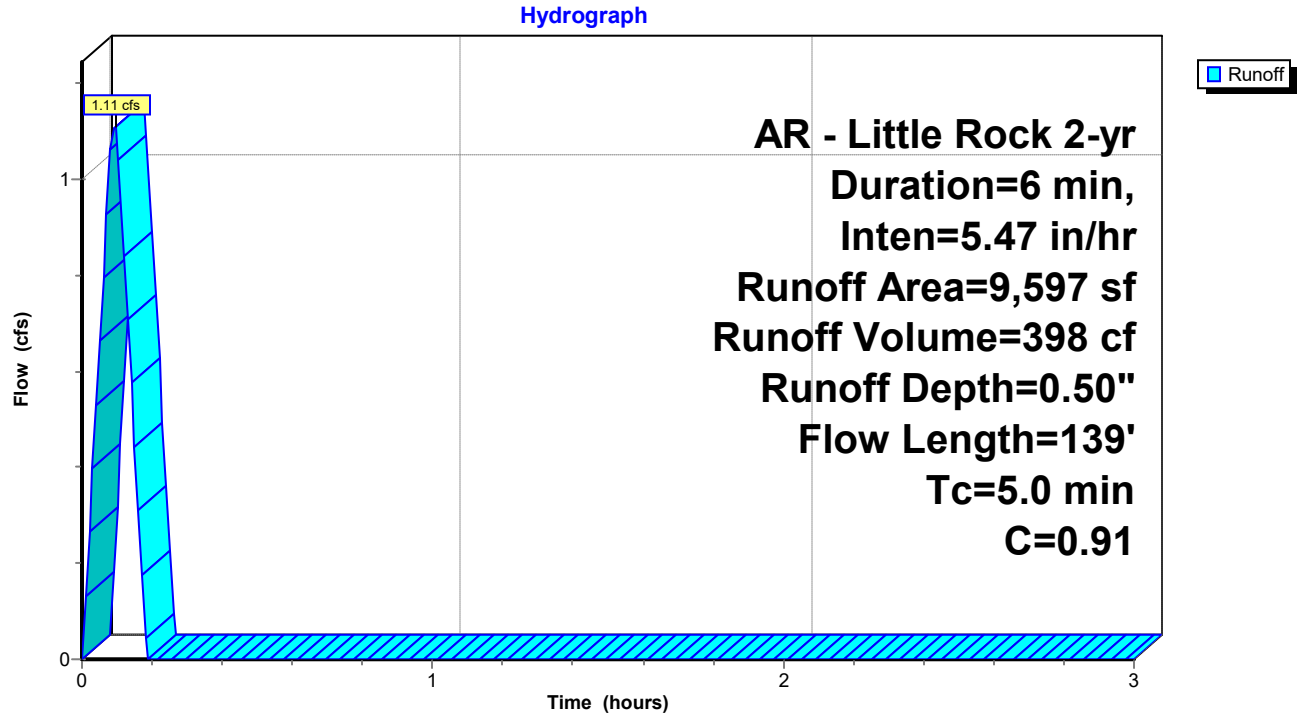
Runoff = 1.11 cfs @ 0.09 hrs, Volume= 398 cf, Depth= 0.50"
Routed to Pond CI-A1 : CURB INLET A1

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 2-yr Duration=6 min, Inten=5.47 in/hr

Area (sf)	C	Description
155	0.35	Sandy Soil 2-7% per manual
9,442	0.92	Paved Areas
9,597	0.91	Weighted Average
9,597		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	28	0.1667	2.85		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	30	0.0160	1.13		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.4	41	0.0520	1.93		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.2	40	0.0360	3.85		**Shallow Concentrated Flow, Gutter Flow** Paved Kv= 20.3 fps
3.8					**Direct Entry, Minimum Adjustment**
5.0	139	Total			

Subcatchment B3: Drainage Basin B3



Summary for Subcatchment B4: Drainage Basin B4

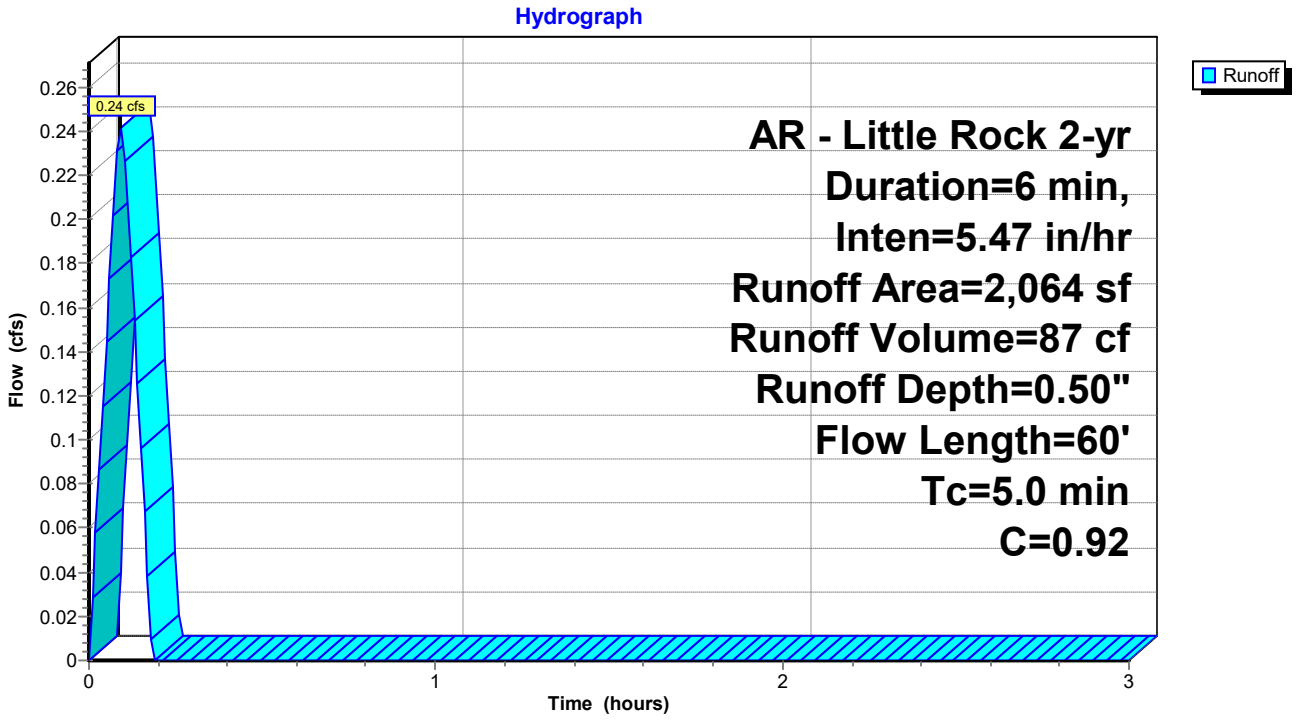
Runoff = 0.24 cfs @ 0.09 hrs, Volume= 87 cf, Depth= 0.50"
Routed to Pond CI-A2 : CURB INLET A2

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 2-yr Duration=6 min, Inten=5.47 in/hr

Area (sf)	C	Description
0	0.35	Sandy Soil 2-7% per manual
2,064	0.92	Paved Areas
2,064	0.92	Weighted Average
2,064		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	45	0.0170	1.26		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.0	15	0.0840	5.88		**Shallow Concentrated Flow, Gutter Flow** Paved Kv= 20.3 fps
4.4					**Direct Entry, Minimum Adjustment**
5.0	60	Total			

Subcatchment B4: Drainage Basin B4



Summary for Subcatchment B5: Drainage Basin B5

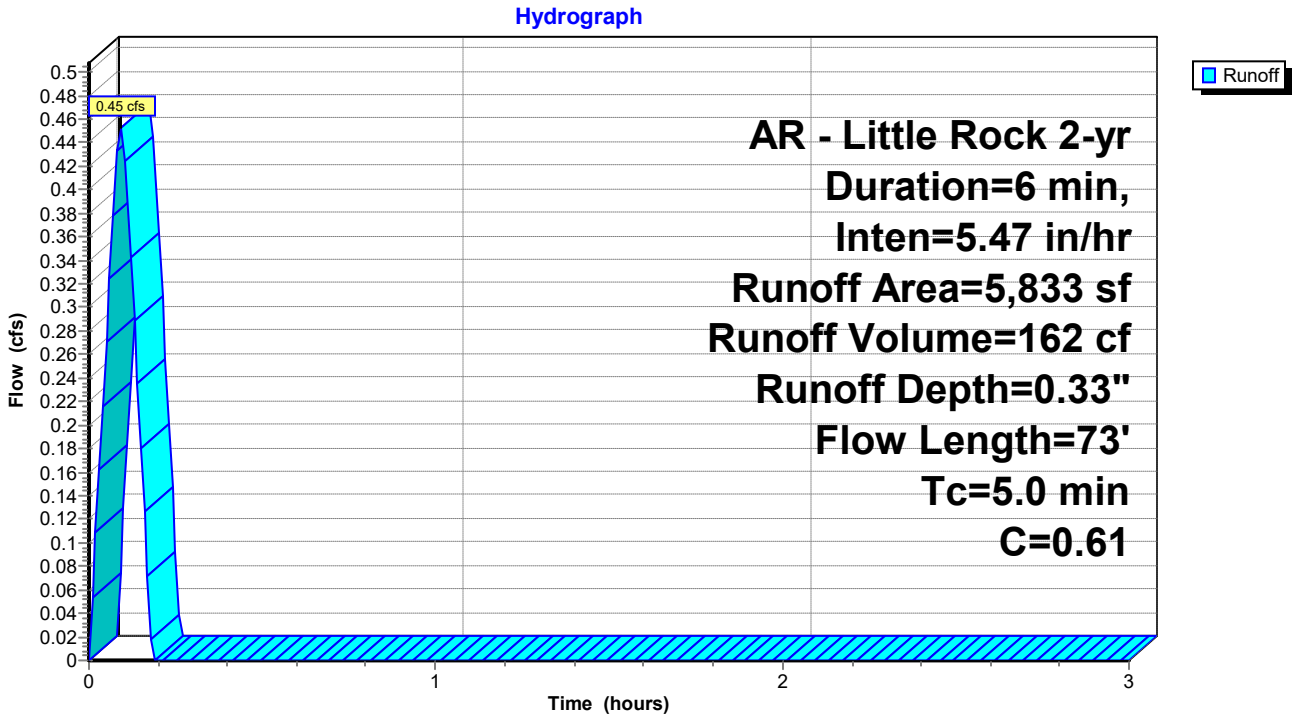
Runoff = 0.45 cfs @ 0.09 hrs, Volume= 162 cf, Depth= 0.33"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 2-yr Duration=6 min, Inten=5.47 in/hr

Area (sf)	C	Description
3,123	0.35	Sandy Soil 2-7% per manual
2,710	0.92	Paved Areas
5,833	0.61	Weighted Average
5,833		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	18	0.1667	2.61		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	55	0.0860	2.05		**Shallow Concentrated Flow, Overland Concentrated** Short Grass Pasture Kv= 7.0 fps
4.5					**Direct Entry, Minimum Adjustment**
5.0	73	Total			

Subcatchment B5: Drainage Basin B5



Summary for Subcatchment B6: Drainage Basin B6

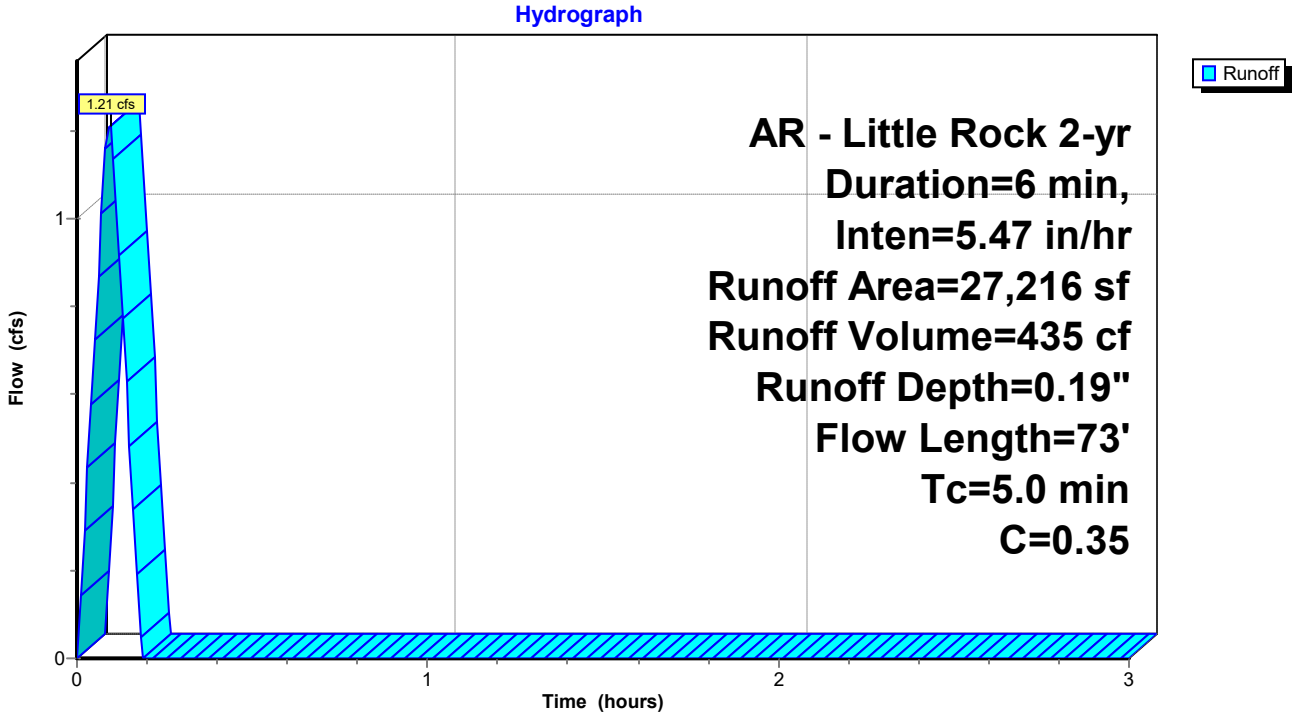
Runoff = 1.21 cfs @ 0.09 hrs, Volume= 435 cf, Depth= 0.19"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 2-yr Duration=6 min, Inten=5.47 in/hr

Area (sf)	C	Description
27,216	0.35	Sandy Soil 2-7% per manual
27,216		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	18	0.1667	2.61		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	55	0.0860	2.05		**Shallow Concentrated Flow, Overland Concentrated** Short Grass Pasture Kv= 7.0 fps
4.5					**Direct Entry, Minimum Adjustment**
5.0	73	Total			

Subcatchment B6: Drainage Basin B6



Summary for Subcatchment B7: Drainage Basin B7

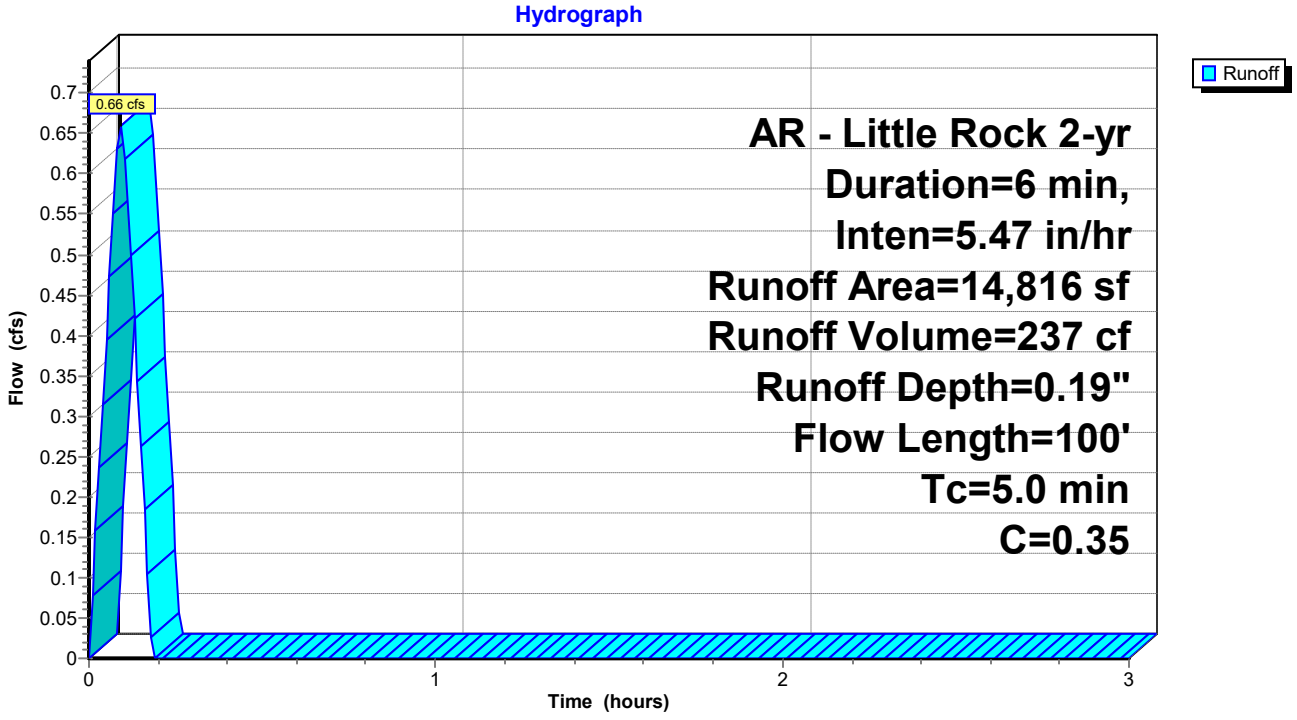
Runoff = 0.66 cfs @ 0.09 hrs, Volume= 237 cf, Depth= 0.19"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 2-yr Duration=6 min, Inten=5.47 in/hr

Area (sf)	C	Description
14,816	0.35	Sandy Soil 2-7% per manual
14,816		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	42	0.1667	3.09		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.5	58	0.0500	2.04		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
4.3					**Direct Entry, Minimum Adjustment**
5.0	100	Total			

Subcatchment B7: Drainage Basin B7



Summary for Pond CI-A1: CURB INLET A1

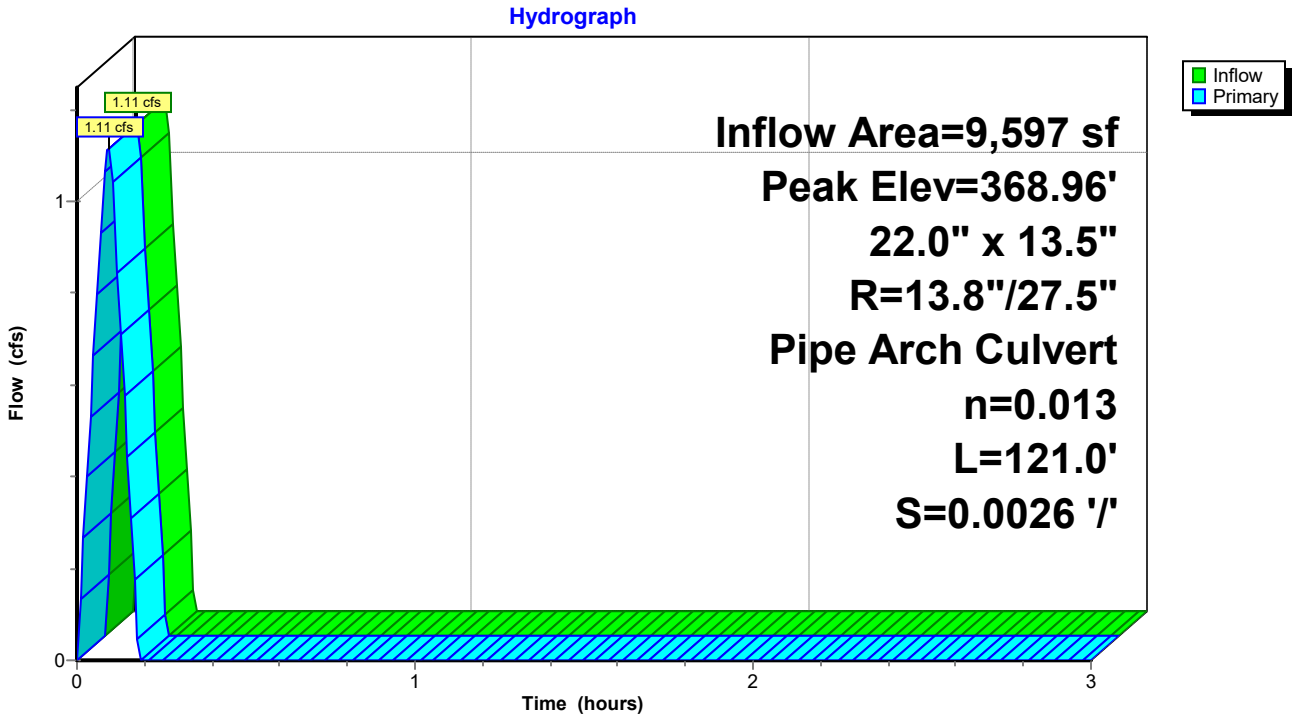
Inflow Area = 9,597 sf, 0.00% Impervious, Inflow Depth = 0.50" for 2-yr event
Inflow = 1.11 cfs @ 0.09 hrs, Volume= 398 cf
Outflow = 1.11 cfs @ 0.09 hrs, Volume= 398 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.11 cfs @ 0.09 hrs, Volume= 398 cf
Routed to Pond CI-A2 : CURB INLET A2

Routing by Stor-Ind method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Peak Elev= 368.96' @ 0.09 hrs

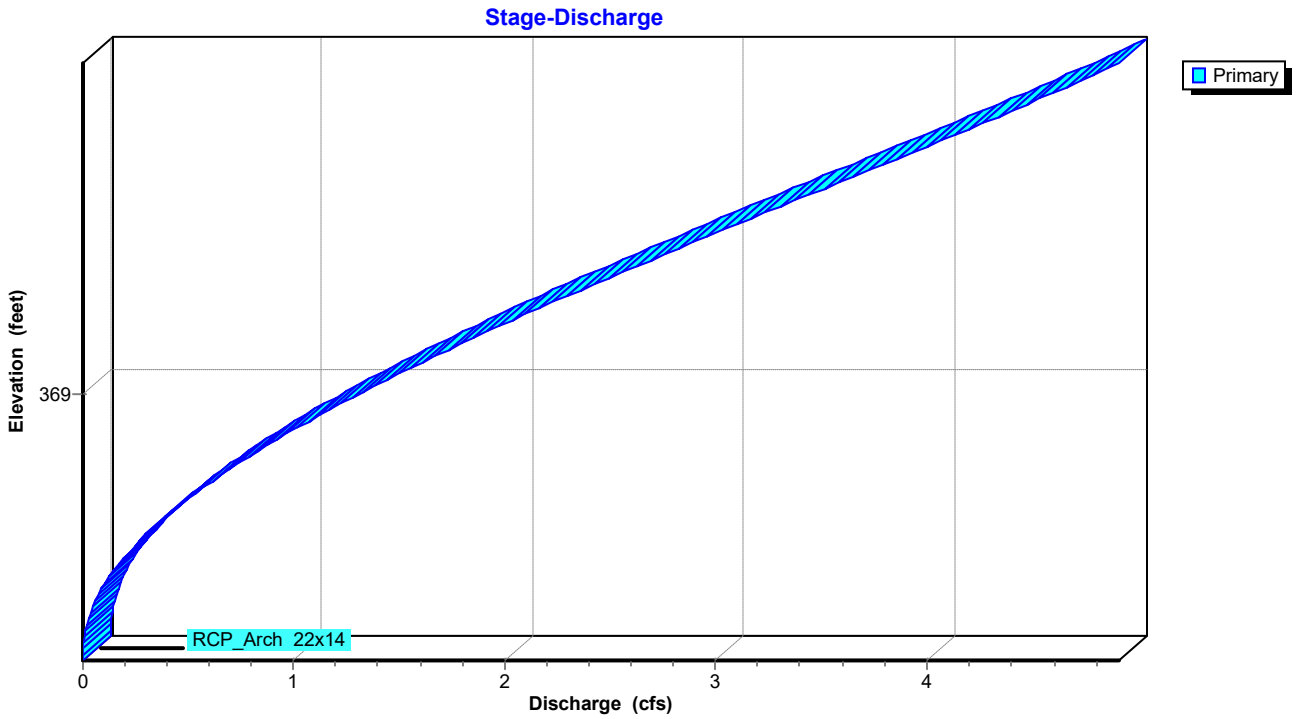
Device	Routing	Invert	Outlet Devices
#1	Primary	368.50'	**22.0" W x 13.5" H, R=13.8"/27.5" Pipe Arch RCP_Arch 22x14** L= 121.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 368.50' / 368.19' S= 0.0026 '/' Cc= 0.900 n= 0.013, Flow Area= 1.65 sf

Primary OutFlow Max=1.11 cfs @ 0.09 hrs HW=368.96' (Free Discharge)
1=RCP_Arch 22x14 (Barrel Controls 1.11 cfs @ 2.30 fps)

Pond CI-A1: CURB INLET A1



Pond CI-A1: CURB INLET A1



Stage-Area-Storage for Pond CI-A1: CURB INLET A1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
368.50	**0**	369.02	0	369.54	0
368.51	0	369.03	0	369.55	0
368.52	0	369.04	0	369.56	0
368.53	0	369.05	0	369.57	0
368.54	0	369.06	0	369.58	0
368.55	0	369.07	0	369.59	0
368.56	0	369.08	0	369.60	0
368.57	0	369.09	0	369.61	0
368.58	0	369.10	0	369.62	0
368.59	0	369.11	0		
368.60	0	369.12	0		
368.61	0	369.13	0		
368.62	0	369.14	0		
368.63	0	369.15	0		
368.64	0	369.16	0		
368.65	0	369.17	0		
368.66	0	369.18	0		
368.67	0	369.19	0		
368.68	0	369.20	0		
368.69	0	369.21	0		
368.70	0	369.22	0		
368.71	0	369.23	0		
368.72	0	369.24	0		
368.73	0	369.25	0		
368.74	0	369.26	0		
368.75	0	369.27	0		
368.76	0	369.28	0		
368.77	0	369.29	0		
368.78	0	369.30	0		
368.79	0	369.31	0		
368.80	0	369.32	0		
368.81	0	369.33	0		
368.82	0	369.34	0		
368.83	0	369.35	0		
368.84	0	369.36	0		
368.85	0	369.37	0		
368.86	0	369.38	0		
368.87	0	369.39	0		
368.88	0	369.40	0		
368.89	0	369.41	0		
368.90	0	369.42	0		
368.91	0	369.43	0		
368.92	0	369.44	0		
368.93	0	369.45	0		
368.94	0	369.46	0		
368.95	0	369.47	0		
368.96	0	369.48	0		
368.97	0	369.49	0		
368.98	0	369.50	0		
368.99	0	369.51	0		
369.00	0	369.52	0		
369.01	0	369.53	0		

Summary for Pond CI-A2: CURB INLET A2

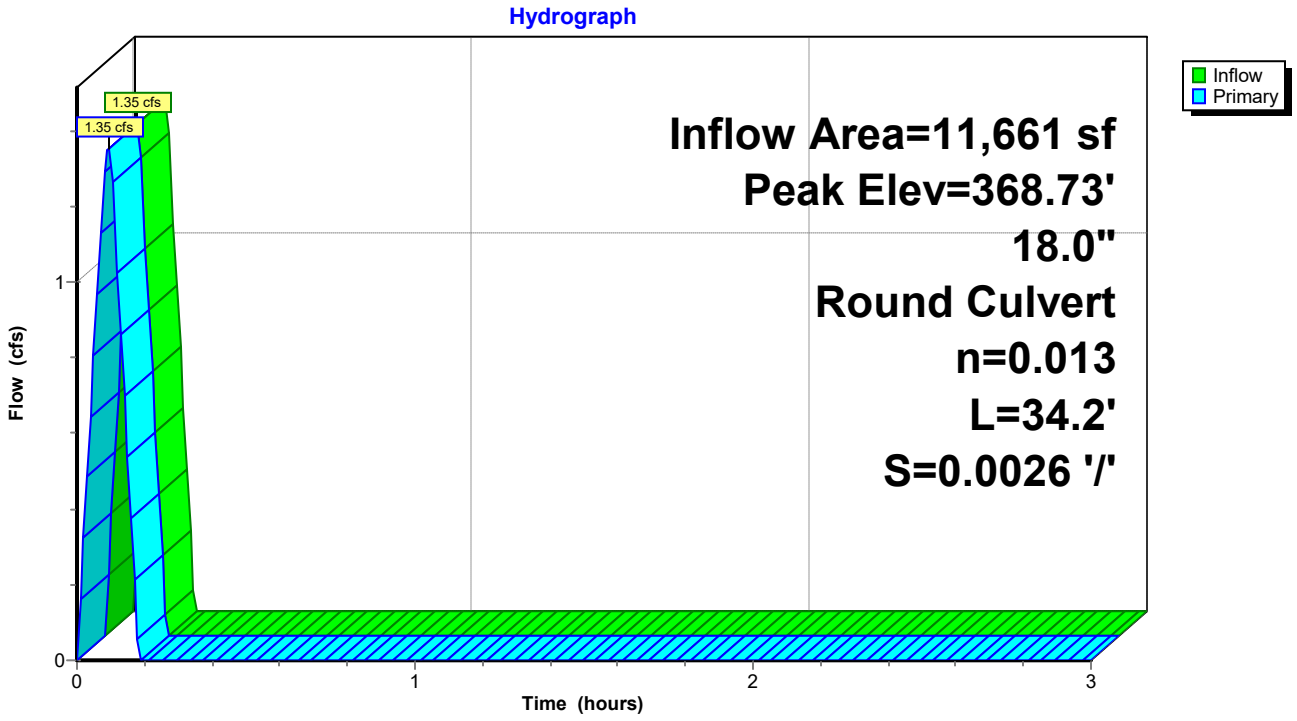
Inflow Area = 11,661 sf, 0.00% Impervious, Inflow Depth = 0.50" for 2-yr event
Inflow = 1.35 cfs @ 0.09 hrs, Volume= 485 cf
Outflow = 1.35 cfs @ 0.09 hrs, Volume= 485 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.35 cfs @ 0.09 hrs, Volume= 485 cf
Routed to Link POST-DEV : Post-Development

Routing by Stor-Ind method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Peak Elev= 368.73' @ 0.09 hrs

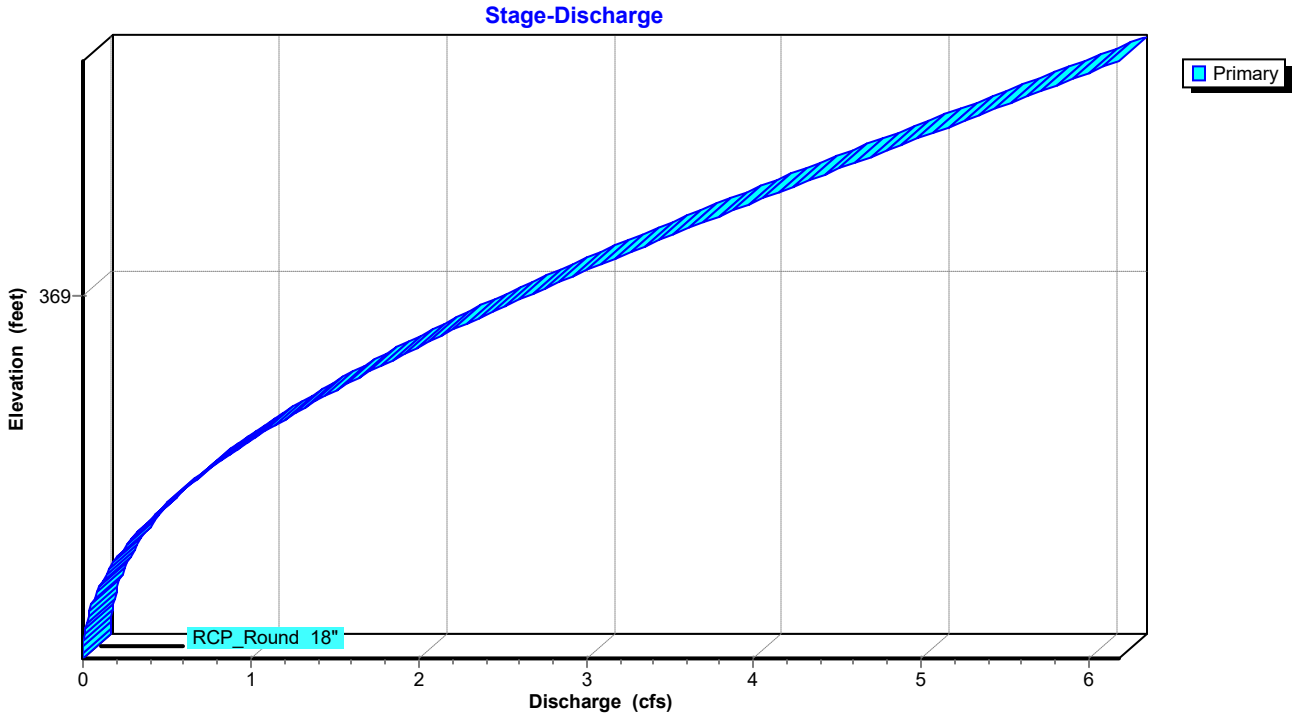
Device	Routing	Invert	Outlet Devices
#1	Primary	368.09'	**18.0" Round RCP_Round 18"** L= 34.2' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 368.09' / 368.00' S= 0.0026 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=1.35 cfs @ 0.09 hrs HW=368.73' (Free Discharge)
1=RCP_Round 18" (Barrel Controls 1.35 cfs @ 2.78 fps)

Pond CI-A2: CURB INLET A2



Pond CI-A2: CURB INLET A2



Stage-Area-Storage for Pond CI-A2: CURB INLET A2

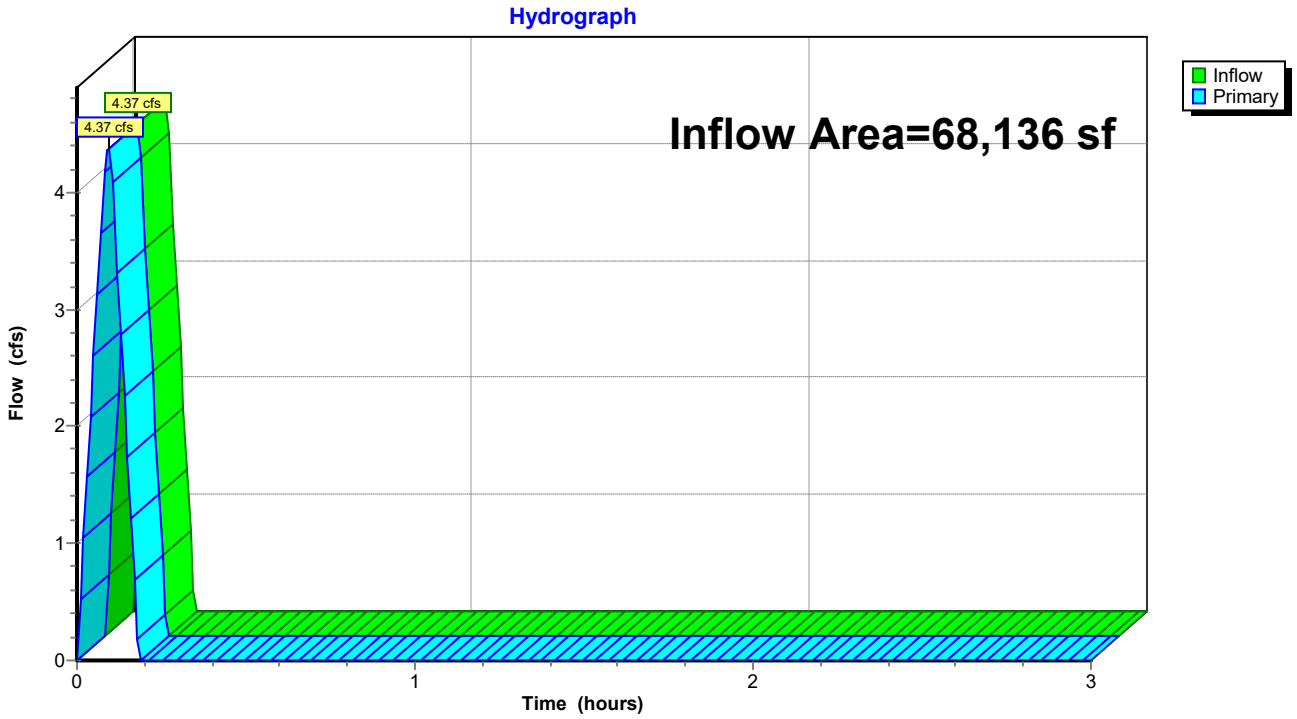
Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
368.09	**0**	368.61	0	369.13	0
368.10	0	368.62	0	369.14	0
368.11	0	368.63	0	369.15	0
368.12	0	368.64	0	369.16	0
368.13	0	368.65	0	369.17	0
368.14	0	368.66	0	369.18	0
368.15	0	368.67	0	369.19	0
368.16	0	368.68	0	369.20	0
368.17	0	368.69	0	369.21	0
368.18	0	368.70	0	369.22	0
368.19	0	368.71	0	369.23	0
368.20	0	368.72	0	369.24	0
368.21	0	368.73	0	369.25	0
368.22	0	368.74	0	369.26	0
368.23	0	368.75	0	369.27	0
368.24	0	368.76	0	369.28	0
368.25	0	368.77	0	369.29	0
368.26	0	368.78	0	369.30	0
368.27	0	368.79	0	369.31	0
368.28	0	368.80	0	369.32	0
368.29	0	368.81	0	369.33	0
368.30	0	368.82	0	369.34	0
368.31	0	368.83	0	369.35	0
368.32	0	368.84	0	369.36	0
368.33	0	368.85	0	369.37	0
368.34	0	368.86	0	369.38	0
368.35	0	368.87	0	369.39	0
368.36	0	368.88	0	369.40	0
368.37	0	368.89	0	369.41	0
368.38	0	368.90	0	369.42	0
368.39	0	368.91	0	369.43	0
368.40	0	368.92	0	369.44	0
368.41	0	368.93	0	369.45	0
368.42	0	368.94	0	369.46	0
368.43	0	368.95	0	369.47	0
368.44	0	368.96	0	369.48	0
368.45	0	368.97	0	369.49	0
368.46	0	368.98	0	369.50	0
368.47	0	368.99	0	369.51	0
368.48	0	369.00	0	369.52	0
368.49	0	369.01	0	369.53	0
368.50	0	369.02	0	369.54	0
368.51	0	369.03	0	369.55	0
368.52	0	369.04	0	369.56	0
368.53	0	369.05	0	369.57	0
368.54	0	369.06	0	369.58	0
368.55	0	369.07	0	369.59	0
368.56	0	369.08	0		
368.57	0	369.09	0		
368.58	0	369.10	0		
368.59	0	369.11	0		
368.60	0	369.12	0		

Summary for Link POST-DEV: Post-Development

Inflow Area = 68,136 sf, 0.00% Impervious, Inflow Depth = 0.28" for 2-yr event
Inflow = 4.37 cfs @ 0.09 hrs, Volume= 1,566 cf
Primary = 4.37 cfs @ 0.09 hrs, Volume= 1,566 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs

Link POST-DEV: Post-Development



Summary for Subcatchment B1: Drainage Basin B1

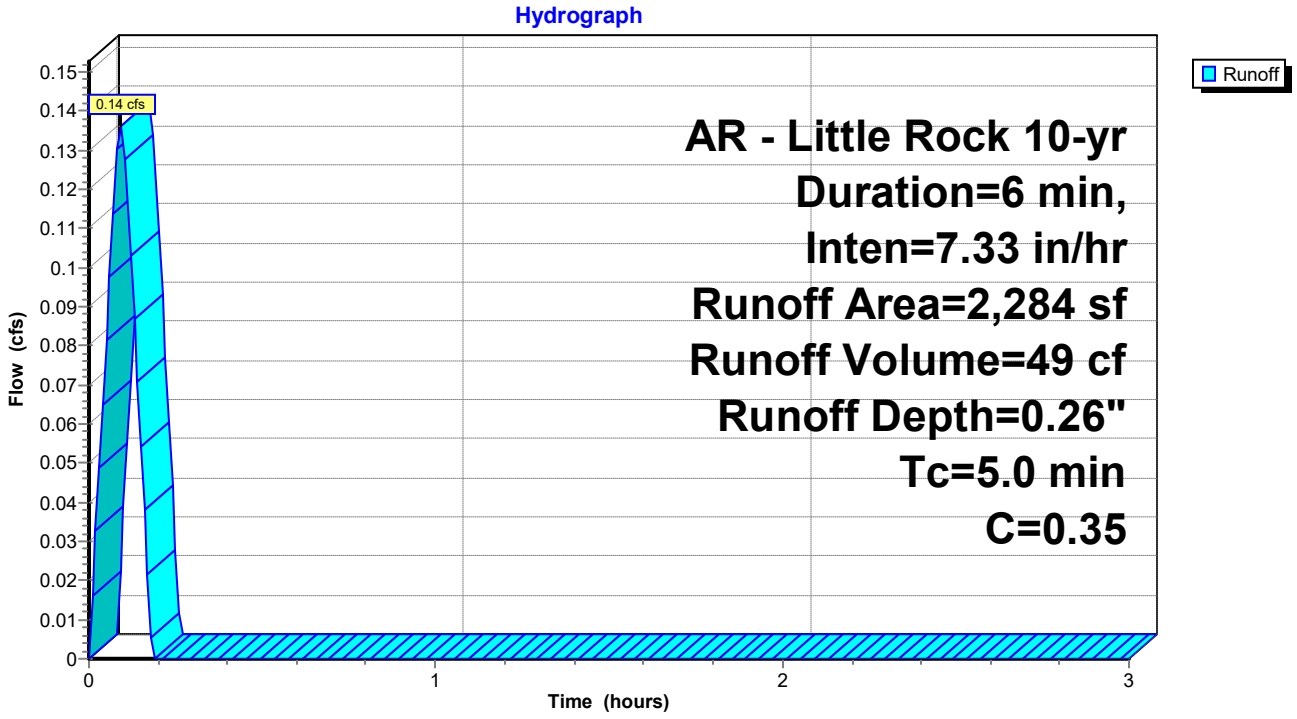
Runoff = 0.14 cfs @ 0.09 hrs, Volume= 49 cf, Depth= 0.26"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 10-yr Duration=6 min, Inten=7.33 in/hr

Area (sf)	C	Description
2,284	0.35	Sandy Soil 2-7% per manual
0	0.92	Paved Areas
2,284	0.35	Weighted Average
2,284		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry, Minimum Adjustment**

Subcatchment B1: Drainage Basin B1



Summary for Subcatchment B2: Drainage Basin B2

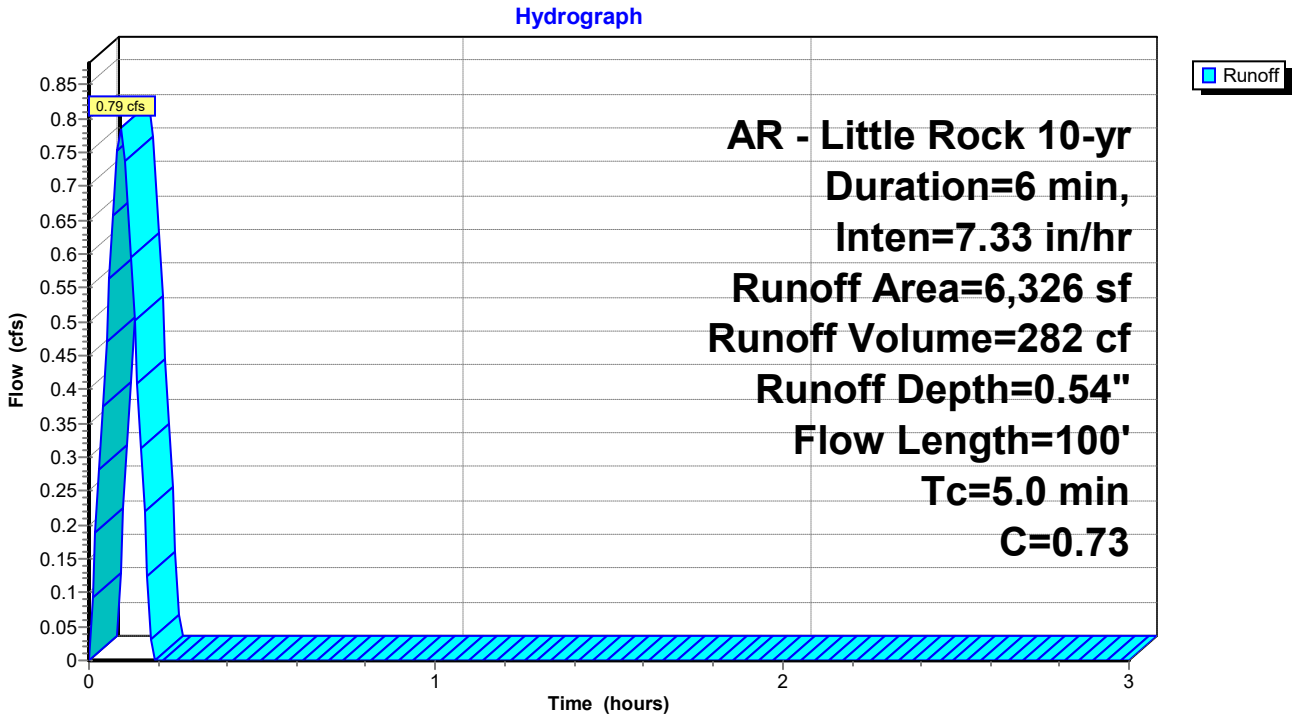
Runoff = 0.79 cfs @ 0.09 hrs, Volume= 282 cf, Depth= 0.54"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 10-yr Duration=6 min, Inten=7.33 in/hr

Area (sf)	C	Description
2,115	0.35	Sandy Soil 2-7% per manual
4,211	0.92	Paved Areas
6,326	0.73	Weighted Average
6,326		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	42	0.1667	3.09		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.5	58	0.0500	2.04		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
4.3					**Direct Entry, Minimum Adjustment**
5.0	100	Total			

Subcatchment B2: Drainage Basin B2



Summary for Subcatchment B3: Drainage Basin B3

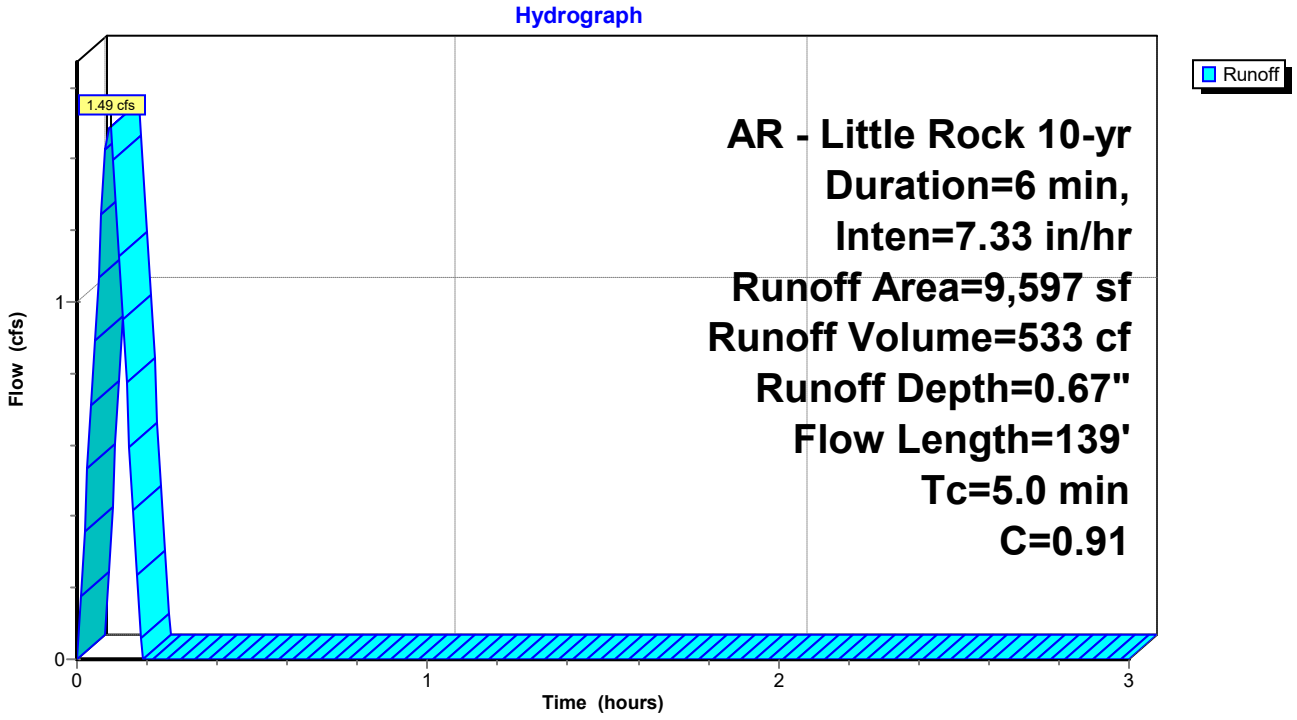
Runoff = 1.49 cfs @ 0.10 hrs, Volume= 533 cf, Depth= 0.67"
Routed to Pond CI-A1 : CURB INLET A1

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 10-yr Duration=6 min, Inten=7.33 in/hr

Area (sf)	C	Description
155	0.35	Sandy Soil 2-7% per manual
9,442	0.92	Paved Areas
9,597	0.91	Weighted Average
9,597		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	28	0.1667	2.85		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	30	0.0160	1.13		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.4	41	0.0520	1.93		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.2	40	0.0360	3.85		**Shallow Concentrated Flow, Gutter Flow** Paved Kv= 20.3 fps
3.8					**Direct Entry, Minimum Adjustment**
5.0	139	Total			

Subcatchment B3: Drainage Basin B3



Summary for Subcatchment B4: Drainage Basin B4

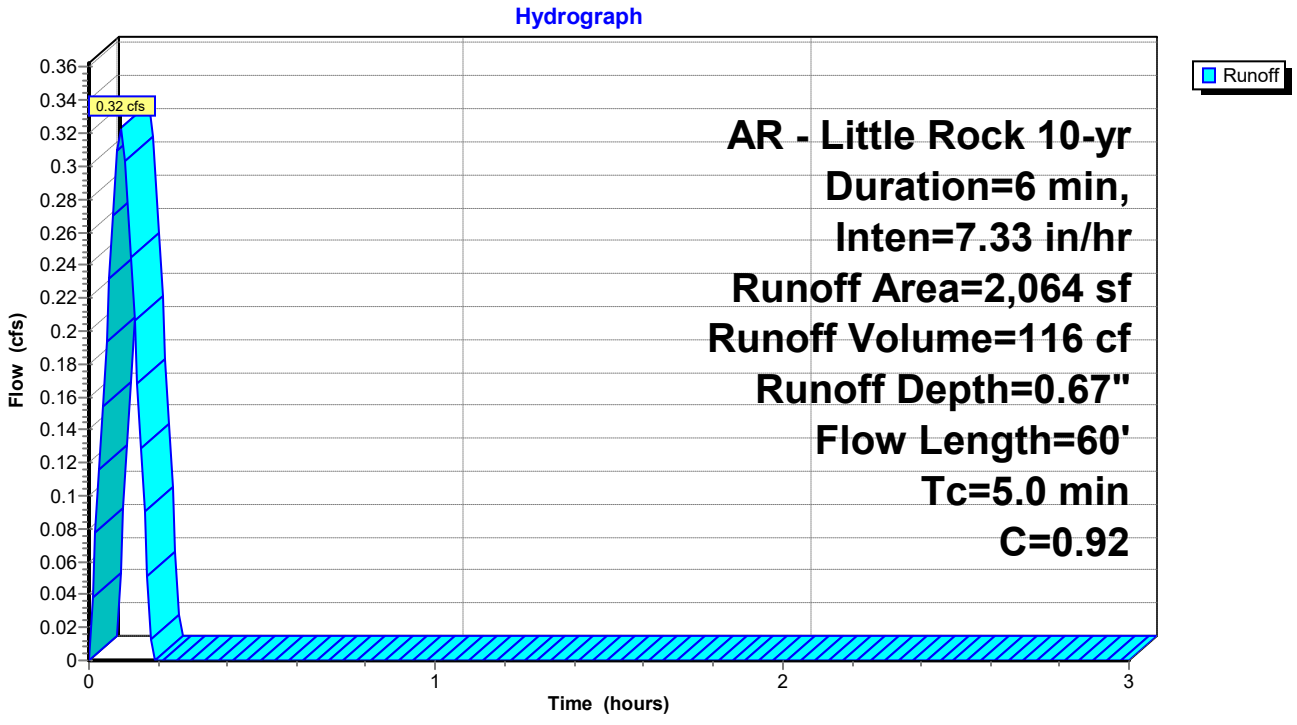
Runoff = 0.32 cfs @ 0.09 hrs, Volume= 116 cf, Depth= 0.67"
Routed to Pond CI-A2 : CURB INLET A2

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 10-yr Duration=6 min, Inten=7.33 in/hr

Area (sf)	C	Description
0	0.35	Sandy Soil 2-7% per manual
2,064	0.92	Paved Areas
2,064	0.92	Weighted Average
2,064		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	45	0.0170	1.26		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.0	15	0.0840	5.88		**Shallow Concentrated Flow, Gutter Flow** Paved Kv= 20.3 fps
4.4					**Direct Entry, Minimum Adjustment**
5.0	60	Total			

Subcatchment B4: Drainage Basin B4



Summary for Subcatchment B5: Drainage Basin B5

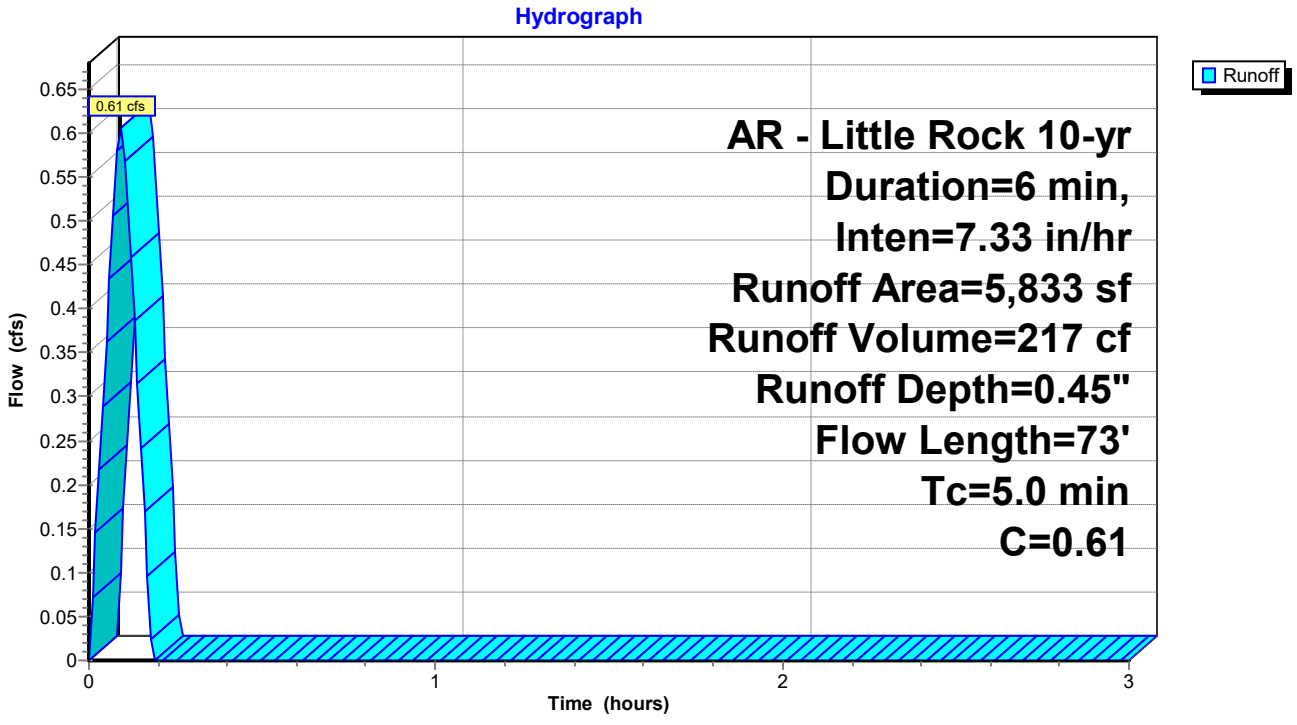
Runoff = 0.61 cfs @ 0.09 hrs, Volume= 217 cf, Depth= 0.45"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 10-yr Duration=6 min, Inten=7.33 in/hr

Area (sf)	C	Description
3,123	0.35	Sandy Soil 2-7% per manual
2,710	0.92	Paved Areas
5,833	0.61	Weighted Average
5,833		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	18	0.1667	2.61		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	55	0.0860	2.05		**Shallow Concentrated Flow, Overland Concentrated** Short Grass Pasture Kv= 7.0 fps
4.5					**Direct Entry, Minimum Adjustment**
5.0	73	Total			

Subcatchment B5: Drainage Basin B5



Summary for Subcatchment B6: Drainage Basin B6

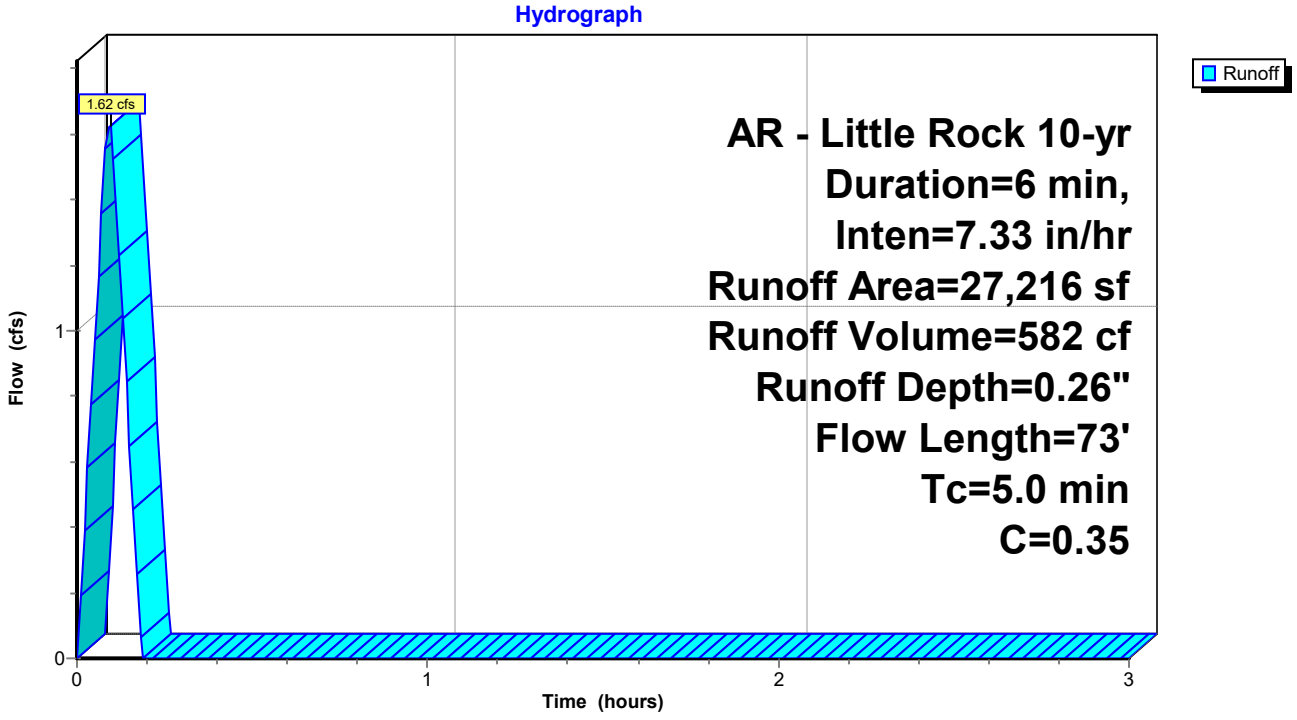
Runoff = 1.62 cfs @ 0.09 hrs, Volume= 582 cf, Depth= 0.26"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 10-yr Duration=6 min, Inten=7.33 in/hr

Area (sf)	C	Description
27,216	0.35	Sandy Soil 2-7% per manual
27,216		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	18	0.1667	2.61		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	55	0.0860	2.05		**Shallow Concentrated Flow, Overland Concentrated** Short Grass Pasture Kv= 7.0 fps
4.5					**Direct Entry, Minimum Adjustment**
5.0	73	Total			

Subcatchment B6: Drainage Basin B6



Summary for Subcatchment B7: Drainage Basin B7

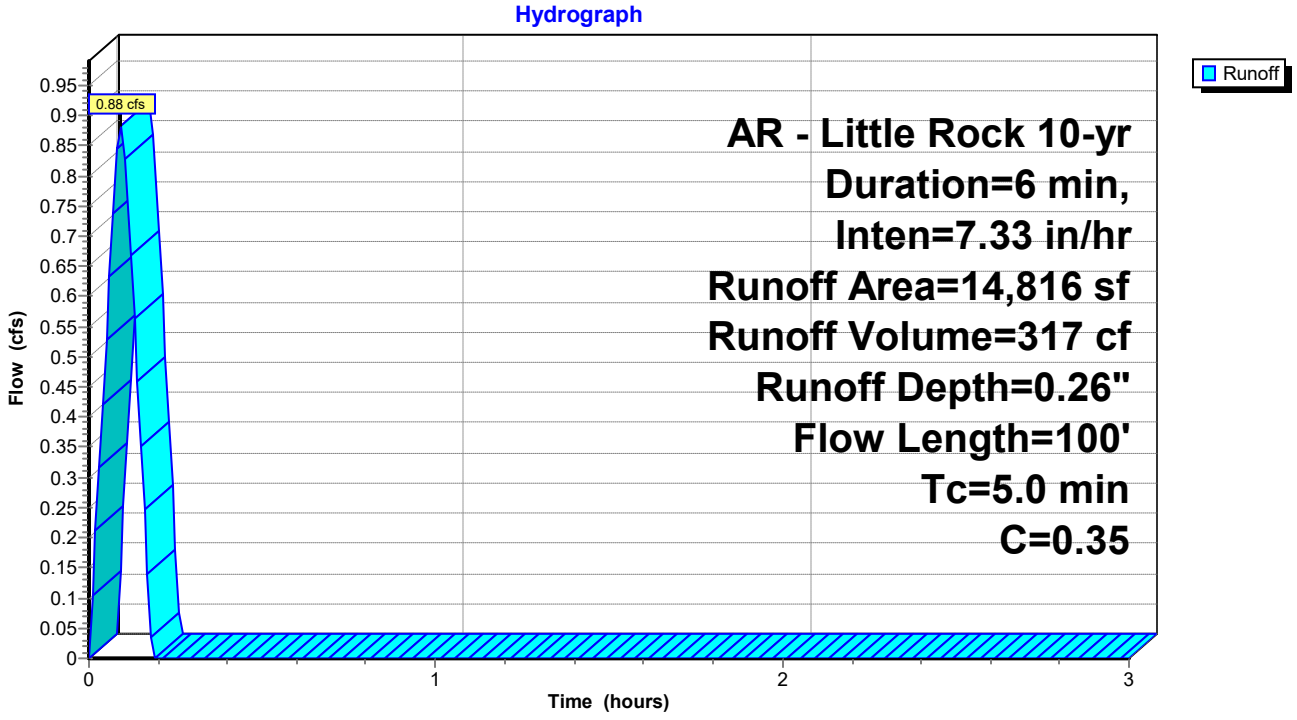
Runoff = 0.88 cfs @ 0.09 hrs, Volume= 317 cf, Depth= 0.26"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 10-yr Duration=6 min, Inten=7.33 in/hr

Area (sf)	C	Description
14,816	0.35	Sandy Soil 2-7% per manual
14,816		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	42	0.1667	3.09		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.5	58	0.0500	2.04		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
4.3					**Direct Entry, Minimum Adjustment**
5.0	100	Total			

Subcatchment B7: Drainage Basin B7



Summary for Pond CI-A1: CURB INLET A1

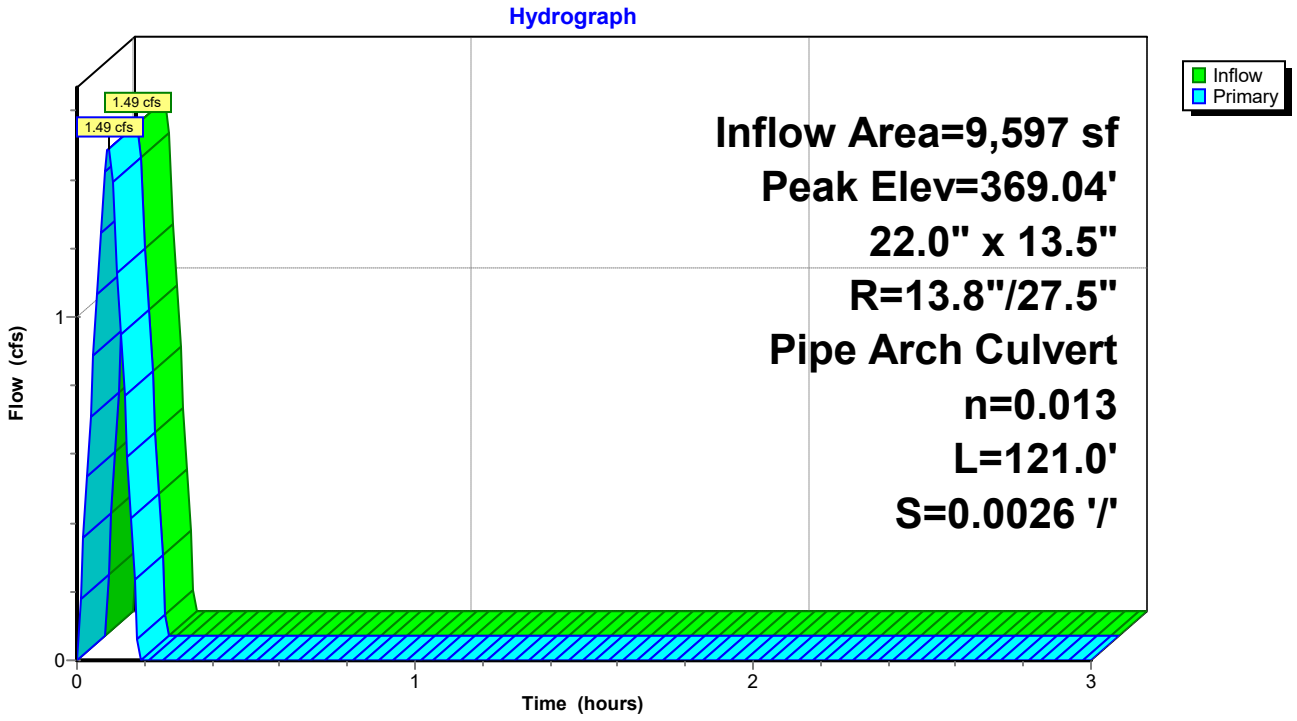
Inflow Area = 9,597 sf, 0.00% Impervious, Inflow Depth = 0.67" for 10-yr event
Inflow = 1.49 cfs @ 0.10 hrs, Volume= 533 cf
Outflow = 1.49 cfs @ 0.09 hrs, Volume= 533 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.49 cfs @ 0.09 hrs, Volume= 533 cf
Routed to Pond CI-A2 : CURB INLET A2

Routing by Stor-Ind method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Peak Elev= 369.04' @ 0.09 hrs

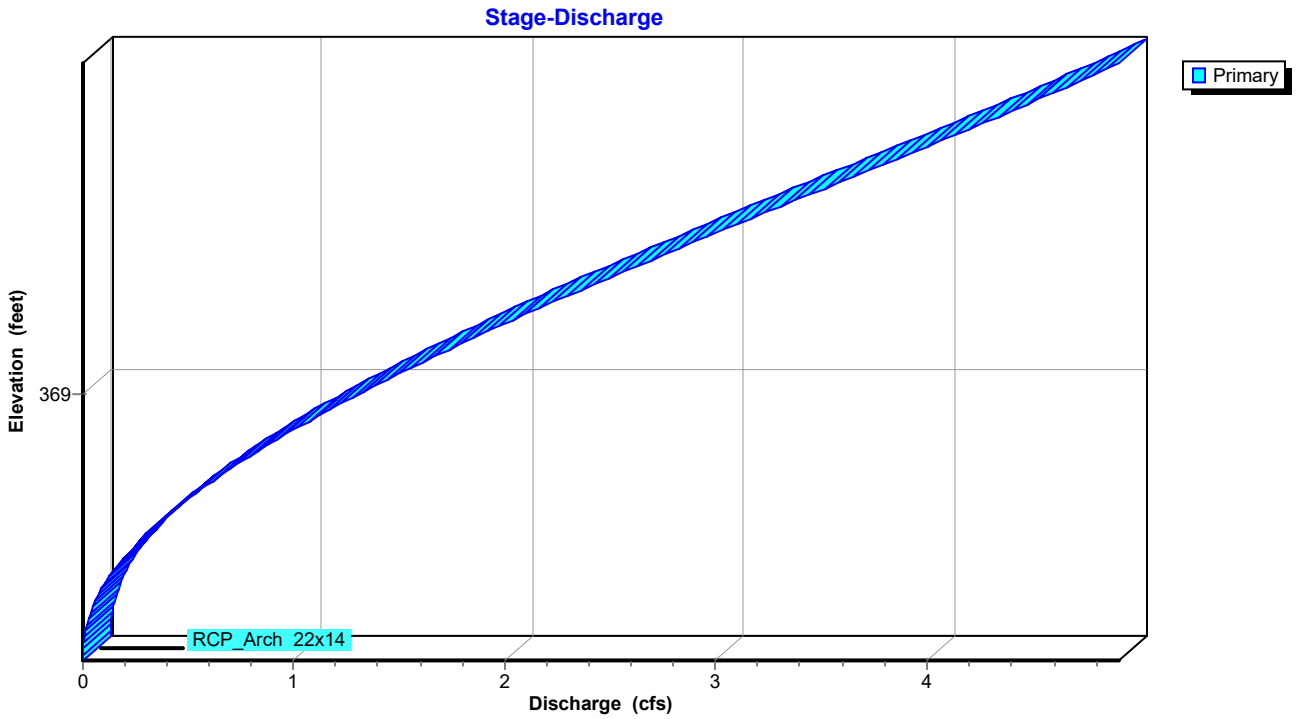
Device	Routing	Invert	Outlet Devices
#1	Primary	368.50'	**22.0" W x 13.5" H, R=13.8"/27.5" Pipe Arch RCP_Arch 22x14** L= 121.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 368.50' / 368.19' S= 0.0026 '/' Cc= 0.900 n= 0.013, Flow Area= 1.65 sf

Primary OutFlow Max=1.48 cfs @ 0.09 hrs HW=369.04' (Free Discharge)
1=RCP_Arch 22x14 (Barrel Controls 1.48 cfs @ 2.53 fps)

Pond CI-A1: CURB INLET A1



Pond CI-A1: CURB INLET A1



Stage-Area-Storage for Pond CI-A1: CURB INLET A1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
368.50	**0**	369.02	0	369.54	0
368.51	0	369.03	0	369.55	0
368.52	0	369.04	0	369.56	0
368.53	0	369.05	0	369.57	0
368.54	0	369.06	0	369.58	0
368.55	0	369.07	0	369.59	0
368.56	0	369.08	0	369.60	0
368.57	0	369.09	0	369.61	0
368.58	0	369.10	0	369.62	0
368.59	0	369.11	0		
368.60	0	369.12	0		
368.61	0	369.13	0		
368.62	0	369.14	0		
368.63	0	369.15	0		
368.64	0	369.16	0		
368.65	0	369.17	0		
368.66	0	369.18	0		
368.67	0	369.19	0		
368.68	0	369.20	0		
368.69	0	369.21	0		
368.70	0	369.22	0		
368.71	0	369.23	0		
368.72	0	369.24	0		
368.73	0	369.25	0		
368.74	0	369.26	0		
368.75	0	369.27	0		
368.76	0	369.28	0		
368.77	0	369.29	0		
368.78	0	369.30	0		
368.79	0	369.31	0		
368.80	0	369.32	0		
368.81	0	369.33	0		
368.82	0	369.34	0		
368.83	0	369.35	0		
368.84	0	369.36	0		
368.85	0	369.37	0		
368.86	0	369.38	0		
368.87	0	369.39	0		
368.88	0	369.40	0		
368.89	0	369.41	0		
368.90	0	369.42	0		
368.91	0	369.43	0		
368.92	0	369.44	0		
368.93	0	369.45	0		
368.94	0	369.46	0		
368.95	0	369.47	0		
368.96	0	369.48	0		
368.97	0	369.49	0		
368.98	0	369.50	0		
368.99	0	369.51	0		
369.00	0	369.52	0		
369.01	0	369.53	0		

Summary for Pond CI-A2: CURB INLET A2

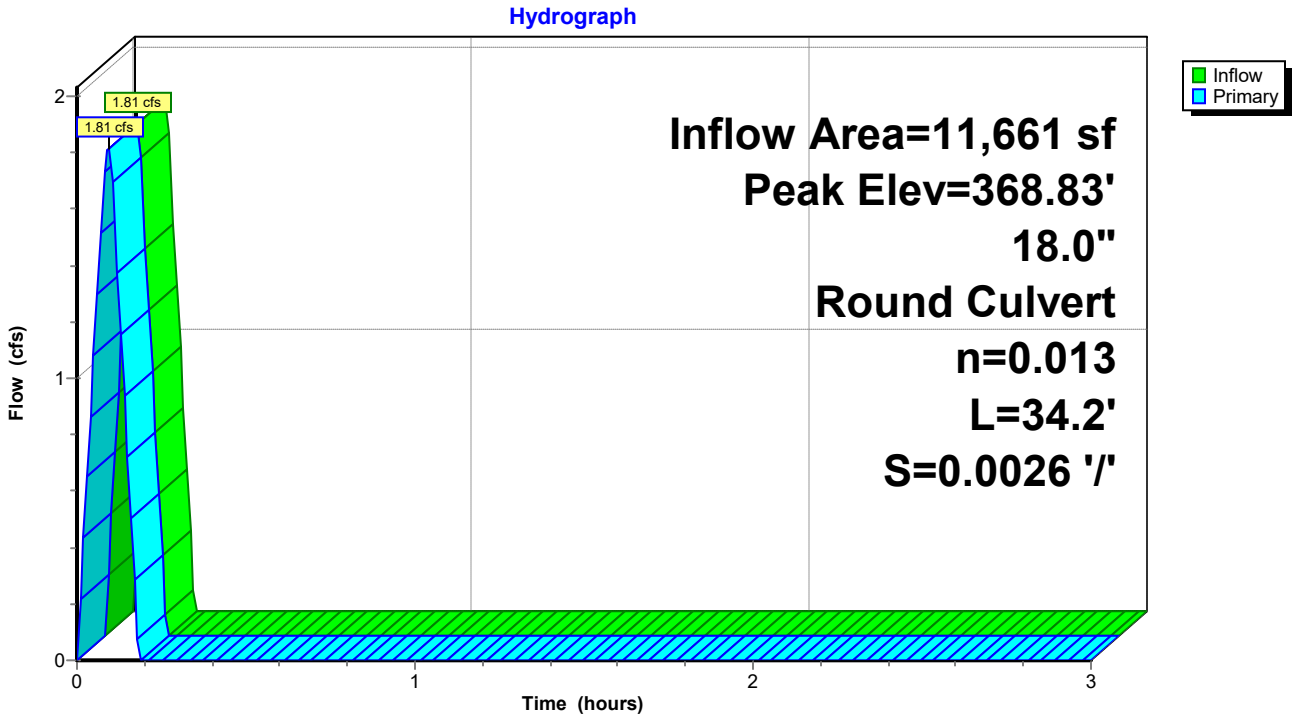
Inflow Area = 11,661 sf, 0.00% Impervious, Inflow Depth = 0.67" for 10-yr event
Inflow = 1.81 cfs @ 0.09 hrs, Volume= 649 cf
Outflow = 1.81 cfs @ 0.09 hrs, Volume= 649 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.81 cfs @ 0.09 hrs, Volume= 649 cf
Routed to Link POST-DEV : Post-Development

Routing by Stor-Ind method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Peak Elev= 368.83' @ 0.09 hrs

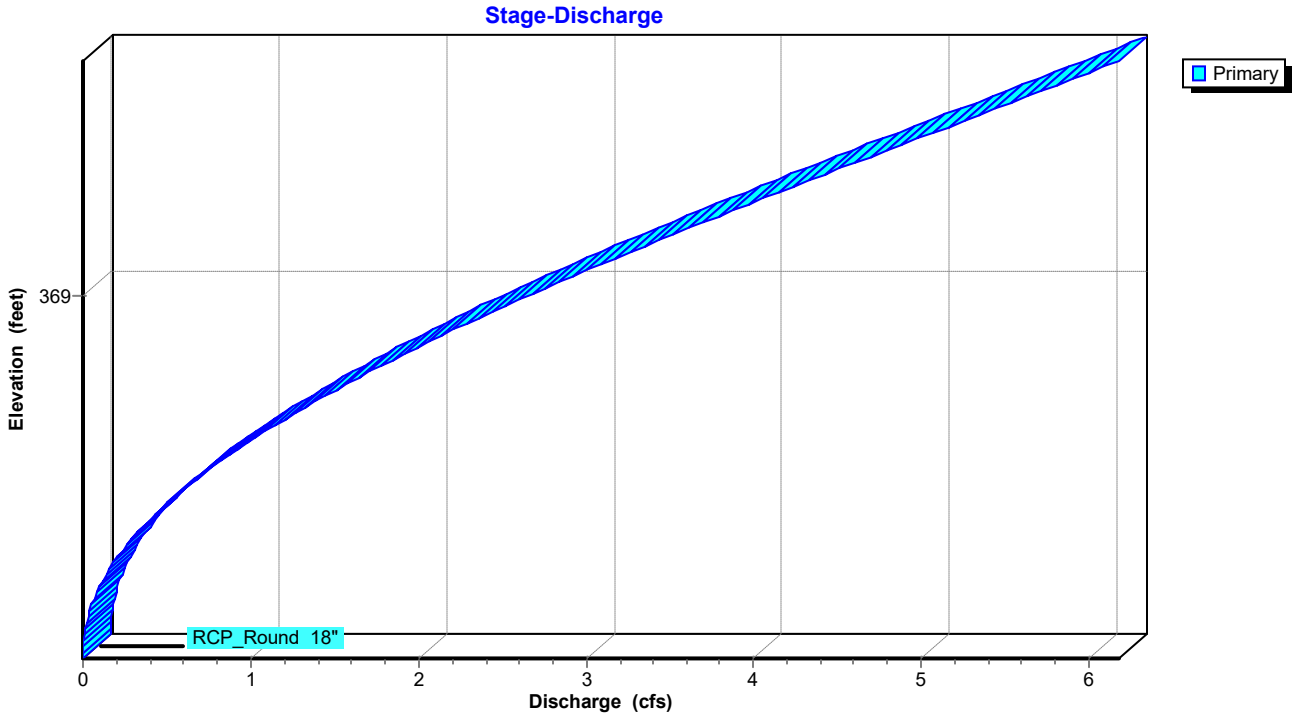
Device	Routing	Invert	Outlet Devices
#1	Primary	368.09'	**18.0" Round RCP_Round 18"** L= 34.2' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 368.09' / 368.00' S= 0.0026 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=1.80 cfs @ 0.09 hrs HW=368.83' (Free Discharge)
1=RCP_Round 18" (Barrel Controls 1.80 cfs @ 3.03 fps)

Pond CI-A2: CURB INLET A2



Pond CI-A2: CURB INLET A2



Stage-Area-Storage for Pond CI-A2: CURB INLET A2

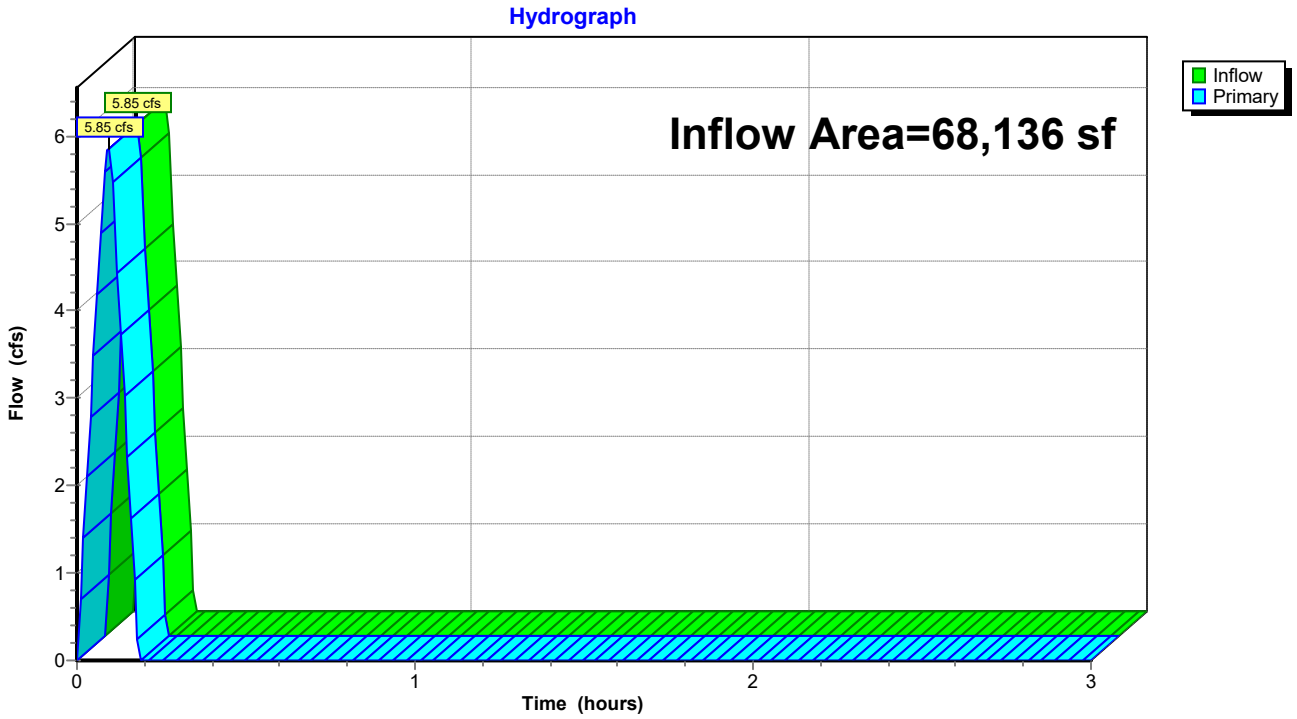
Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
368.09	**0**	368.61	0	369.13	0
368.10	0	368.62	0	369.14	0
368.11	0	368.63	0	369.15	0
368.12	0	368.64	0	369.16	0
368.13	0	368.65	0	369.17	0
368.14	0	368.66	0	369.18	0
368.15	0	368.67	0	369.19	0
368.16	0	368.68	0	369.20	0
368.17	0	368.69	0	369.21	0
368.18	0	368.70	0	369.22	0
368.19	0	368.71	0	369.23	0
368.20	0	368.72	0	369.24	0
368.21	0	368.73	0	369.25	0
368.22	0	368.74	0	369.26	0
368.23	0	368.75	0	369.27	0
368.24	0	368.76	0	369.28	0
368.25	0	368.77	0	369.29	0
368.26	0	368.78	0	369.30	0
368.27	0	368.79	0	369.31	0
368.28	0	368.80	0	369.32	0
368.29	0	368.81	0	369.33	0
368.30	0	368.82	0	369.34	0
368.31	0	368.83	0	369.35	0
368.32	0	368.84	0	369.36	0
368.33	0	368.85	0	369.37	0
368.34	0	368.86	0	369.38	0
368.35	0	368.87	0	369.39	0
368.36	0	368.88	0	369.40	0
368.37	0	368.89	0	369.41	0
368.38	0	368.90	0	369.42	0
368.39	0	368.91	0	369.43	0
368.40	0	368.92	0	369.44	0
368.41	0	368.93	0	369.45	0
368.42	0	368.94	0	369.46	0
368.43	0	368.95	0	369.47	0
368.44	0	368.96	0	369.48	0
368.45	0	368.97	0	369.49	0
368.46	0	368.98	0	369.50	0
368.47	0	368.99	0	369.51	0
368.48	0	369.00	0	369.52	0
368.49	0	369.01	0	369.53	0
368.50	0	369.02	0	369.54	0
368.51	0	369.03	0	369.55	0
368.52	0	369.04	0	369.56	0
368.53	0	369.05	0	369.57	0
368.54	0	369.06	0	369.58	0
368.55	0	369.07	0	369.59	0
368.56	0	369.08	0		
368.57	0	369.09	0		
368.58	0	369.10	0		
368.59	0	369.11	0		
368.60	0	369.12	0		

Summary for Link POST-DEV: Post-Development

Inflow Area = 68,136 sf, 0.00% Impervious, Inflow Depth = 0.37" for 10-yr event
Inflow = 5.85 cfs @ 0.09 hrs, Volume= 2,096 cf
Primary = 5.85 cfs @ 0.09 hrs, Volume= 2,096 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs

Link POST-DEV: Post-Development



Summary for Subcatchment B1: Drainage Basin B1

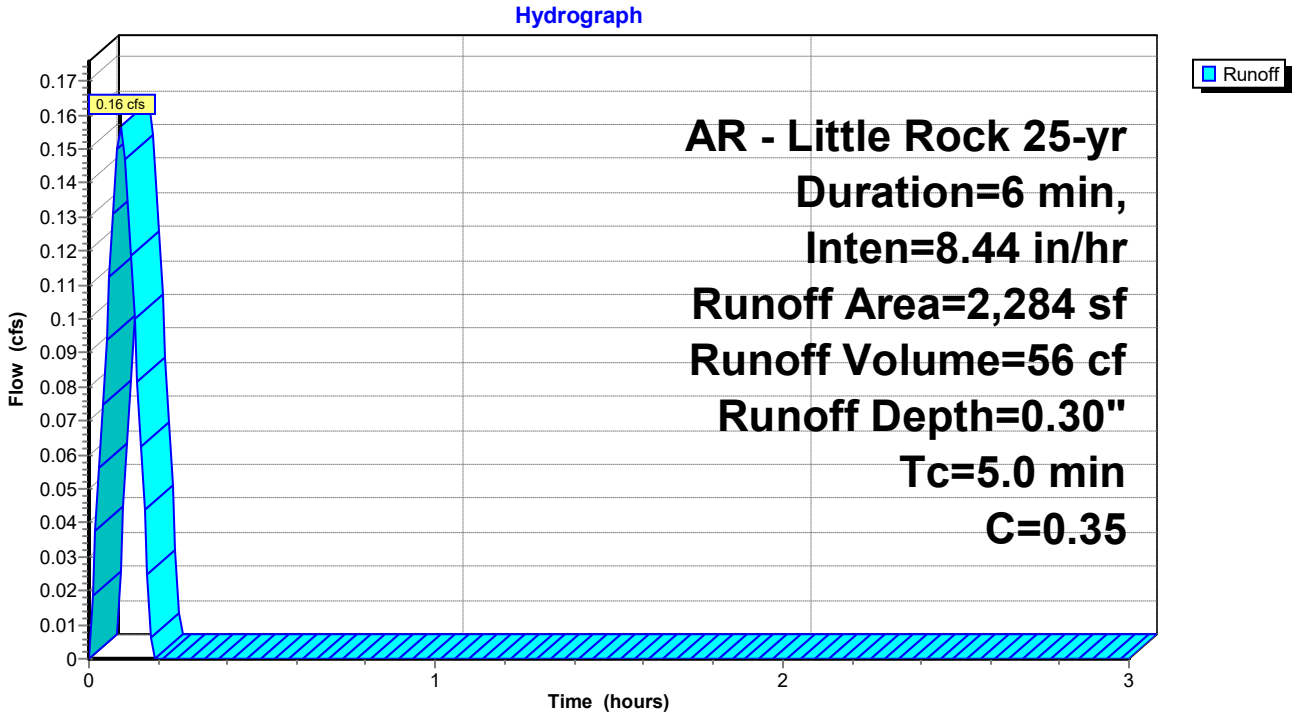
Runoff = 0.16 cfs @ 0.09 hrs, Volume= 56 cf, Depth= 0.30"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 25-yr Duration=6 min, Inten=8.44 in/hr

Area (sf)	C	Description
2,284	0.35	Sandy Soil 2-7% per manual
0	0.92	Paved Areas
2,284	0.35	Weighted Average
2,284		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry, Minimum Adjustment**

Subcatchment B1: Drainage Basin B1



Summary for Subcatchment B2: Drainage Basin B2

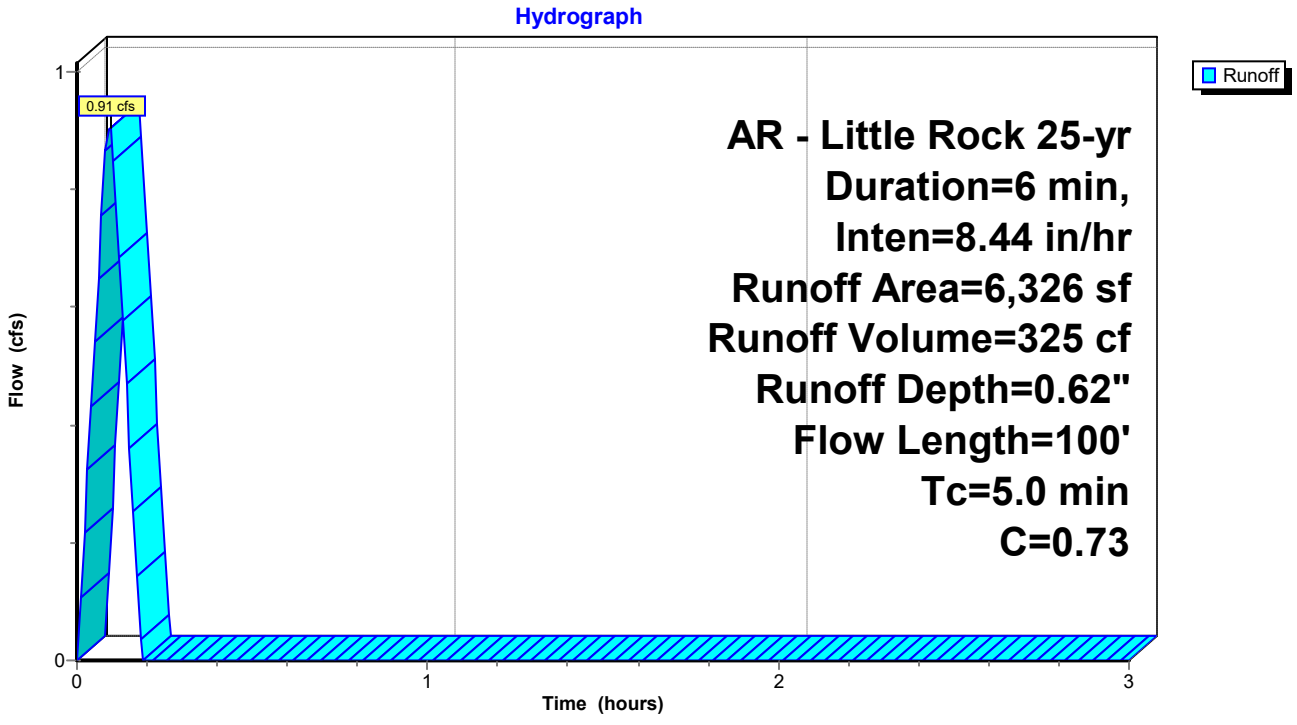
Runoff = 0.91 cfs @ 0.09 hrs, Volume= 325 cf, Depth= 0.62"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 25-yr Duration=6 min, Inten=8.44 in/hr

Area (sf)	C	Description
2,115	0.35	Sandy Soil 2-7% per manual
4,211	0.92	Paved Areas
6,326	0.73	Weighted Average
6,326		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	42	0.1667	3.09		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.5	58	0.0500	2.04		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
4.3					**Direct Entry, Minimum Adjustment**
5.0	100	Total			

Subcatchment B2: Drainage Basin B2



Summary for Subcatchment B3: Drainage Basin B3

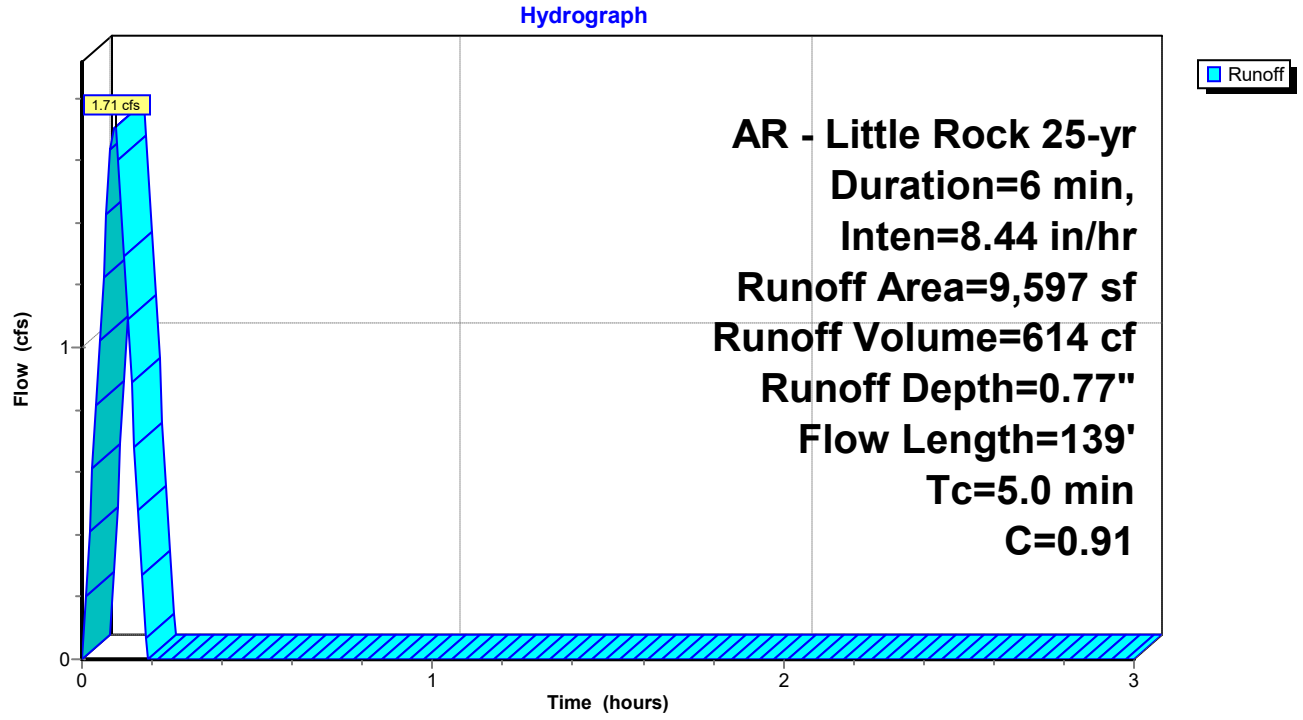
Runoff = 1.71 cfs @ 0.09 hrs, Volume= 614 cf, Depth= 0.77"
Routed to Pond CI-A1 : CURB INLET A1

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 25-yr Duration=6 min, Inten=8.44 in/hr

Area (sf)	C	Description
155	0.35	Sandy Soil 2-7% per manual
9,442	0.92	Paved Areas
9,597	0.91	Weighted Average
9,597		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	28	0.1667	2.85		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	30	0.0160	1.13		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.4	41	0.0520	1.93		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.2	40	0.0360	3.85		**Shallow Concentrated Flow, Gutter Flow** Paved Kv= 20.3 fps
3.8					**Direct Entry, Minimum Adjustment**
5.0	139	Total			

Subcatchment B3: Drainage Basin B3



Summary for Subcatchment B4: Drainage Basin B4

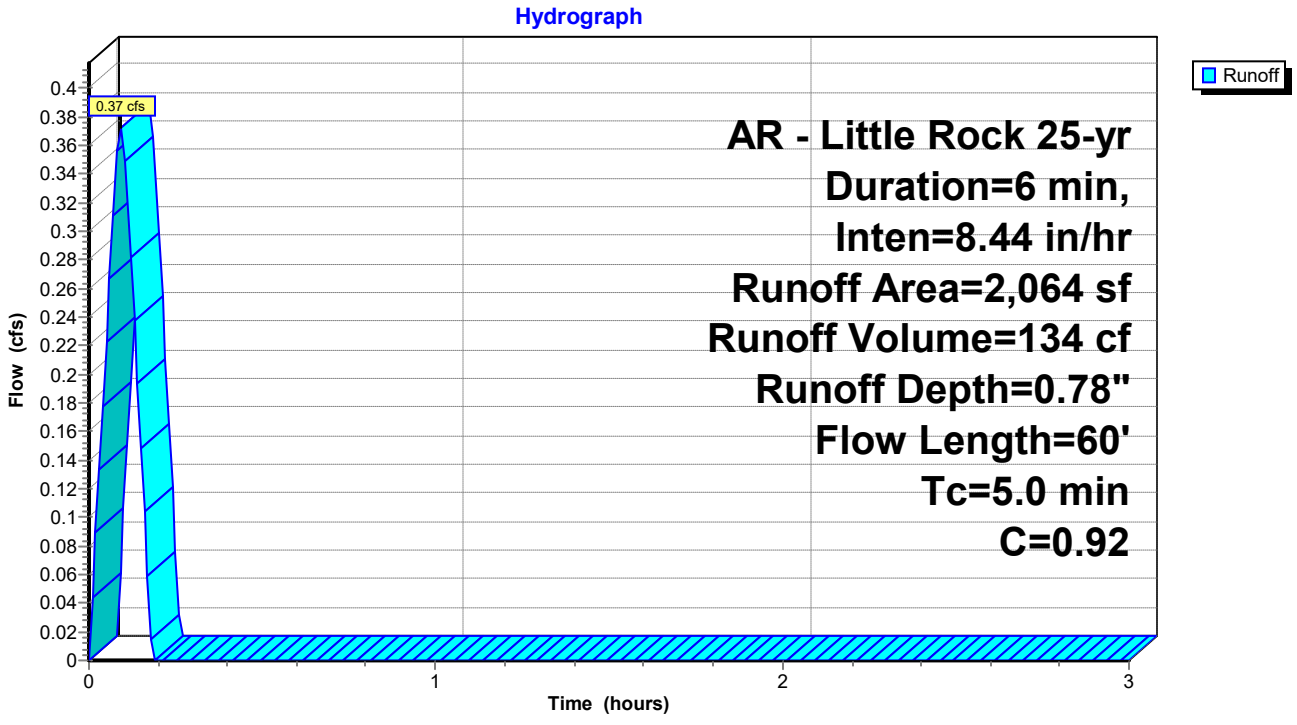
Runoff = 0.37 cfs @ 0.09 hrs, Volume= 134 cf, Depth= 0.78"
Routed to Pond CI-A2 : CURB INLET A2

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 25-yr Duration=6 min, Inten=8.44 in/hr

Area (sf)	C	Description
0	0.35	Sandy Soil 2-7% per manual
2,064	0.92	Paved Areas
2,064	0.92	Weighted Average
2,064		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	45	0.0170	1.26		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.0	15	0.0840	5.88		**Shallow Concentrated Flow, Gutter Flow** Paved Kv= 20.3 fps
4.4					**Direct Entry, Minimum Adjustment**
5.0	60	Total			

Subcatchment B4: Drainage Basin B4



Summary for Subcatchment B5: Drainage Basin B5

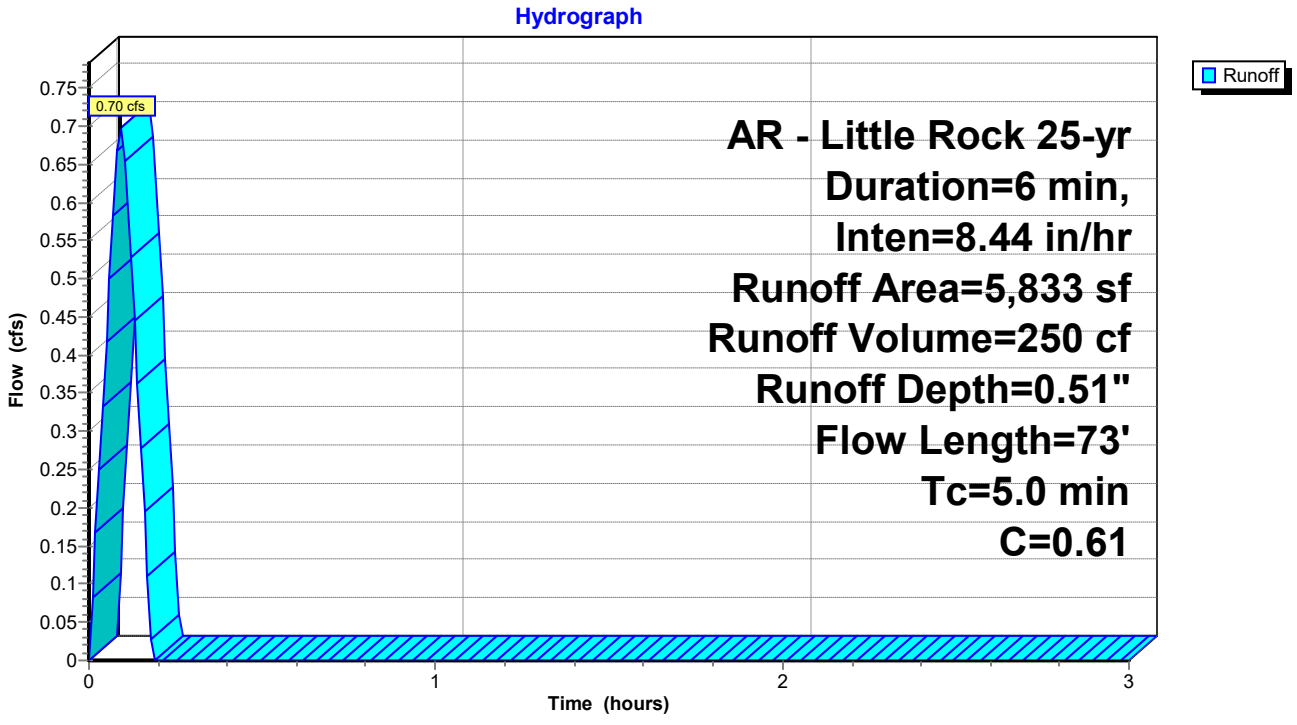
Runoff = 0.70 cfs @ 0.09 hrs, Volume= 250 cf, Depth= 0.51"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 25-yr Duration=6 min, Inten=8.44 in/hr

Area (sf)	C	Description
3,123	0.35	Sandy Soil 2-7% per manual
2,710	0.92	Paved Areas
5,833	0.61	Weighted Average
5,833		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	18	0.1667	2.61		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	55	0.0860	2.05		**Shallow Concentrated Flow, Overland Concentrated** Short Grass Pasture Kv= 7.0 fps
4.5					**Direct Entry, Minimum Adjustment**
5.0	73	Total			

Subcatchment B5: Drainage Basin B5



Summary for Subcatchment B6: Drainage Basin B6

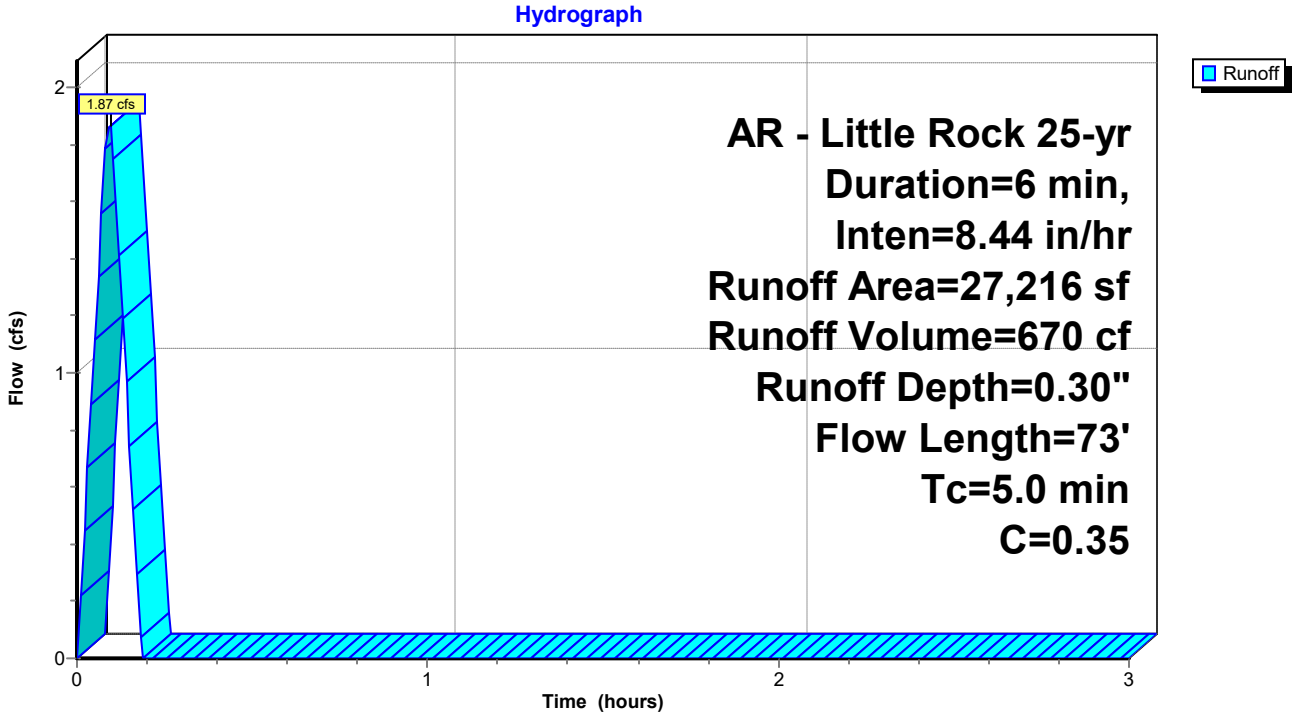
Runoff = 1.87 cfs @ 0.09 hrs, Volume= 670 cf, Depth= 0.30"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 25-yr Duration=6 min, Inten=8.44 in/hr

Area (sf)	C	Description
27,216	0.35	Sandy Soil 2-7% per manual
27,216		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	18	0.1667	2.61		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	55	0.0860	2.05		**Shallow Concentrated Flow, Overland Concentrated** Short Grass Pasture Kv= 7.0 fps
4.5					**Direct Entry, Minimum Adjustment**
5.0	73	Total			

Subcatchment B6: Drainage Basin B6



Summary for Subcatchment B7: Drainage Basin B7

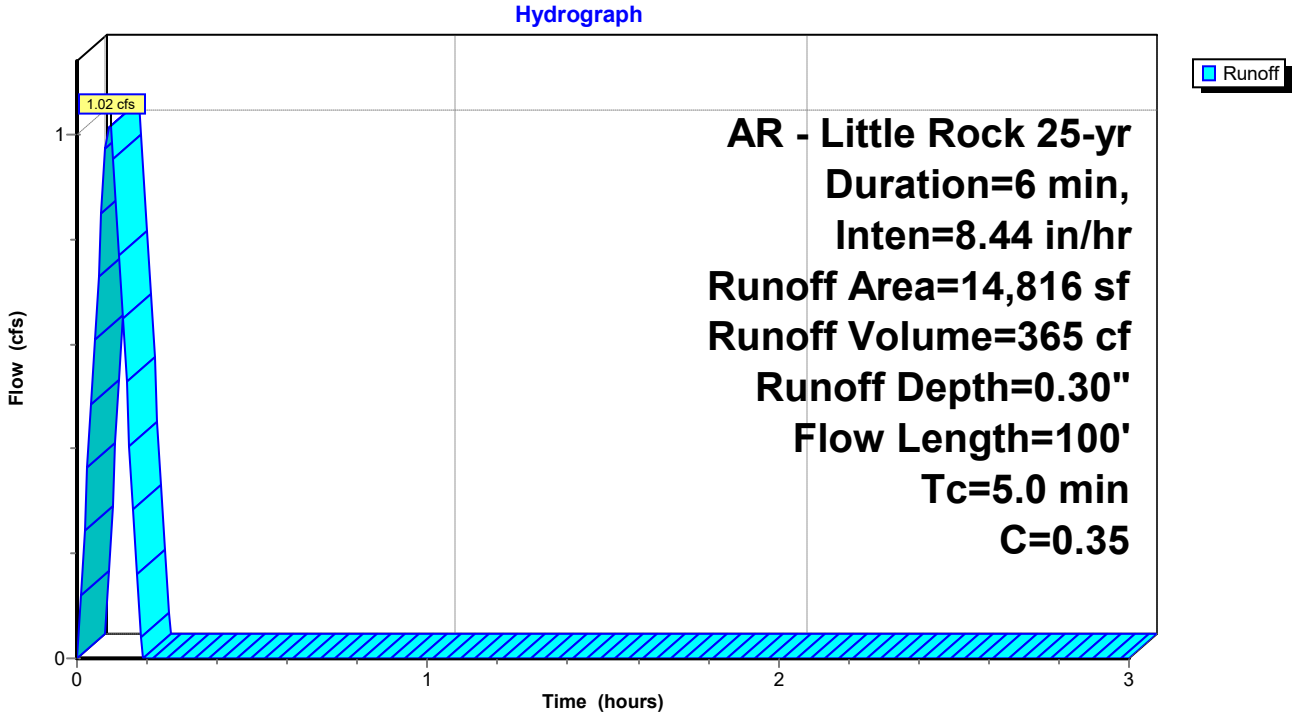
Runoff = 1.02 cfs @ 0.09 hrs, Volume= 365 cf, Depth= 0.30"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 25-yr Duration=6 min, Inten=8.44 in/hr

Area (sf)	C	Description
14,816	0.35	Sandy Soil 2-7% per manual
14,816		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	42	0.1667	3.09		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.5	58	0.0500	2.04		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
4.3					**Direct Entry, Minimum Adjustment**
5.0	100	Total			

Subcatchment B7: Drainage Basin B7



Summary for Pond CI-A1: CURB INLET A1

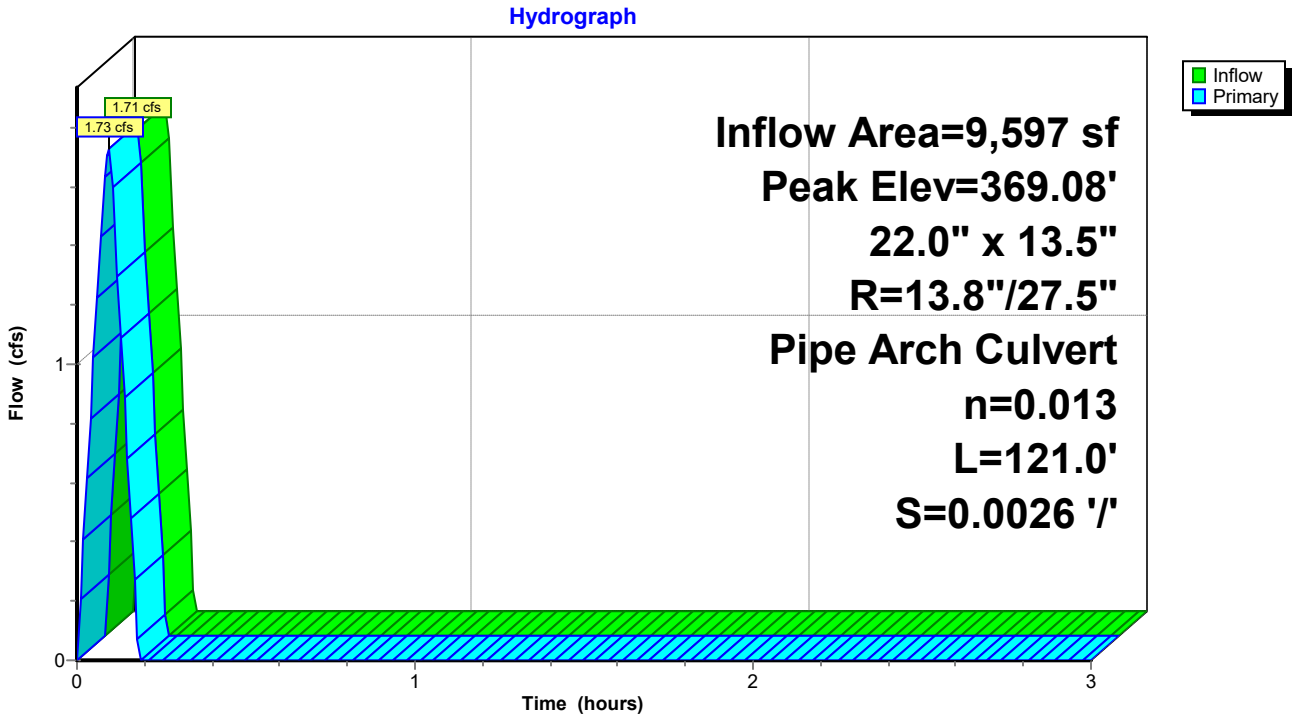
Inflow Area = 9,597 sf, 0.00% Impervious, Inflow Depth = 0.77" for 25-yr event
Inflow = 1.71 cfs @ 0.09 hrs, Volume= 614 cf
Outflow = 1.73 cfs @ 0.10 hrs, Volume= 614 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.73 cfs @ 0.10 hrs, Volume= 614 cf
Routed to Pond CI-A2 : CURB INLET A2

Routing by Stor-Ind method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Peak Elev= 369.08' @ 0.09 hrs

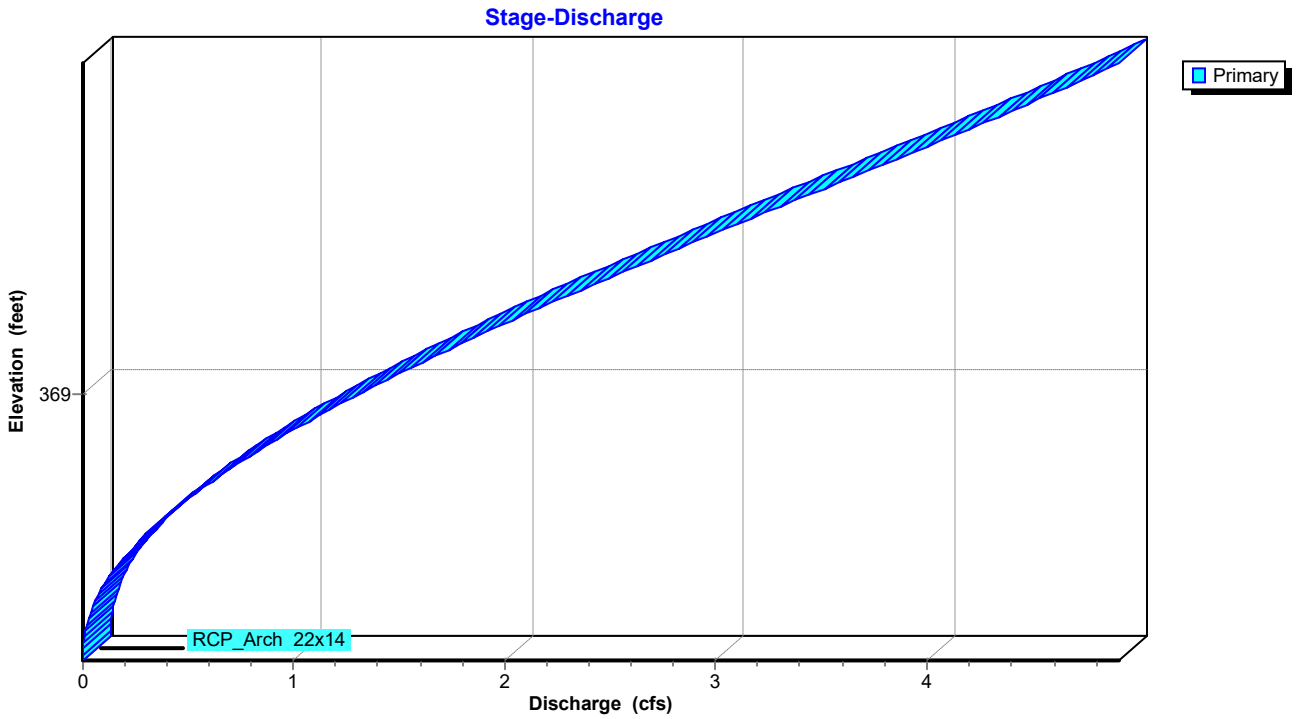
Device	Routing	Invert	Outlet Devices
#1	Primary	368.50'	**22.0" W x 13.5" H, R=13.8"/27.5" Pipe Arch RCP_Arch 22x14** L= 121.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 368.50' / 368.19' S= 0.0026 '/' Cc= 0.900 n= 0.013, Flow Area= 1.65 sf

Primary OutFlow Max=1.71 cfs @ 0.10 hrs HW=369.08' (Free Discharge)
1=RCP_Arch 22x14 (Barrel Controls 1.71 cfs @ 2.64 fps)

Pond CI-A1: CURB INLET A1



Pond CI-A1: CURB INLET A1



Stage-Area-Storage for Pond CI-A1: CURB INLET A1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
368.50	**0**	369.02	0	369.54	0
368.51	0	369.03	0	369.55	0
368.52	0	369.04	0	369.56	0
368.53	0	369.05	0	369.57	0
368.54	0	369.06	0	369.58	0
368.55	0	369.07	0	369.59	0
368.56	0	369.08	0	369.60	0
368.57	0	369.09	0	369.61	0
368.58	0	369.10	0	369.62	0
368.59	0	369.11	0		
368.60	0	369.12	0		
368.61	0	369.13	0		
368.62	0	369.14	0		
368.63	0	369.15	0		
368.64	0	369.16	0		
368.65	0	369.17	0		
368.66	0	369.18	0		
368.67	0	369.19	0		
368.68	0	369.20	0		
368.69	0	369.21	0		
368.70	0	369.22	0		
368.71	0	369.23	0		
368.72	0	369.24	0		
368.73	0	369.25	0		
368.74	0	369.26	0		
368.75	0	369.27	0		
368.76	0	369.28	0		
368.77	0	369.29	0		
368.78	0	369.30	0		
368.79	0	369.31	0		
368.80	0	369.32	0		
368.81	0	369.33	0		
368.82	0	369.34	0		
368.83	0	369.35	0		
368.84	0	369.36	0		
368.85	0	369.37	0		
368.86	0	369.38	0		
368.87	0	369.39	0		
368.88	0	369.40	0		
368.89	0	369.41	0		
368.90	0	369.42	0		
368.91	0	369.43	0		
368.92	0	369.44	0		
368.93	0	369.45	0		
368.94	0	369.46	0		
368.95	0	369.47	0		
368.96	0	369.48	0		
368.97	0	369.49	0		
368.98	0	369.50	0		
368.99	0	369.51	0		
369.00	0	369.52	0		
369.01	0	369.53	0		

Summary for Pond CI-A2: CURB INLET A2

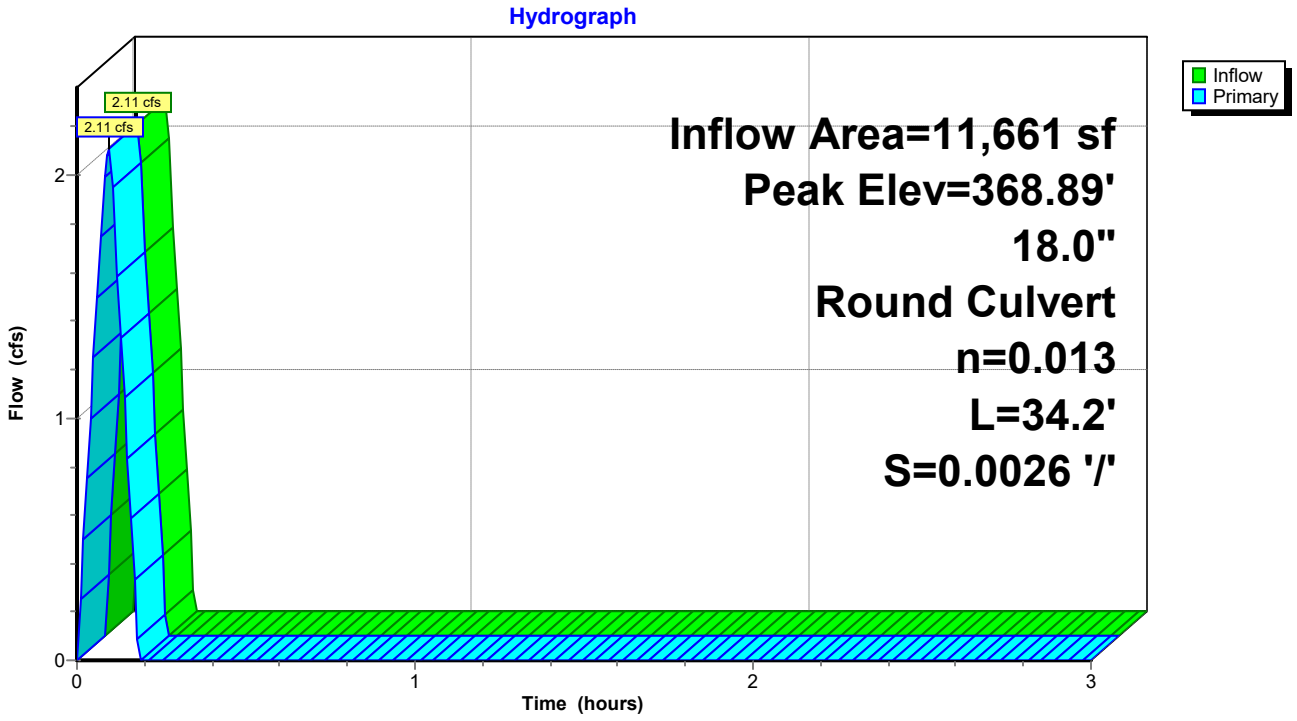
Inflow Area = 11,661 sf, 0.00% Impervious, Inflow Depth = 0.77" for 25-yr event
Inflow = 2.11 cfs @ 0.10 hrs, Volume= 748 cf
Outflow = 2.11 cfs @ 0.10 hrs, Volume= 748 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.11 cfs @ 0.10 hrs, Volume= 748 cf
Routed to Link POST-DEV : Post-Development

Routing by Stor-Ind method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Peak Elev= 368.89' @ 0.09 hrs

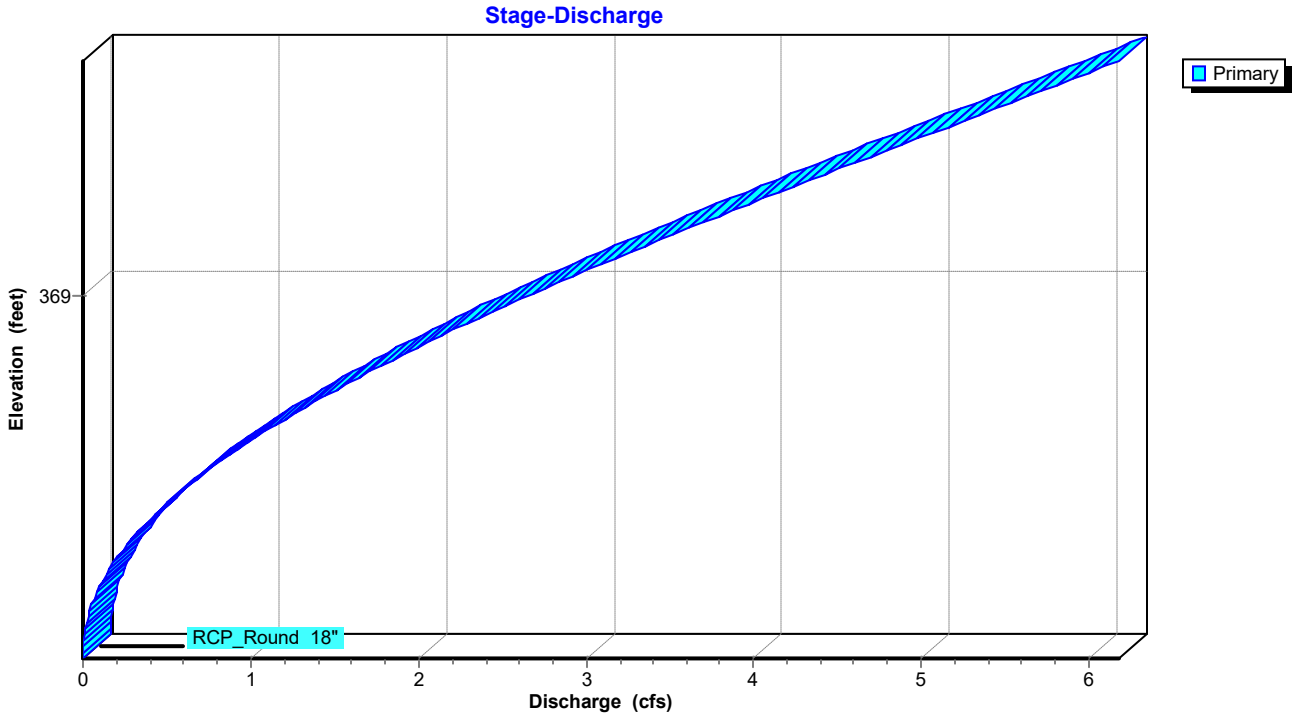
Device	Routing	Invert	Outlet Devices
#1	Primary	368.09'	**18.0" Round RCP_Round 18"** L= 34.2' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 368.09' / 368.00' S= 0.0026 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=2.08 cfs @ 0.10 hrs HW=368.89' (Free Discharge)
1=RCP_Round 18" (Barrel Controls 2.08 cfs @ 3.16 fps)

Pond CI-A2: CURB INLET A2



Pond CI-A2: CURB INLET A2



Stage-Area-Storage for Pond CI-A2: CURB INLET A2

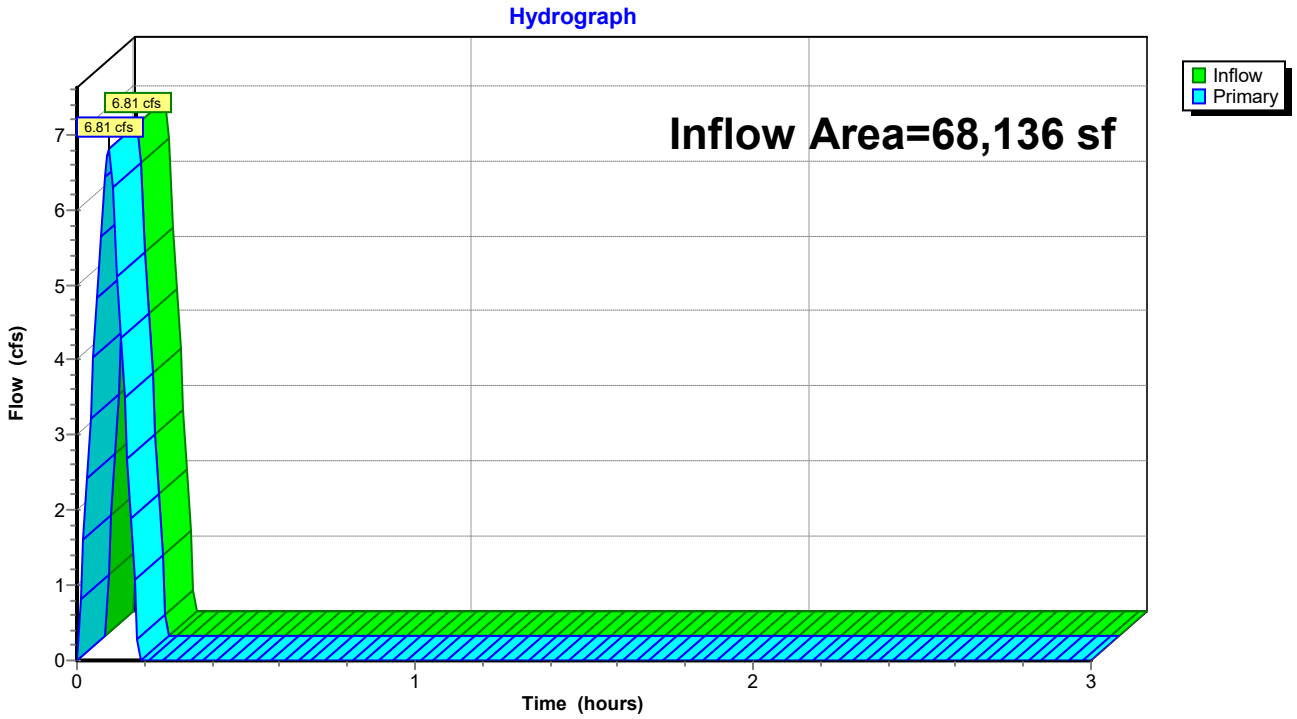
Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
368.09	**0**	368.61	0	369.13	0
368.10	0	368.62	0	369.14	0
368.11	0	368.63	0	369.15	0
368.12	0	368.64	0	369.16	0
368.13	0	368.65	0	369.17	0
368.14	0	368.66	0	369.18	0
368.15	0	368.67	0	369.19	0
368.16	0	368.68	0	369.20	0
368.17	0	368.69	0	369.21	0
368.18	0	368.70	0	369.22	0
368.19	0	368.71	0	369.23	0
368.20	0	368.72	0	369.24	0
368.21	0	368.73	0	369.25	0
368.22	0	368.74	0	369.26	0
368.23	0	368.75	0	369.27	0
368.24	0	368.76	0	369.28	0
368.25	0	368.77	0	369.29	0
368.26	0	368.78	0	369.30	0
368.27	0	368.79	0	369.31	0
368.28	0	368.80	0	369.32	0
368.29	0	368.81	0	369.33	0
368.30	0	368.82	0	369.34	0
368.31	0	368.83	0	369.35	0
368.32	0	368.84	0	369.36	0
368.33	0	368.85	0	369.37	0
368.34	0	368.86	0	369.38	0
368.35	0	368.87	0	369.39	0
368.36	0	368.88	0	369.40	0
368.37	0	368.89	0	369.41	0
368.38	0	368.90	0	369.42	0
368.39	0	368.91	0	369.43	0
368.40	0	368.92	0	369.44	0
368.41	0	368.93	0	369.45	0
368.42	0	368.94	0	369.46	0
368.43	0	368.95	0	369.47	0
368.44	0	368.96	0	369.48	0
368.45	0	368.97	0	369.49	0
368.46	0	368.98	0	369.50	0
368.47	0	368.99	0	369.51	0
368.48	0	369.00	0	369.52	0
368.49	0	369.01	0	369.53	0
368.50	0	369.02	0	369.54	0
368.51	0	369.03	0	369.55	0
368.52	0	369.04	0	369.56	0
368.53	0	369.05	0	369.57	0
368.54	0	369.06	0	369.58	0
368.55	0	369.07	0	369.59	0
368.56	0	369.08	0		
368.57	0	369.09	0		
368.58	0	369.10	0		
368.59	0	369.11	0		
368.60	0	369.12	0		

Summary for Link POST-DEV: Post-Development

Inflow Area = 68,136 sf, 0.00% Impervious, Inflow Depth = 0.43" for 25-yr event
Inflow = 6.81 cfs @ 0.09 hrs, Volume= 2,414 cf
Primary = 6.81 cfs @ 0.09 hrs, Volume= 2,414 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs

Link POST-DEV: Post-Development



Summary for Subcatchment B1: Drainage Basin B1

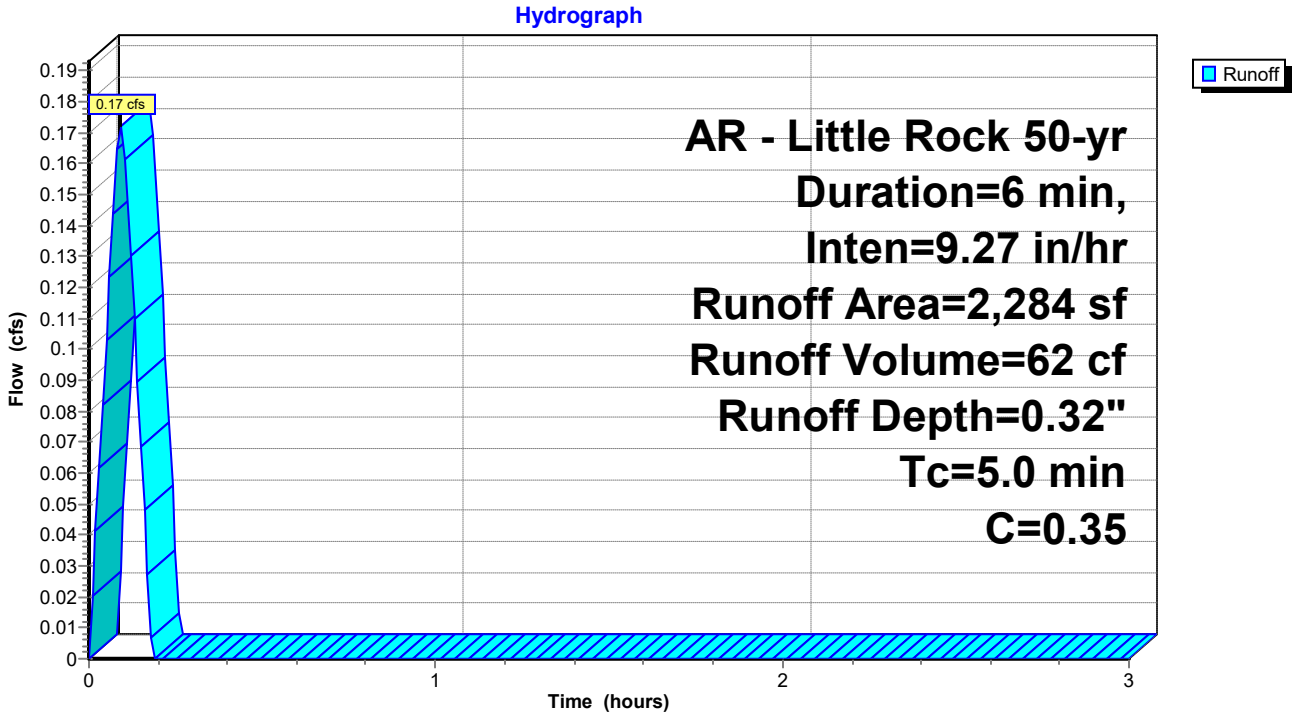
Runoff = 0.17 cfs @ 0.09 hrs, Volume= 62 cf, Depth= 0.32"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 50-yr Duration=6 min, Inten=9.27 in/hr

Area (sf)	C	Description
2,284	0.35	Sandy Soil 2-7% per manual
0	0.92	Paved Areas
2,284	0.35	Weighted Average
2,284		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry, Minimum Adjustment**

Subcatchment B1: Drainage Basin B1



Summary for Subcatchment B2: Drainage Basin B2

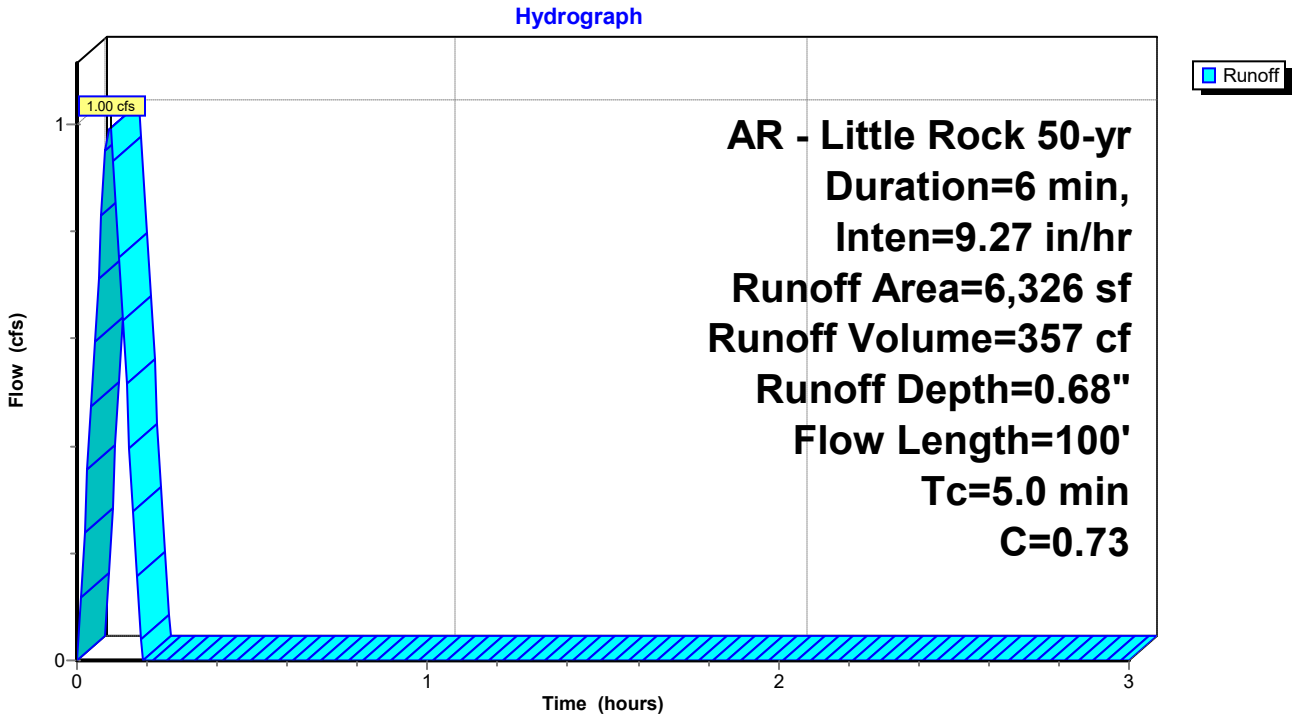
Runoff = 1.00 cfs @ 0.09 hrs, Volume= 357 cf, Depth= 0.68"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 50-yr Duration=6 min, Inten=9.27 in/hr

Area (sf)	C	Description
2,115	0.35	Sandy Soil 2-7% per manual
4,211	0.92	Paved Areas
6,326	0.73	Weighted Average
6,326		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	42	0.1667	3.09		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.5	58	0.0500	2.04		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
4.3					**Direct Entry, Minimum Adjustment**
5.0	100	Total			

Subcatchment B2: Drainage Basin B2



Summary for Subcatchment B3: Drainage Basin B3

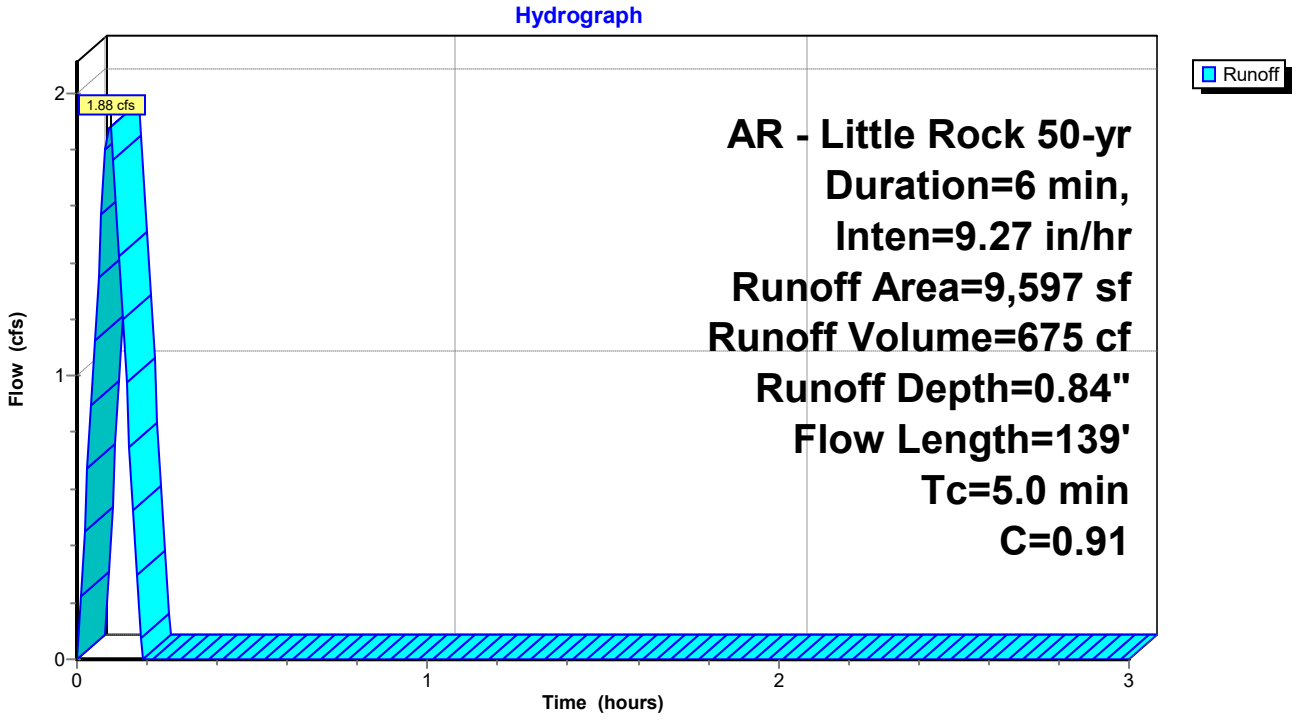
Runoff = 1.88 cfs @ 0.09 hrs, Volume= 675 cf, Depth= 0.84"
Routed to Pond CI-A1 : CURB INLET A1

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 50-yr Duration=6 min, Inten=9.27 in/hr

Area (sf)	C	Description
155	0.35	Sandy Soil 2-7% per manual
9,442	0.92	Paved Areas
9,597	0.91	Weighted Average
9,597		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	28	0.1667	2.85		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	30	0.0160	1.13		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.4	41	0.0520	1.93		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.2	40	0.0360	3.85		**Shallow Concentrated Flow, Gutter Flow** Paved Kv= 20.3 fps
3.8					**Direct Entry, Minimum Adjustment**
5.0	139	Total			

Subcatchment B3: Drainage Basin B3



Summary for Subcatchment B4: Drainage Basin B4

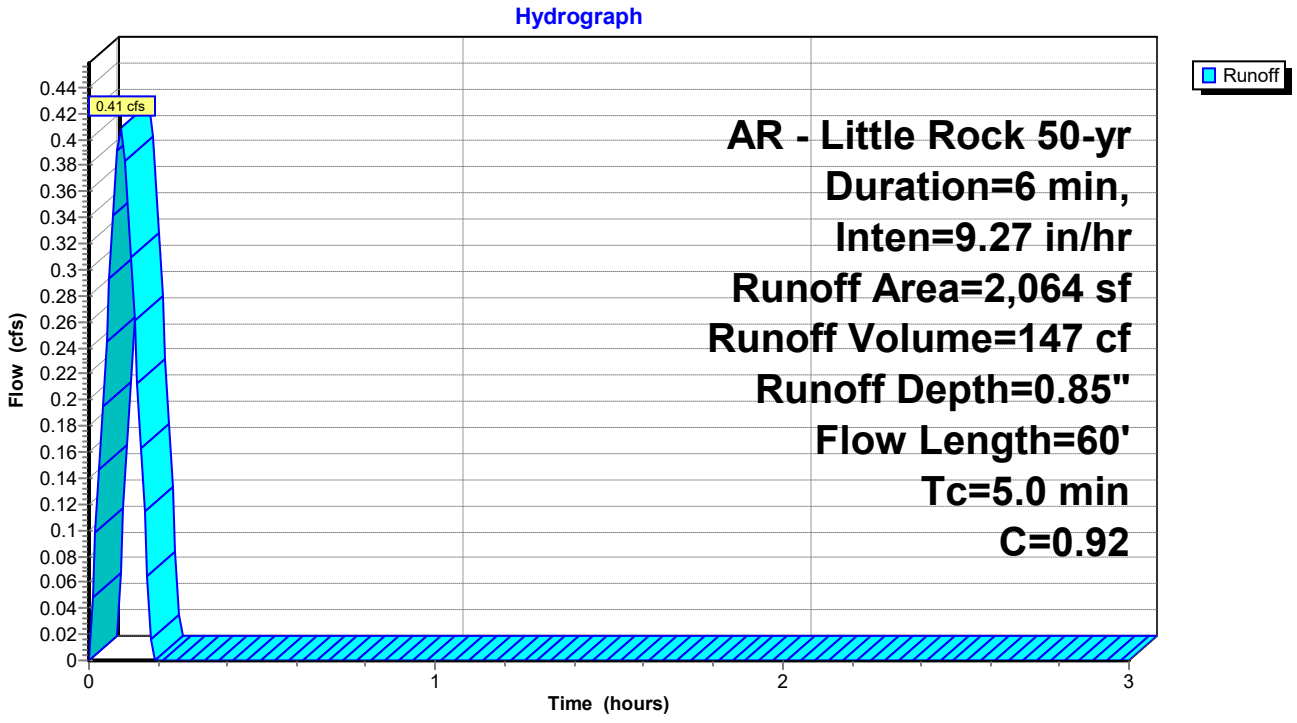
Runoff = 0.41 cfs @ 0.09 hrs, Volume= 147 cf, Depth= 0.85"
Routed to Pond CI-A2 : CURB INLET A2

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 50-yr Duration=6 min, Inten=9.27 in/hr

Area (sf)	C	Description
0	0.35	Sandy Soil 2-7% per manual
2,064	0.92	Paved Areas
2,064	0.92	Weighted Average
2,064		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	45	0.0170	1.26		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.0	15	0.0840	5.88		**Shallow Concentrated Flow, Gutter Flow** Paved Kv= 20.3 fps
4.4					**Direct Entry, Minimum Adjustment**
5.0	60	Total			

Subcatchment B4: Drainage Basin B4



Summary for Subcatchment B5: Drainage Basin B5

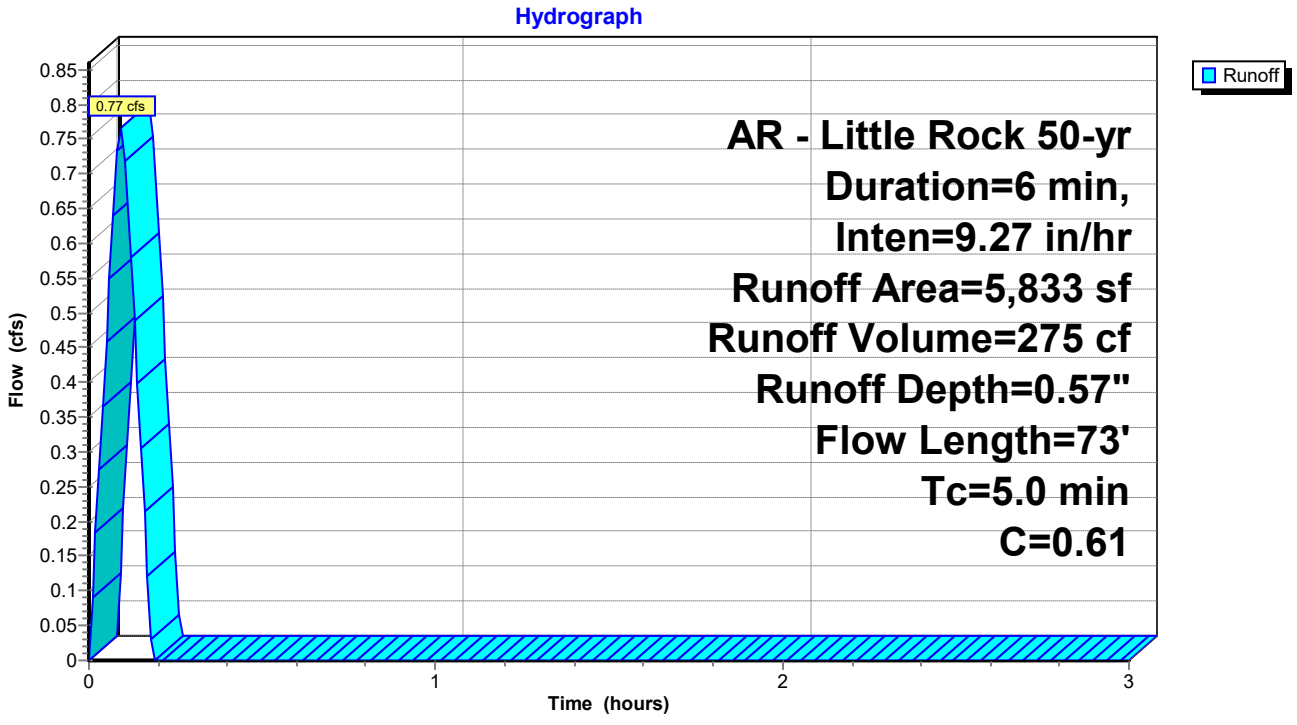
Runoff = 0.77 cfs @ 0.09 hrs, Volume= 275 cf, Depth= 0.57"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 50-yr Duration=6 min, Inten=9.27 in/hr

Area (sf)	C	Description
3,123	0.35	Sandy Soil 2-7% per manual
2,710	0.92	Paved Areas
5,833	0.61	Weighted Average
5,833		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	18	0.1667	2.61		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	55	0.0860	2.05		**Shallow Concentrated Flow, Overland Concentrated** Short Grass Pasture Kv= 7.0 fps
4.5					**Direct Entry, Minimum Adjustment**
5.0	73	Total			

Subcatchment B5: Drainage Basin B5



Summary for Subcatchment B6: Drainage Basin B6

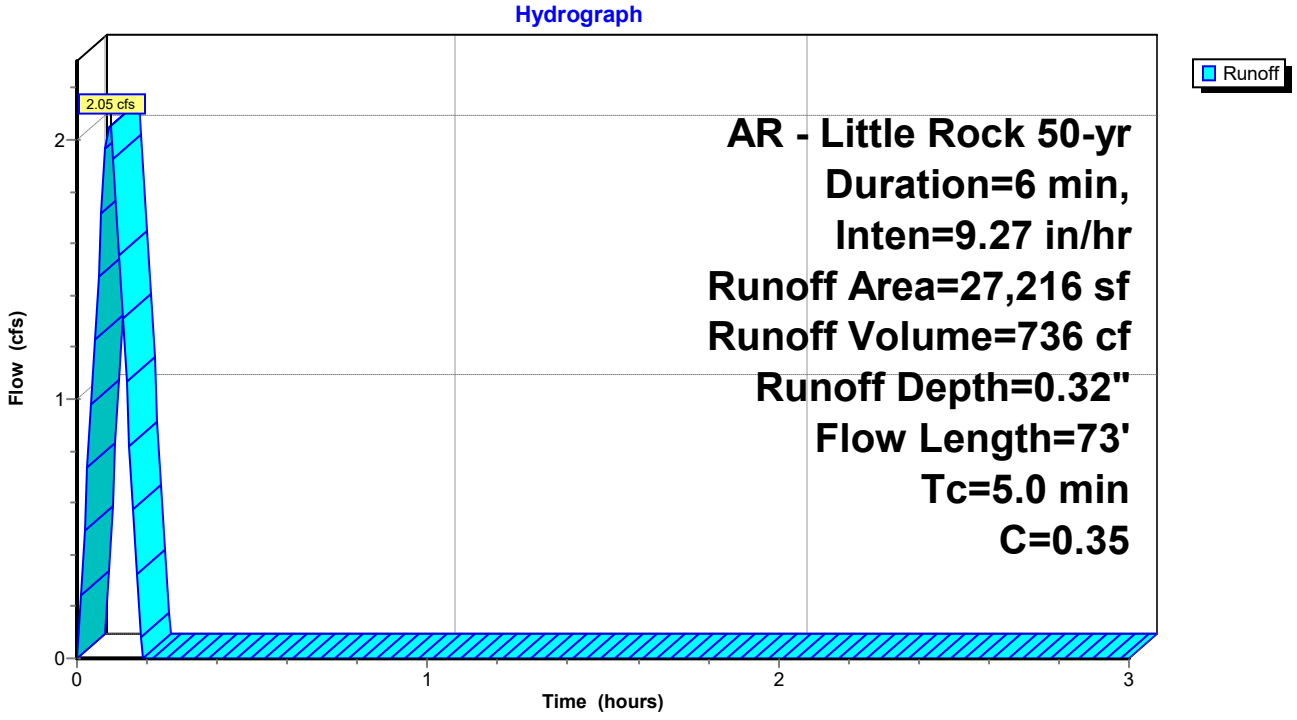
Runoff = 2.05 cfs @ 0.09 hrs, Volume= 736 cf, Depth= 0.32"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 50-yr Duration=6 min, Inten=9.27 in/hr

Area (sf)	C	Description
27,216	0.35	Sandy Soil 2-7% per manual
27,216		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	18	0.1667	2.61		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	55	0.0860	2.05		**Shallow Concentrated Flow, Overland Concentrated** Short Grass Pasture Kv= 7.0 fps
4.5					**Direct Entry, Minimum Adjustment**
5.0	73	Total			

Subcatchment B6: Drainage Basin B6



Summary for Subcatchment B7: Drainage Basin B7

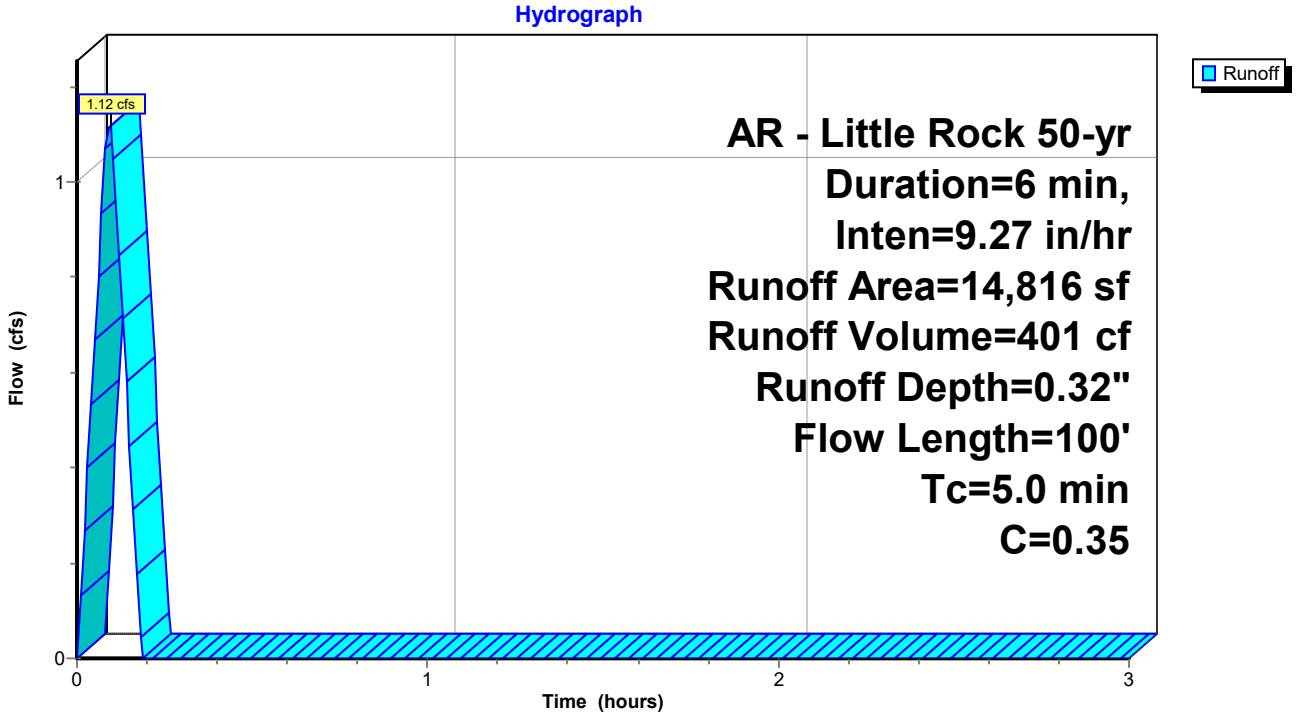
Runoff = 1.12 cfs @ 0.09 hrs, Volume= 401 cf, Depth= 0.32"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 50-yr Duration=6 min, Inten=9.27 in/hr

Area (sf)	C	Description
14,816	0.35	Sandy Soil 2-7% per manual
14,816		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	42	0.1667	3.09		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.5	58	0.0500	2.04		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
4.3					**Direct Entry, Minimum Adjustment**
5.0	100	Total			

Subcatchment B7: Drainage Basin B7



Summary for Pond CI-A1: CURB INLET A1

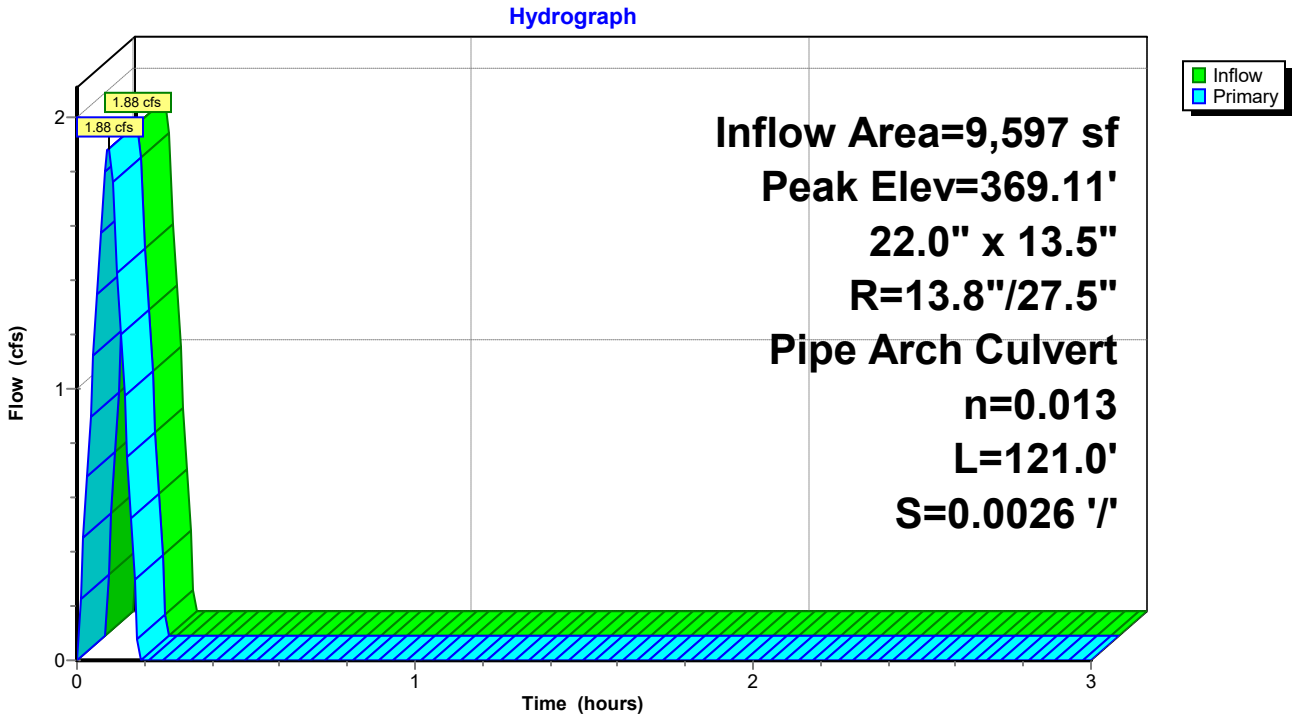
Inflow Area = 9,597 sf, 0.00% Impervious, Inflow Depth = 0.84" for 50-yr event
Inflow = 1.88 cfs @ 0.09 hrs, Volume= 675 cf
Outflow = 1.88 cfs @ 0.09 hrs, Volume= 675 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.88 cfs @ 0.09 hrs, Volume= 675 cf
Routed to Pond CI-A2 : CURB INLET A2

Routing by Stor-Ind method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Peak Elev= 369.11' @ 0.09 hrs

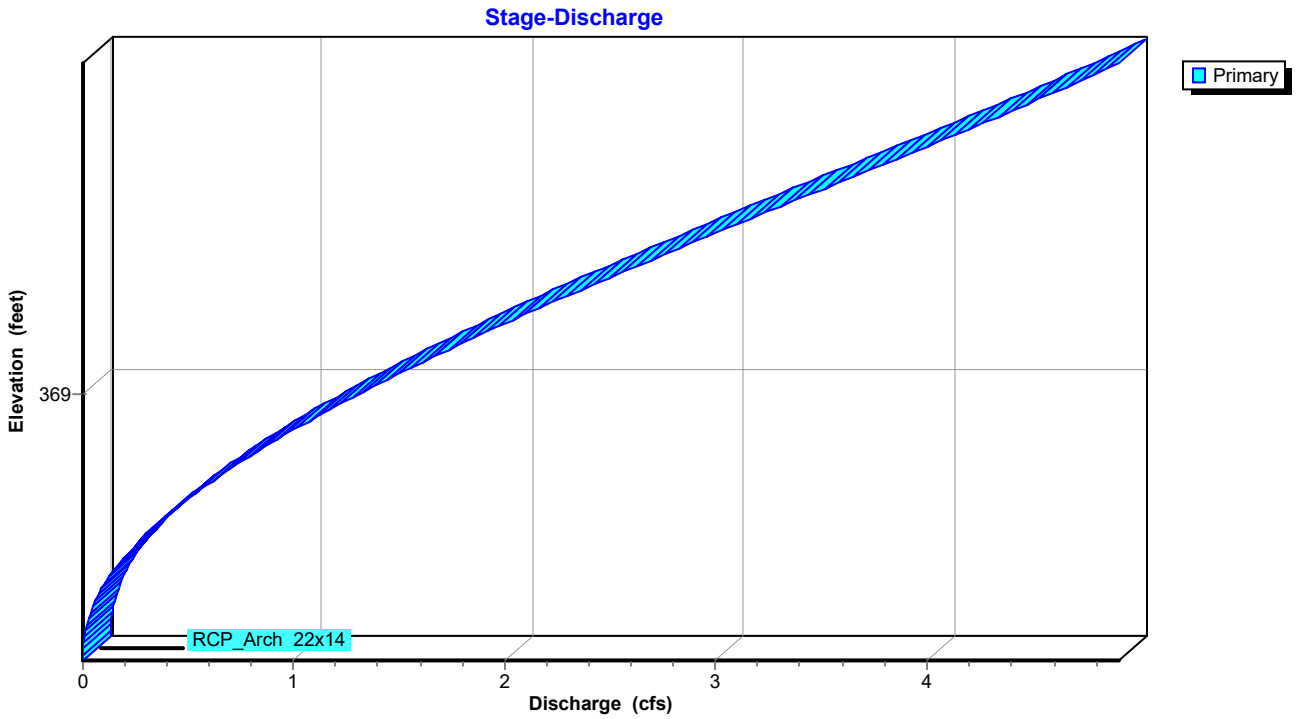
Device	Routing	Invert	Outlet Devices
#1	Primary	368.50'	**22.0" W x 13.5" H, R=13.8"/27.5" Pipe Arch RCP_Arch 22x14** L= 121.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 368.50' / 368.19' S= 0.0026 '/' Cc= 0.900 n= 0.013, Flow Area= 1.65 sf

Primary OutFlow Max=1.87 cfs @ 0.09 hrs HW=369.11' (Free Discharge)
1=RCP_Arch 22x14 (Barrel Controls 1.87 cfs @ 2.72 fps)

Pond CI-A1: CURB INLET A1



Pond CI-A1: CURB INLET A1



Stage-Area-Storage for Pond CI-A1: CURB INLET A1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
368.50	**0**	369.02	0	369.54	0
368.51	0	369.03	0	369.55	0
368.52	0	369.04	0	369.56	0
368.53	0	369.05	0	369.57	0
368.54	0	369.06	0	369.58	0
368.55	0	369.07	0	369.59	0
368.56	0	369.08	0	369.60	0
368.57	0	369.09	0	369.61	0
368.58	0	369.10	0	369.62	0
368.59	0	369.11	0		
368.60	0	369.12	0		
368.61	0	369.13	0		
368.62	0	369.14	0		
368.63	0	369.15	0		
368.64	0	369.16	0		
368.65	0	369.17	0		
368.66	0	369.18	0		
368.67	0	369.19	0		
368.68	0	369.20	0		
368.69	0	369.21	0		
368.70	0	369.22	0		
368.71	0	369.23	0		
368.72	0	369.24	0		
368.73	0	369.25	0		
368.74	0	369.26	0		
368.75	0	369.27	0		
368.76	0	369.28	0		
368.77	0	369.29	0		
368.78	0	369.30	0		
368.79	0	369.31	0		
368.80	0	369.32	0		
368.81	0	369.33	0		
368.82	0	369.34	0		
368.83	0	369.35	0		
368.84	0	369.36	0		
368.85	0	369.37	0		
368.86	0	369.38	0		
368.87	0	369.39	0		
368.88	0	369.40	0		
368.89	0	369.41	0		
368.90	0	369.42	0		
368.91	0	369.43	0		
368.92	0	369.44	0		
368.93	0	369.45	0		
368.94	0	369.46	0		
368.95	0	369.47	0		
368.96	0	369.48	0		
368.97	0	369.49	0		
368.98	0	369.50	0		
368.99	0	369.51	0		
369.00	0	369.52	0		
369.01	0	369.53	0		

Summary for Pond CI-A2: CURB INLET A2

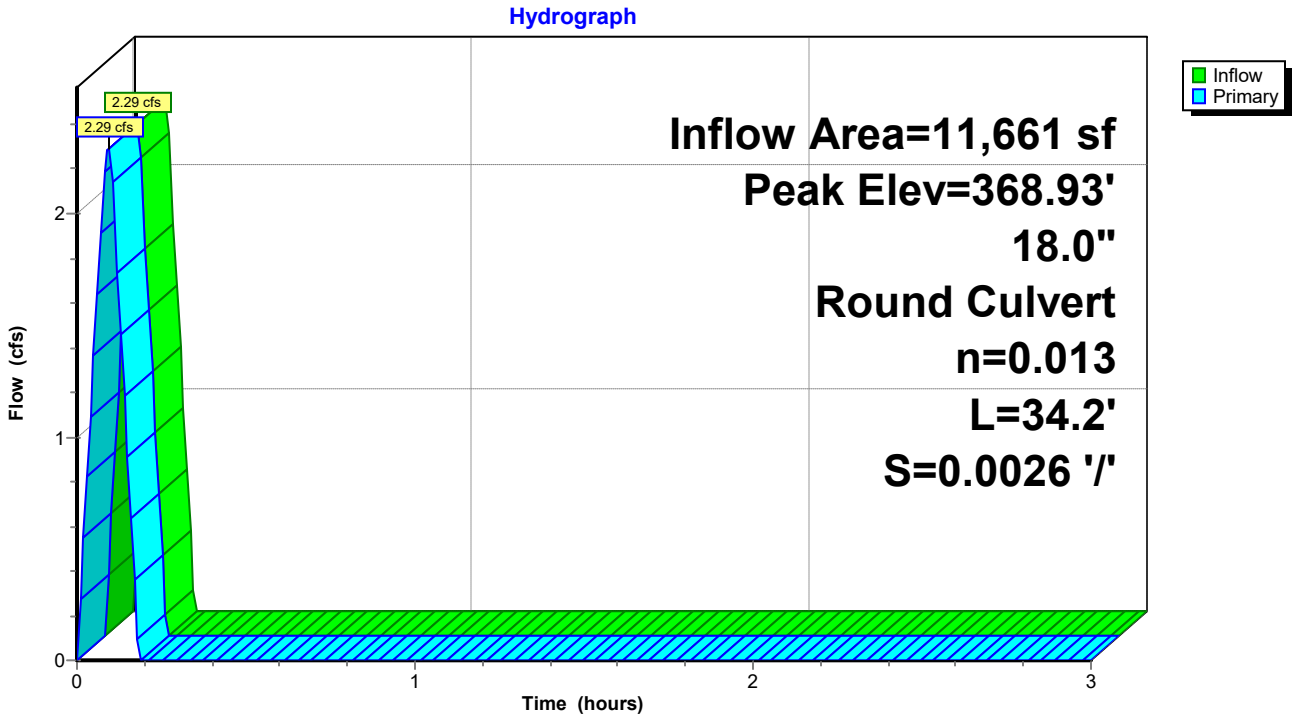
Inflow Area = 11,661 sf, 0.00% Impervious, Inflow Depth = 0.85" for 50-yr event
Inflow = 2.29 cfs @ 0.09 hrs, Volume= 821 cf
Outflow = 2.29 cfs @ 0.09 hrs, Volume= 821 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.29 cfs @ 0.09 hrs, Volume= 821 cf
Routed to Link POST-DEV : Post-Development

Routing by Stor-Ind method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Peak Elev= 368.93' @ 0.09 hrs

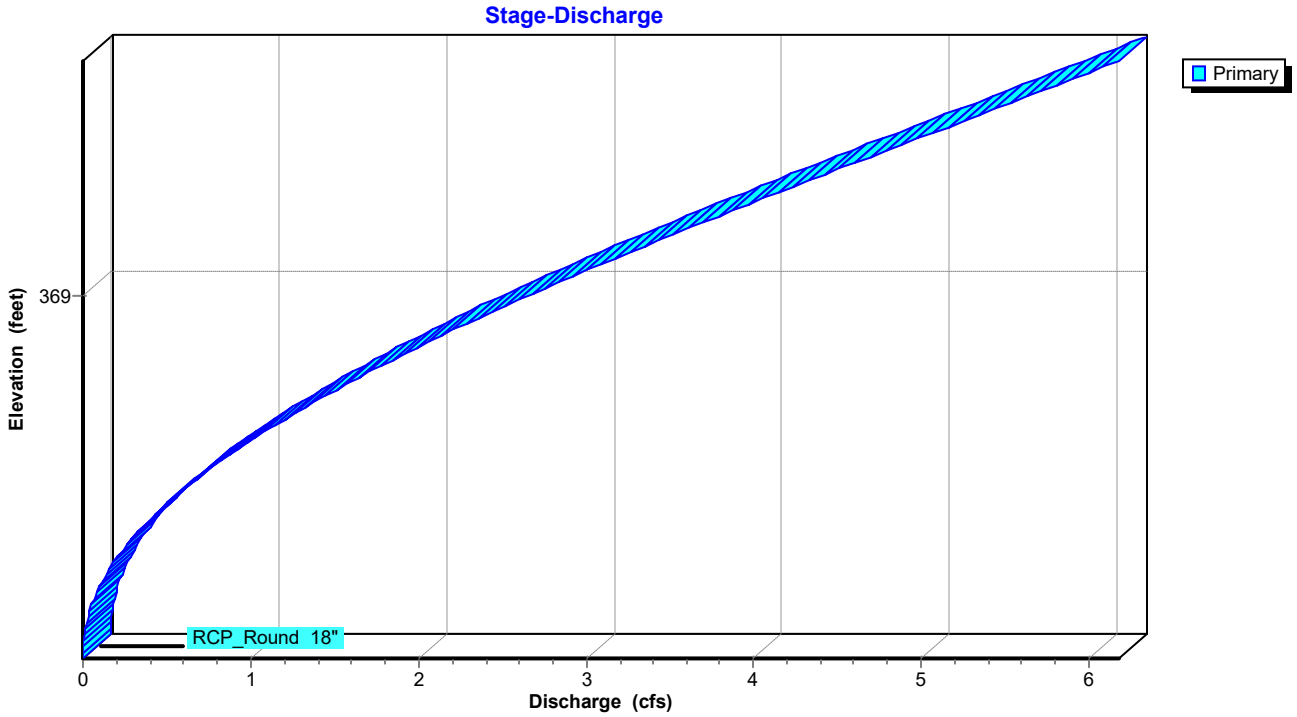
Device	Routing	Invert	Outlet Devices
#1	Primary	368.09'	**18.0" Round RCP_Round 18"** L= 34.2' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 368.09' / 368.00' S= 0.0026 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=2.28 cfs @ 0.09 hrs HW=368.93' (Free Discharge)
1=RCP_Round 18" (Barrel Controls 2.28 cfs @ 3.24 fps)

Pond CI-A2: CURB INLET A2



Pond CI-A2: CURB INLET A2



Stage-Area-Storage for Pond CI-A2: CURB INLET A2

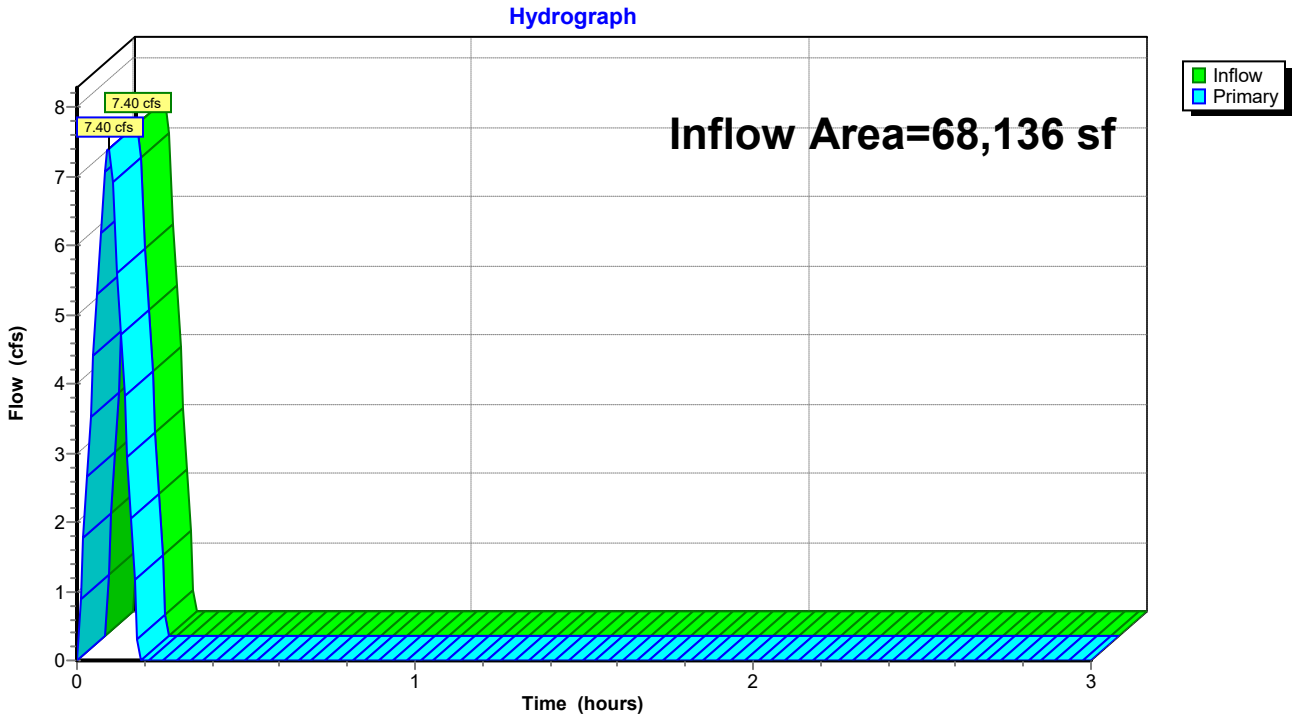
Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
368.09	**0**	368.61	0	369.13	0
368.10	0	368.62	0	369.14	0
368.11	0	368.63	0	369.15	0
368.12	0	368.64	0	369.16	0
368.13	0	368.65	0	369.17	0
368.14	0	368.66	0	369.18	0
368.15	0	368.67	0	369.19	0
368.16	0	368.68	0	369.20	0
368.17	0	368.69	0	369.21	0
368.18	0	368.70	0	369.22	0
368.19	0	368.71	0	369.23	0
368.20	0	368.72	0	369.24	0
368.21	0	368.73	0	369.25	0
368.22	0	368.74	0	369.26	0
368.23	0	368.75	0	369.27	0
368.24	0	368.76	0	369.28	0
368.25	0	368.77	0	369.29	0
368.26	0	368.78	0	369.30	0
368.27	0	368.79	0	369.31	0
368.28	0	368.80	0	369.32	0
368.29	0	368.81	0	369.33	0
368.30	0	368.82	0	369.34	0
368.31	0	368.83	0	369.35	0
368.32	0	368.84	0	369.36	0
368.33	0	368.85	0	369.37	0
368.34	0	368.86	0	369.38	0
368.35	0	368.87	0	369.39	0
368.36	0	368.88	0	369.40	0
368.37	0	368.89	0	369.41	0
368.38	0	368.90	0	369.42	0
368.39	0	368.91	0	369.43	0
368.40	0	368.92	0	369.44	0
368.41	0	368.93	0	369.45	0
368.42	0	368.94	0	369.46	0
368.43	0	368.95	0	369.47	0
368.44	0	368.96	0	369.48	0
368.45	0	368.97	0	369.49	0
368.46	0	368.98	0	369.50	0
368.47	0	368.99	0	369.51	0
368.48	0	369.00	0	369.52	0
368.49	0	369.01	0	369.53	0
368.50	0	369.02	0	369.54	0
368.51	0	369.03	0	369.55	0
368.52	0	369.04	0	369.56	0
368.53	0	369.05	0	369.57	0
368.54	0	369.06	0	369.58	0
368.55	0	369.07	0	369.59	0
368.56	0	369.08	0		
368.57	0	369.09	0		
368.58	0	369.10	0		
368.59	0	369.11	0		
368.60	0	369.12	0		

Summary for Link POST-DEV: Post-Development

Inflow Area = 68,136 sf, 0.00% Impervious, Inflow Depth = 0.47" for 50-yr event
Inflow = 7.40 cfs @ 0.09 hrs, Volume= 2,651 cf
Primary = 7.40 cfs @ 0.09 hrs, Volume= 2,651 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs

Link POST-DEV: Post-Development



Summary for Subcatchment B1: Drainage Basin B1

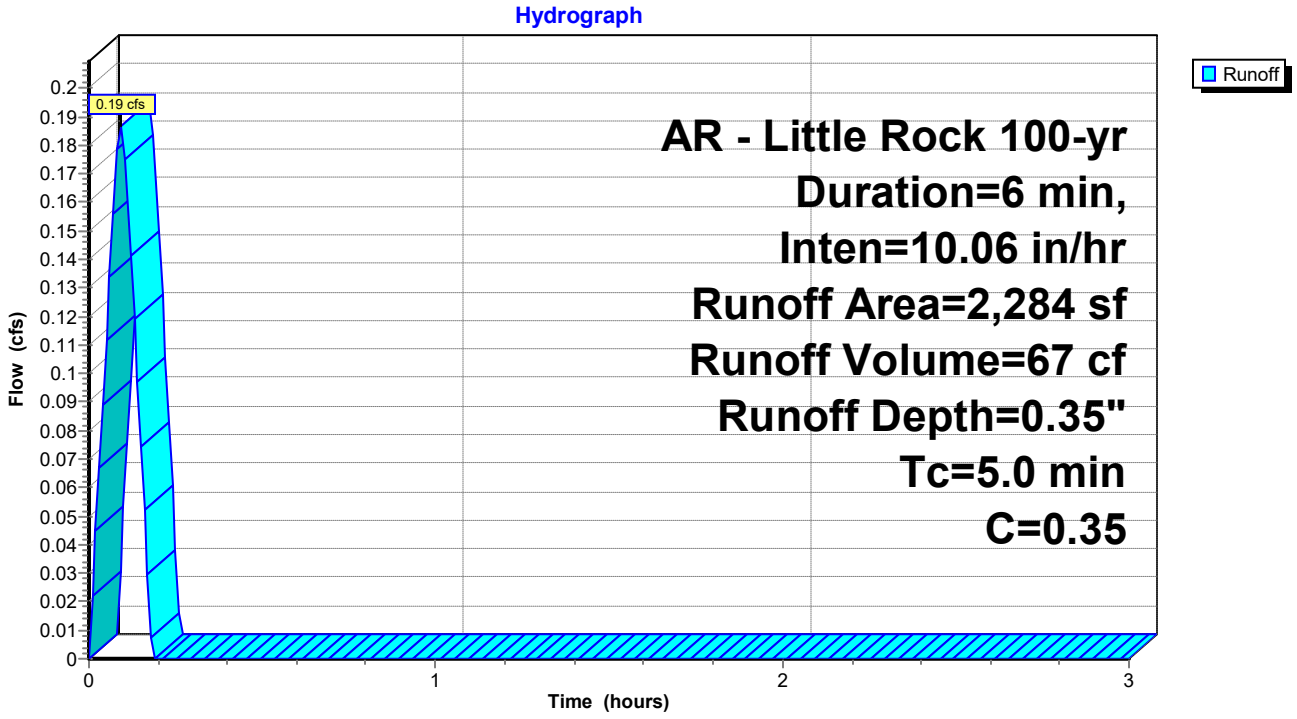
Runoff = 0.19 cfs @ 0.09 hrs, Volume= 67 cf, Depth= 0.35"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 100-yr Duration=6 min, Inten=10.06 in/hr

Area (sf)	C	Description
2,284	0.35	Sandy Soil 2-7% per manual
0	0.92	Paved Areas
2,284	0.35	Weighted Average
2,284		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					**Direct Entry, Minimum Adjustment**

Subcatchment B1: Drainage Basin B1



Summary for Subcatchment B2: Drainage Basin B2

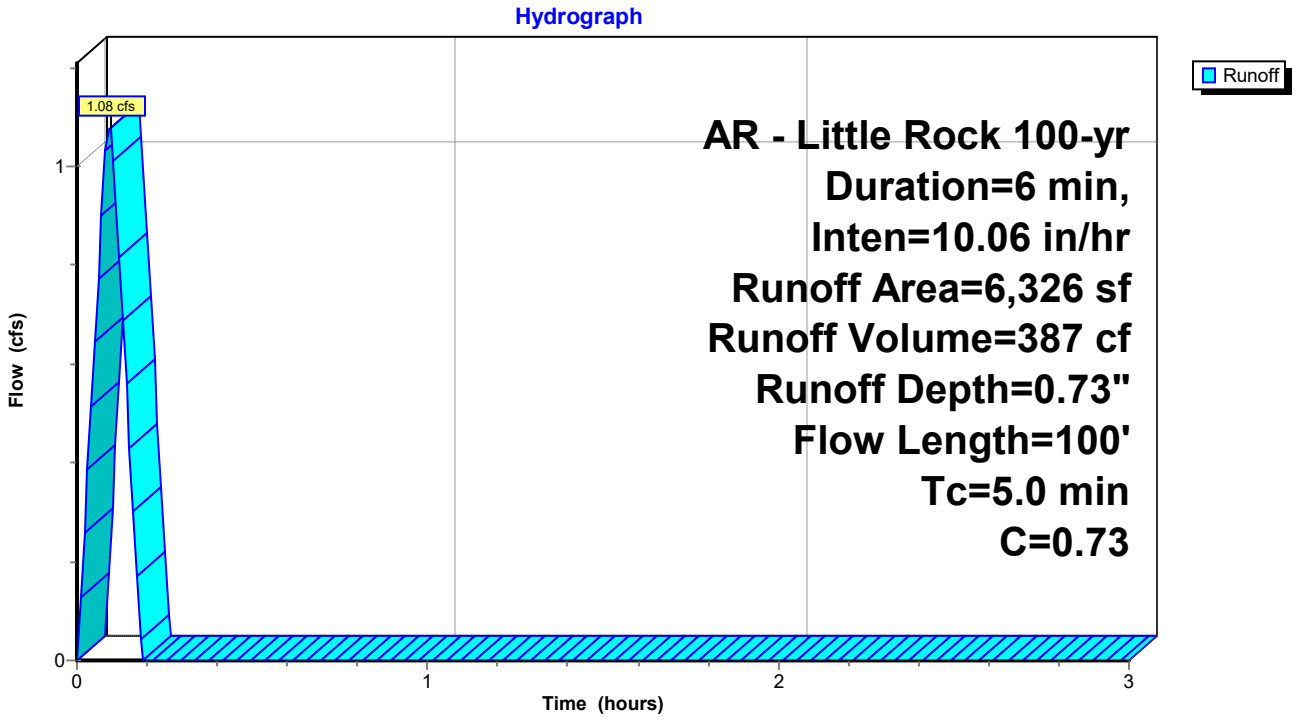
Runoff = 1.08 cfs @ 0.09 hrs, Volume= 387 cf, Depth= 0.73"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 100-yr Duration=6 min, Inten=10.06 in/hr

Area (sf)	C	Description
2,115	0.35	Sandy Soil 2-7% per manual
4,211	0.92	Paved Areas
6,326	0.73	Weighted Average
6,326		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	42	0.1667	3.09		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.5	58	0.0500	2.04		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
4.3					**Direct Entry, Minimum Adjustment**
5.0	100	Total			

Subcatchment B2: Drainage Basin B2



Summary for Subcatchment B3: Drainage Basin B3

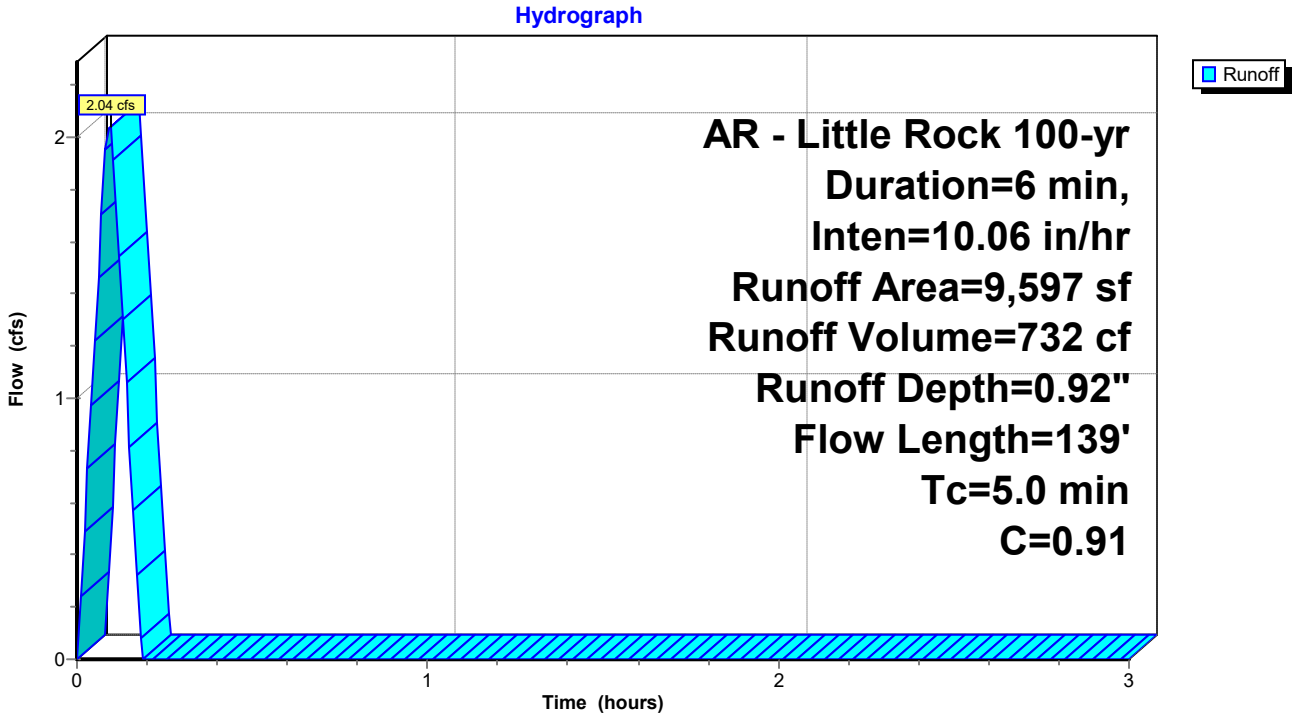
Runoff = 2.04 cfs @ 0.09 hrs, Volume= 732 cf, Depth= 0.92"
Routed to Pond CI-A1 : CURB INLET A1

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 100-yr Duration=6 min, Inten=10.06 in/hr

Area (sf)	C	Description
155	0.35	Sandy Soil 2-7% per manual
9,442	0.92	Paved Areas
9,597	0.91	Weighted Average
9,597		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	28	0.1667	2.85		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	30	0.0160	1.13		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.4	41	0.0520	1.93		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.2	40	0.0360	3.85		**Shallow Concentrated Flow, Gutter Flow** Paved Kv= 20.3 fps
3.8					**Direct Entry, Minimum Adjustment**
5.0	139	Total			

Subcatchment B3: Drainage Basin B3



Summary for Subcatchment B4: Drainage Basin B4

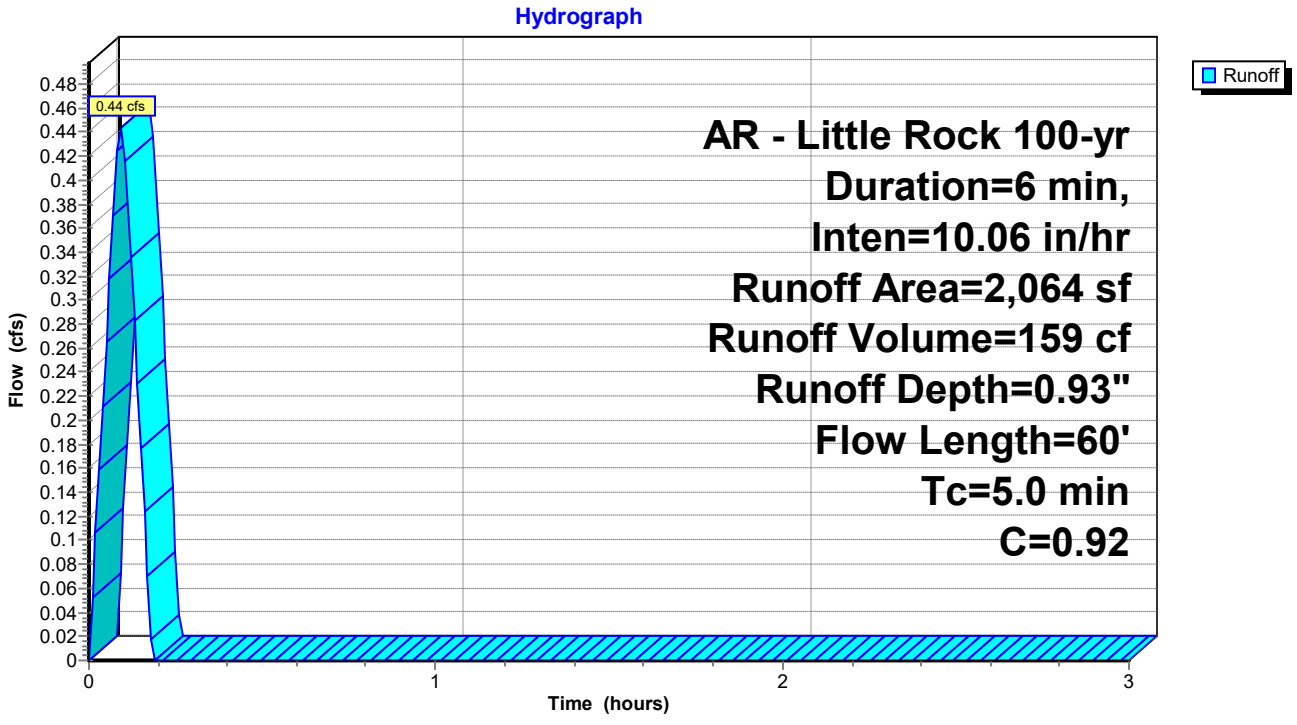
Runoff = 0.44 cfs @ 0.09 hrs, Volume= 159 cf, Depth= 0.93"
Routed to Pond CI-A2 : CURB INLET A2

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 100-yr Duration=6 min, Inten=10.06 in/hr

Area (sf)	C	Description
0	0.35	Sandy Soil 2-7% per manual
2,064	0.92	Paved Areas
2,064	0.92	Weighted Average
2,064		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	45	0.0170	1.26		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
0.0	15	0.0840	5.88		**Shallow Concentrated Flow, Gutter Flow** Paved Kv= 20.3 fps
4.4					**Direct Entry, Minimum Adjustment**
5.0	60	Total			

Subcatchment B4: Drainage Basin B4



Summary for Subcatchment B5: Drainage Basin B5

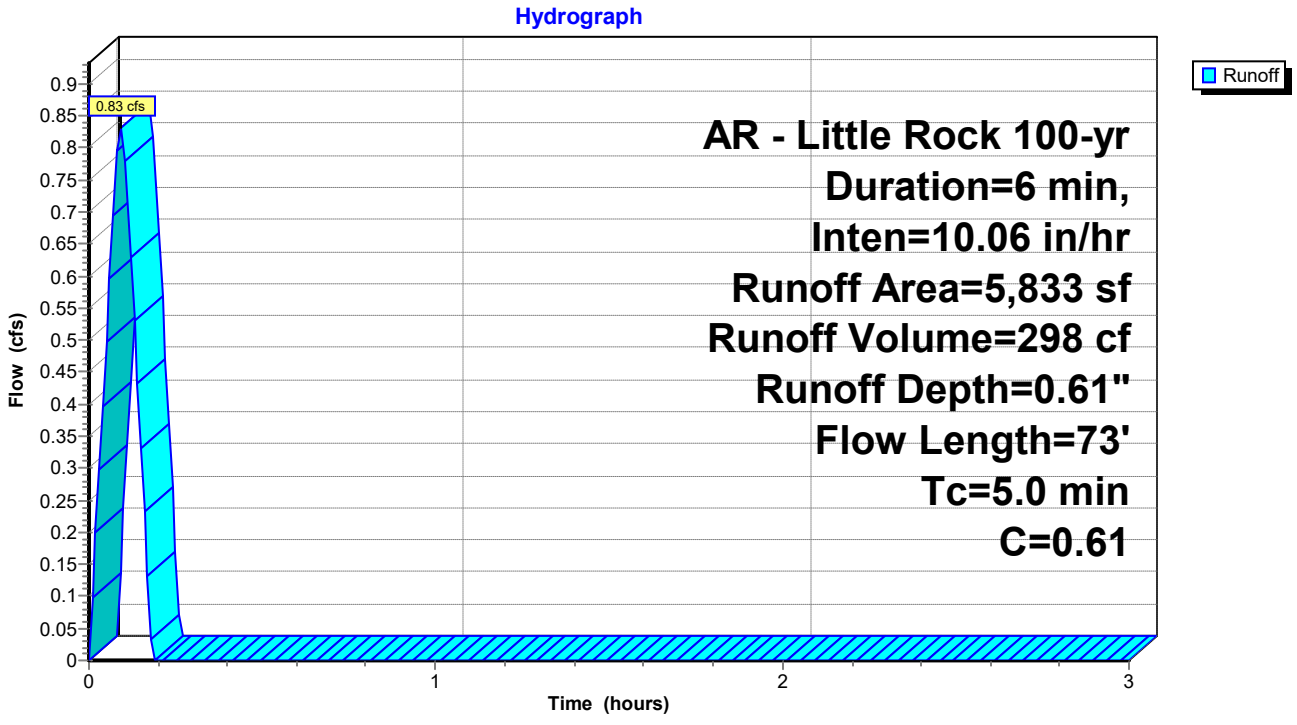
Runoff = 0.83 cfs @ 0.09 hrs, Volume= 298 cf, Depth= 0.61"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 100-yr Duration=6 min, Inten=10.06 in/hr

Area (sf)	C	Description
3,123	0.35	Sandy Soil 2-7% per manual
2,710	0.92	Paved Areas
5,833	0.61	Weighted Average
5,833		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	18	0.1667	2.61		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	55	0.0860	2.05		**Shallow Concentrated Flow, Overland Concentrated** Short Grass Pasture Kv= 7.0 fps
4.5					**Direct Entry, Minimum Adjustment**
5.0	73	Total			

Subcatchment B5: Drainage Basin B5



Summary for Subcatchment B6: Drainage Basin B6

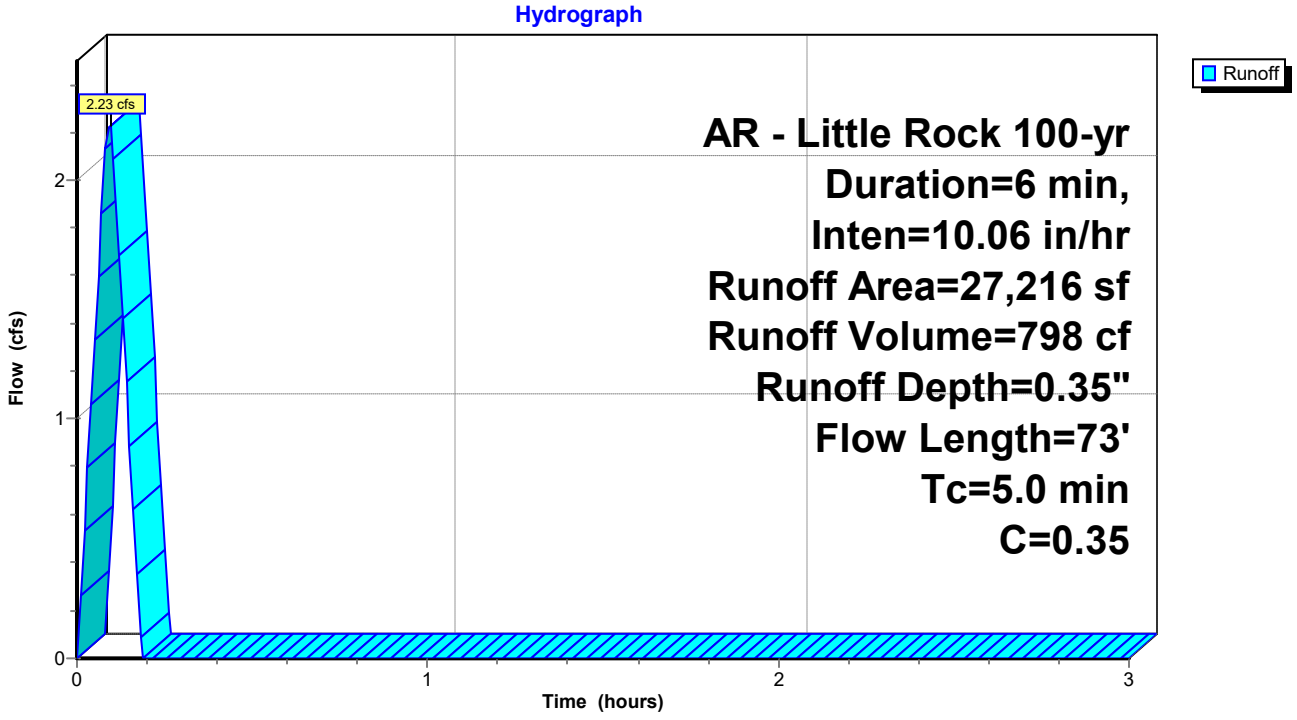
Runoff = 2.23 cfs @ 0.09 hrs, Volume= 798 cf, Depth= 0.35"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 100-yr Duration=6 min, Inten=10.06 in/hr

Area (sf)	C	Description
27,216	0.35	Sandy Soil 2-7% per manual
27,216		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.1	18	0.1667	2.61		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.4	55	0.0860	2.05		**Shallow Concentrated Flow, Overland Concentrated** Short Grass Pasture Kv= 7.0 fps
4.5					**Direct Entry, Minimum Adjustment**
5.0	73	Total			

Subcatchment B6: Drainage Basin B6



Summary for Subcatchment B7: Drainage Basin B7

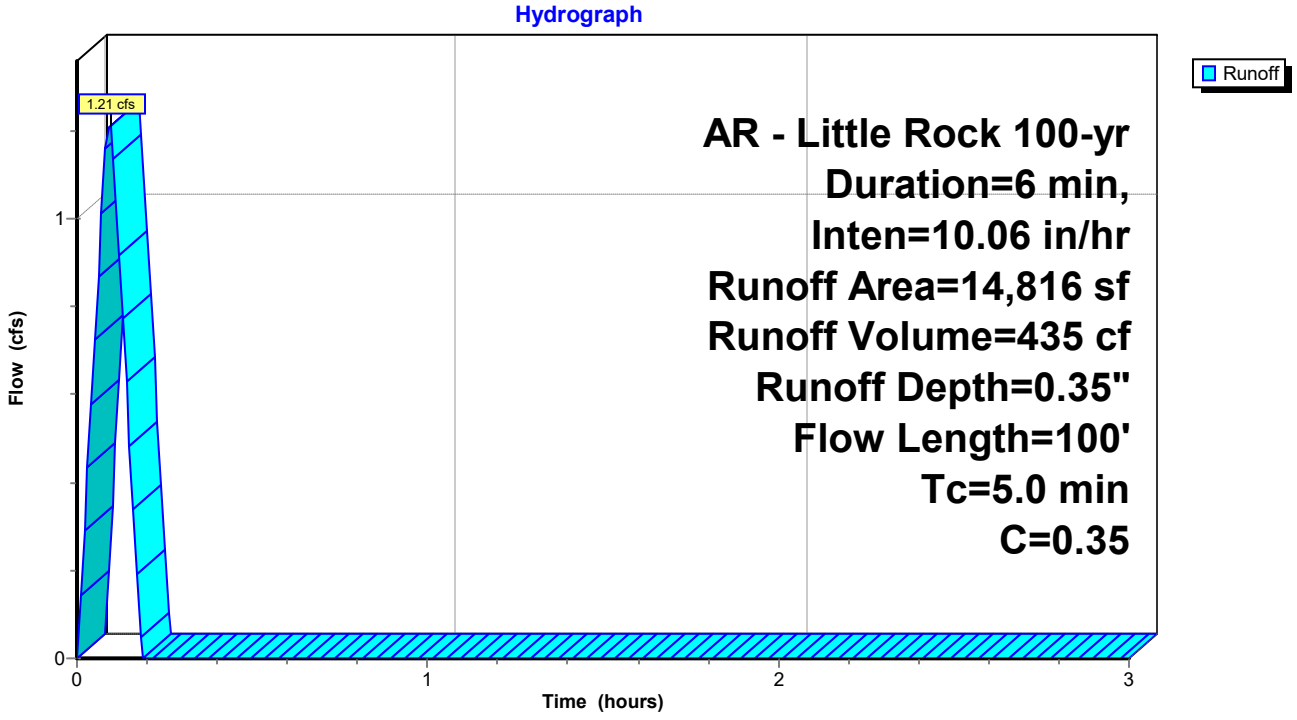
Runoff = 1.21 cfs @ 0.09 hrs, Volume= 435 cf, Depth= 0.35"
Routed to Link POST-DEV : Post-Development

Runoff by Rational method, Rise/Fall=1.0/1.0 xTc, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
AR - Little Rock 100-yr Duration=6 min, Inten=10.06 in/hr

Area (sf)	C	Description
14,816	0.35	Sandy Soil 2-7% per manual
14,816		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	42	0.1667	3.09		**Sheet Flow, Rooftop** Smooth surfaces n= 0.011 P2= 4.20"
0.5	58	0.0500	2.04		**Sheet Flow, Asphalt Sheet Flow** Smooth surfaces n= 0.011 P2= 4.20"
4.3					**Direct Entry, Minimum Adjustment**
5.0	100	Total			

Subcatchment B7: Drainage Basin B7



Summary for Pond CI-A1: CURB INLET A1

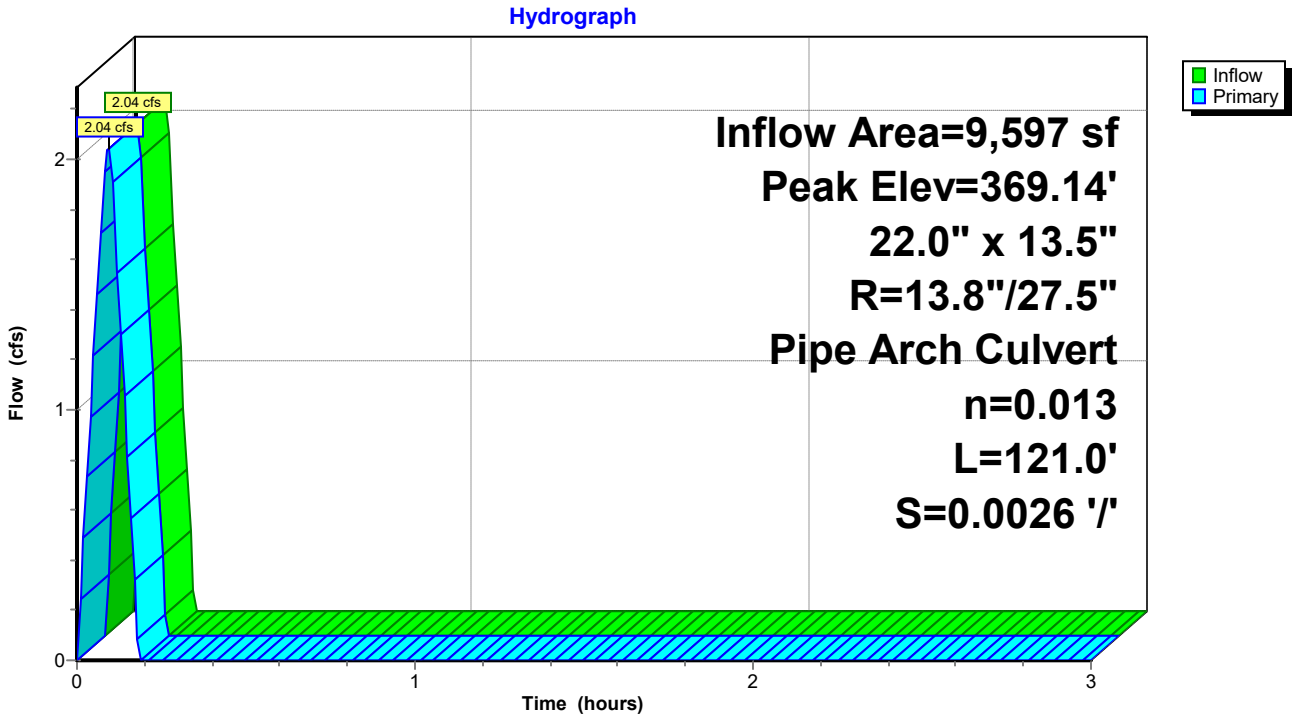
Inflow Area = 9,597 sf, 0.00% Impervious, Inflow Depth = 0.92" for 100-yr event
Inflow = 2.04 cfs @ 0.09 hrs, Volume= 732 cf
Outflow = 2.04 cfs @ 0.09 hrs, Volume= 732 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.04 cfs @ 0.09 hrs, Volume= 732 cf
Routed to Pond CI-A2 : CURB INLET A2

Routing by Stor-Ind method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Peak Elev= 369.14' @ 0.09 hrs

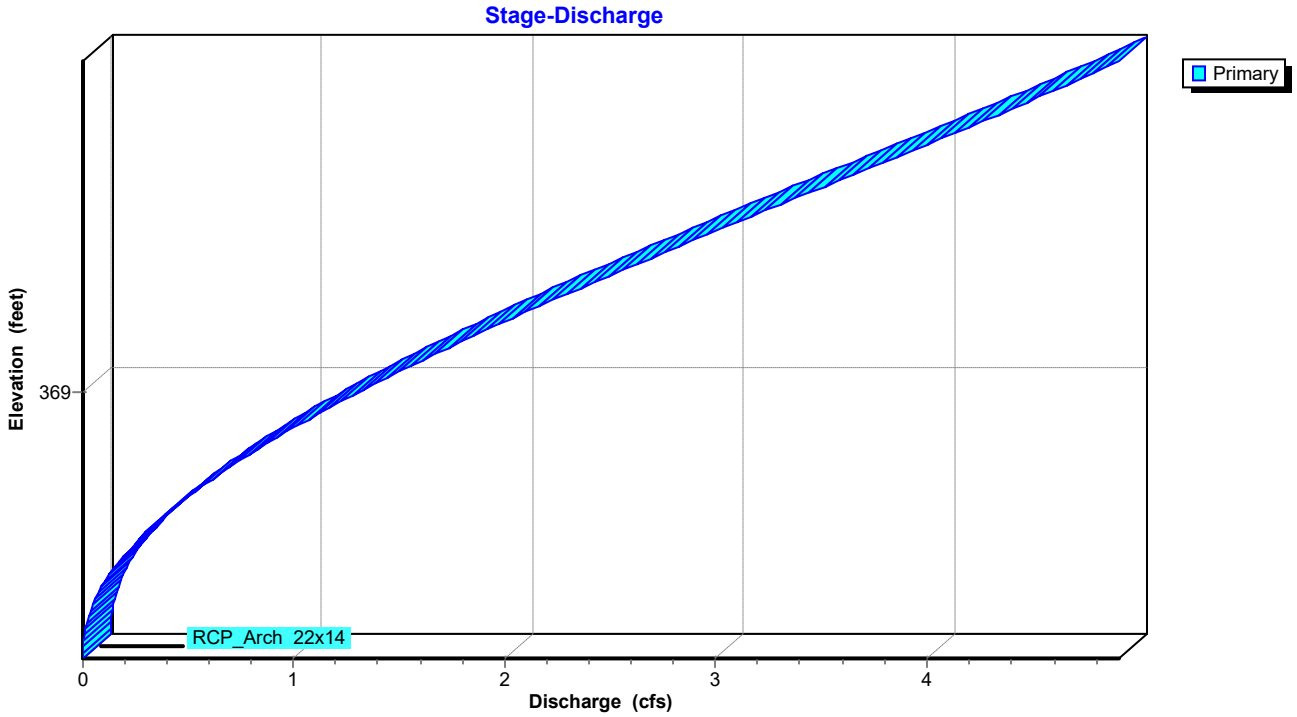
Device	Routing	Invert	Outlet Devices
#1	Primary	368.50'	**22.0" W x 13.5" H, R=13.8"/27.5" Pipe Arch RCP_Arch 22x14** L= 121.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 368.50' / 368.19' S= 0.0026 '/' Cc= 0.900 n= 0.013, Flow Area= 1.65 sf

Primary OutFlow Max=2.03 cfs @ 0.09 hrs HW=369.14' (Free Discharge)
1=RCP_Arch 22x14 (Barrel Controls 2.03 cfs @ 2.79 fps)

Pond CI-A1: CURB INLET A1



Pond CI-A1: CURB INLET A1



Stage-Area-Storage for Pond CI-A1: CURB INLET A1

Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
368.50	**0**	369.02	0	369.54	0
368.51	0	369.03	0	369.55	0
368.52	0	369.04	0	369.56	0
368.53	0	369.05	0	369.57	0
368.54	0	369.06	0	369.58	0
368.55	0	369.07	0	369.59	0
368.56	0	369.08	0	369.60	0
368.57	0	369.09	0	369.61	0
368.58	0	369.10	0	369.62	0
368.59	0	369.11	0		
368.60	0	369.12	0		
368.61	0	369.13	0		
368.62	0	369.14	0		
368.63	0	369.15	0		
368.64	0	369.16	0		
368.65	0	369.17	0		
368.66	0	369.18	0		
368.67	0	369.19	0		
368.68	0	369.20	0		
368.69	0	369.21	0		
368.70	0	369.22	0		
368.71	0	369.23	0		
368.72	0	369.24	0		
368.73	0	369.25	0		
368.74	0	369.26	0		
368.75	0	369.27	0		
368.76	0	369.28	0		
368.77	0	369.29	0		
368.78	0	369.30	0		
368.79	0	369.31	0		
368.80	0	369.32	0		
368.81	0	369.33	0		
368.82	0	369.34	0		
368.83	0	369.35	0		
368.84	0	369.36	0		
368.85	0	369.37	0		
368.86	0	369.38	0		
368.87	0	369.39	0		
368.88	0	369.40	0		
368.89	0	369.41	0		
368.90	0	369.42	0		
368.91	0	369.43	0		
368.92	0	369.44	0		
368.93	0	369.45	0		
368.94	0	369.46	0		
368.95	0	369.47	0		
368.96	0	369.48	0		
368.97	0	369.49	0		
368.98	0	369.50	0		
368.99	0	369.51	0		
369.00	0	369.52	0		
369.01	0	369.53	0		

Summary for Pond CI-A2: CURB INLET A2

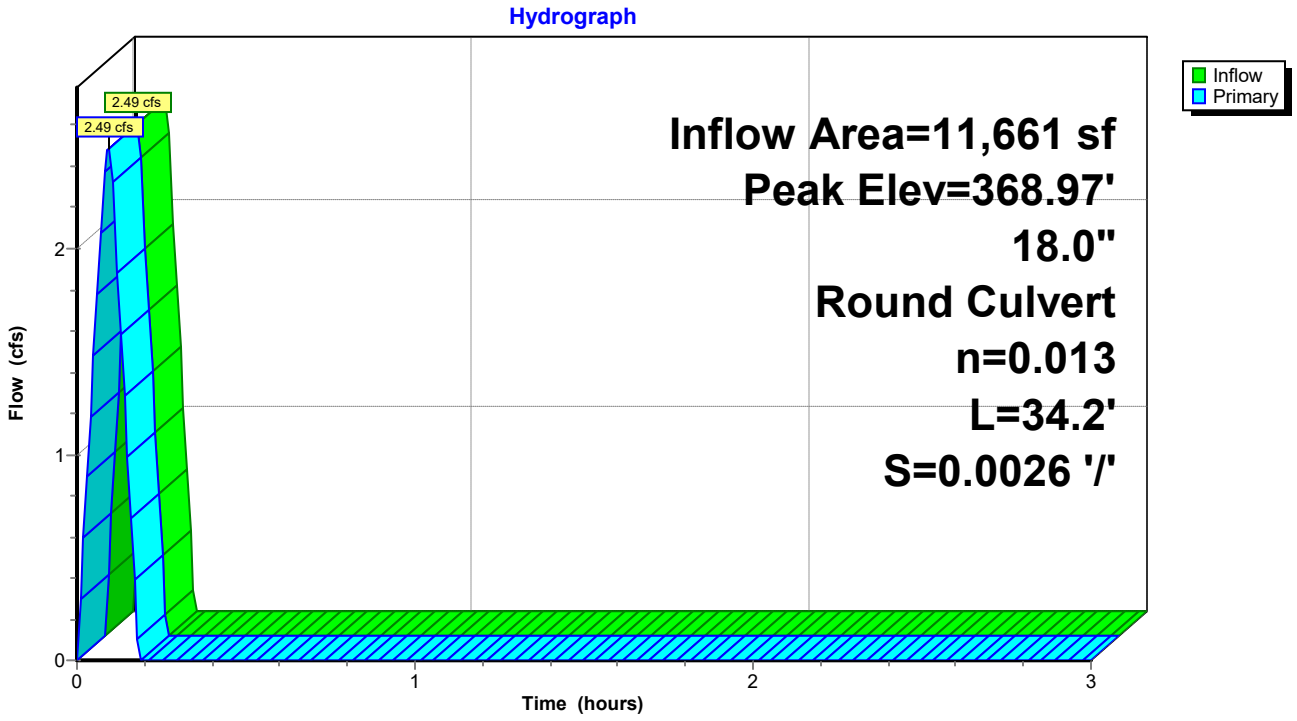
Inflow Area = 11,661 sf, 0.00% Impervious, Inflow Depth = 0.92" for 100-yr event
Inflow = 2.49 cfs @ 0.09 hrs, Volume= 891 cf
Outflow = 2.49 cfs @ 0.09 hrs, Volume= 891 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.49 cfs @ 0.09 hrs, Volume= 891 cf
Routed to Link POST-DEV : Post-Development

Routing by Stor-Ind method, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs
Peak Elev= 368.97' @ 0.09 hrs

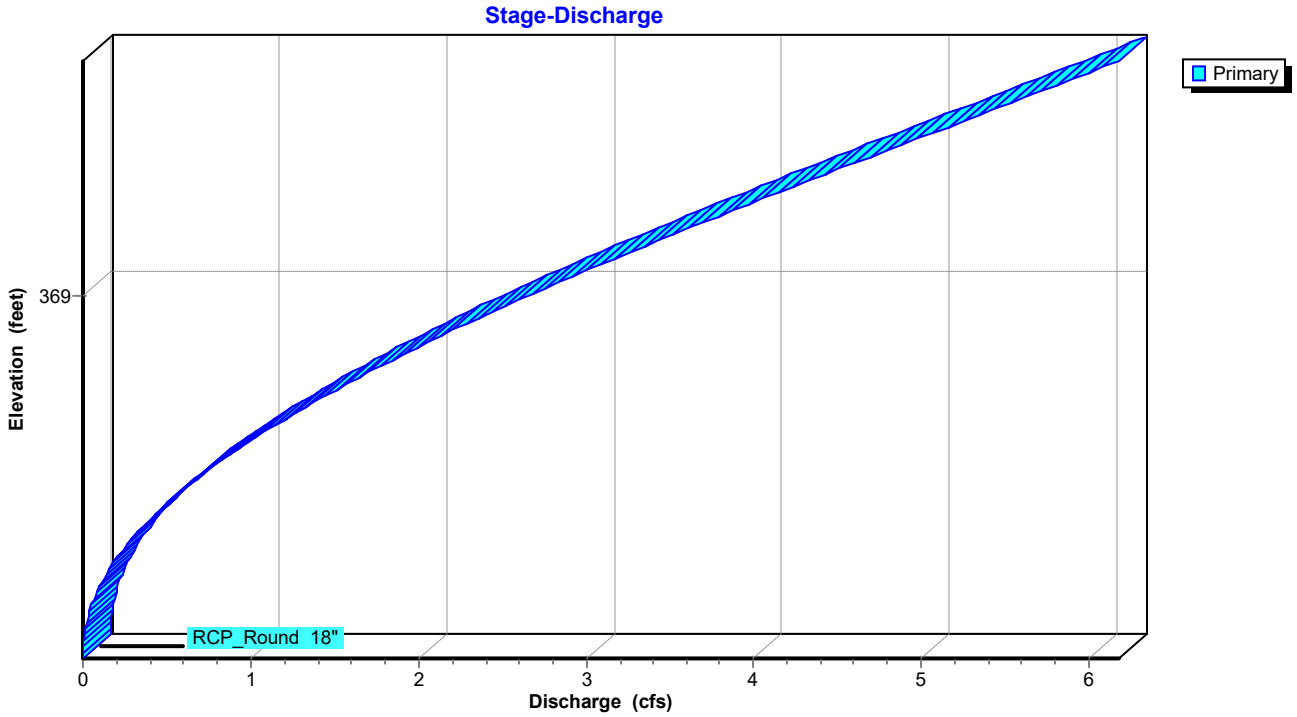
Device	Routing	Invert	Outlet Devices
#1	Primary	368.09'	**18.0" Round RCP_Round 18"** L= 34.2' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 368.09' / 368.00' S= 0.0026 '/' Cc= 0.900 n= 0.013, Flow Area= 1.77 sf

Primary OutFlow Max=2.47 cfs @ 0.09 hrs HW=368.97' (Free Discharge)
1=RCP_Round 18" (Barrel Controls 2.47 cfs @ 3.32 fps)

Pond CI-A2: CURB INLET A2



Pond CI-A2: CURB INLET A2



Stage-Area-Storage for Pond CI-A2: CURB INLET A2

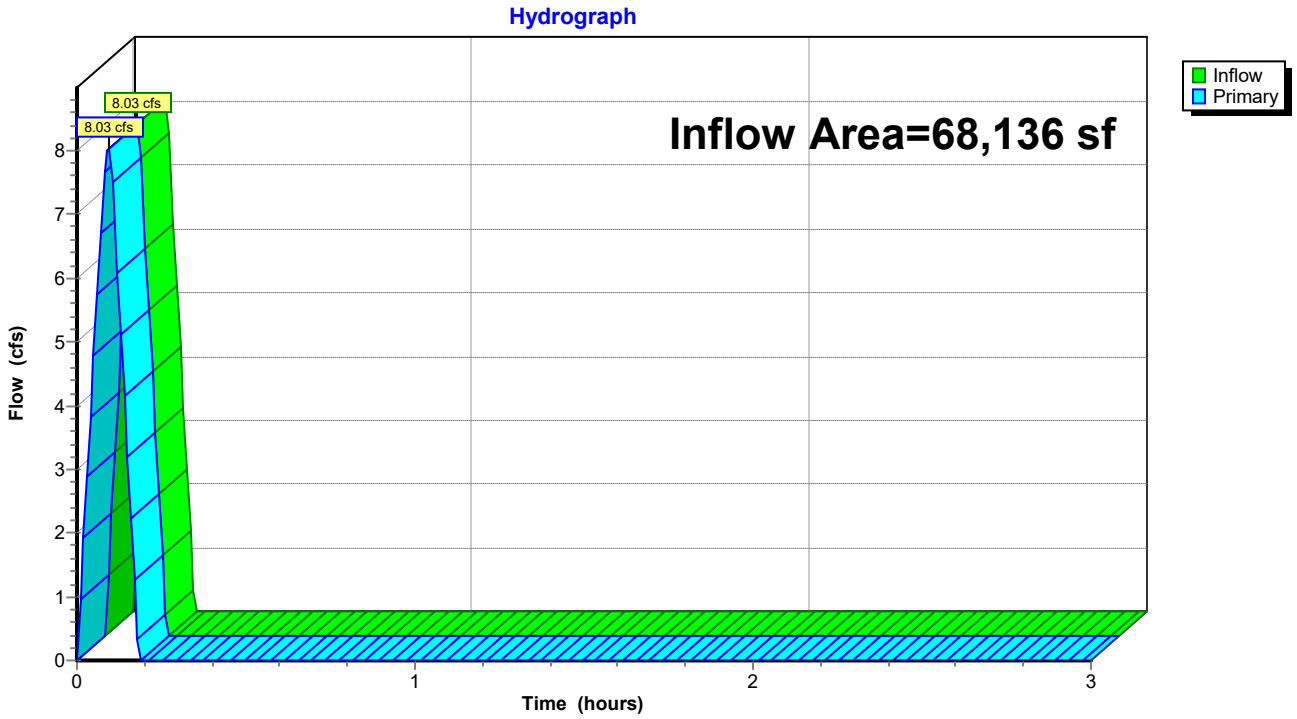
Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)	Elevation (feet)	Storage (cubic-feet)
368.09	**0**	368.61	0	369.13	0
368.10	0	368.62	0	369.14	0
368.11	0	368.63	0	369.15	0
368.12	0	368.64	0	369.16	0
368.13	0	368.65	0	369.17	0
368.14	0	368.66	0	369.18	0
368.15	0	368.67	0	369.19	0
368.16	0	368.68	0	369.20	0
368.17	0	368.69	0	369.21	0
368.18	0	368.70	0	369.22	0
368.19	0	368.71	0	369.23	0
368.20	0	368.72	0	369.24	0
368.21	0	368.73	0	369.25	0
368.22	0	368.74	0	369.26	0
368.23	0	368.75	0	369.27	0
368.24	0	368.76	0	369.28	0
368.25	0	368.77	0	369.29	0
368.26	0	368.78	0	369.30	0
368.27	0	368.79	0	369.31	0
368.28	0	368.80	0	369.32	0
368.29	0	368.81	0	369.33	0
368.30	0	368.82	0	369.34	0
368.31	0	368.83	0	369.35	0
368.32	0	368.84	0	369.36	0
368.33	0	368.85	0	369.37	0
368.34	0	368.86	0	369.38	0
368.35	0	368.87	0	369.39	0
368.36	0	368.88	0	369.40	0
368.37	0	368.89	0	369.41	0
368.38	0	368.90	0	369.42	0
368.39	0	368.91	0	369.43	0
368.40	0	368.92	0	369.44	0
368.41	0	368.93	0	369.45	0
368.42	0	368.94	0	369.46	0
368.43	0	368.95	0	369.47	0
368.44	0	368.96	0	369.48	0
368.45	0	368.97	0	369.49	0
368.46	0	368.98	0	369.50	0
368.47	0	368.99	0	369.51	0
368.48	0	369.00	0	369.52	0
368.49	0	369.01	0	369.53	0
368.50	0	369.02	0	369.54	0
368.51	0	369.03	0	369.55	0
368.52	0	369.04	0	369.56	0
368.53	0	369.05	0	369.57	0
368.54	0	369.06	0	369.58	0
368.55	0	369.07	0	369.59	0
368.56	0	369.08	0		
368.57	0	369.09	0		
368.58	0	369.10	0		
368.59	0	369.11	0		
368.60	0	369.12	0		

Summary for Link POST-DEV: Post-Development

Inflow Area = 68,136 sf, 0.00% Impervious, Inflow Depth = 0.51" for 100-yr event
Inflow = 8.03 cfs @ 0.09 hrs, Volume= 2,876 cf
Primary = 8.03 cfs @ 0.09 hrs, Volume= 2,876 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-3.00 hrs, dt= 0.01 hrs

Link POST-DEV: Post-Development



STORM SEWER SIZING

Inlet Report

CI-A1 (25 YEAR)

Curb Inlet

Location	= Sag
Curb Length (ft)	= 4.00
Throat Height (in)	= 4.00
Grate Area (sqft)	= -0-
Grate Width (ft)	= -0-
Grate Length (ft)	= -0-

Gutter

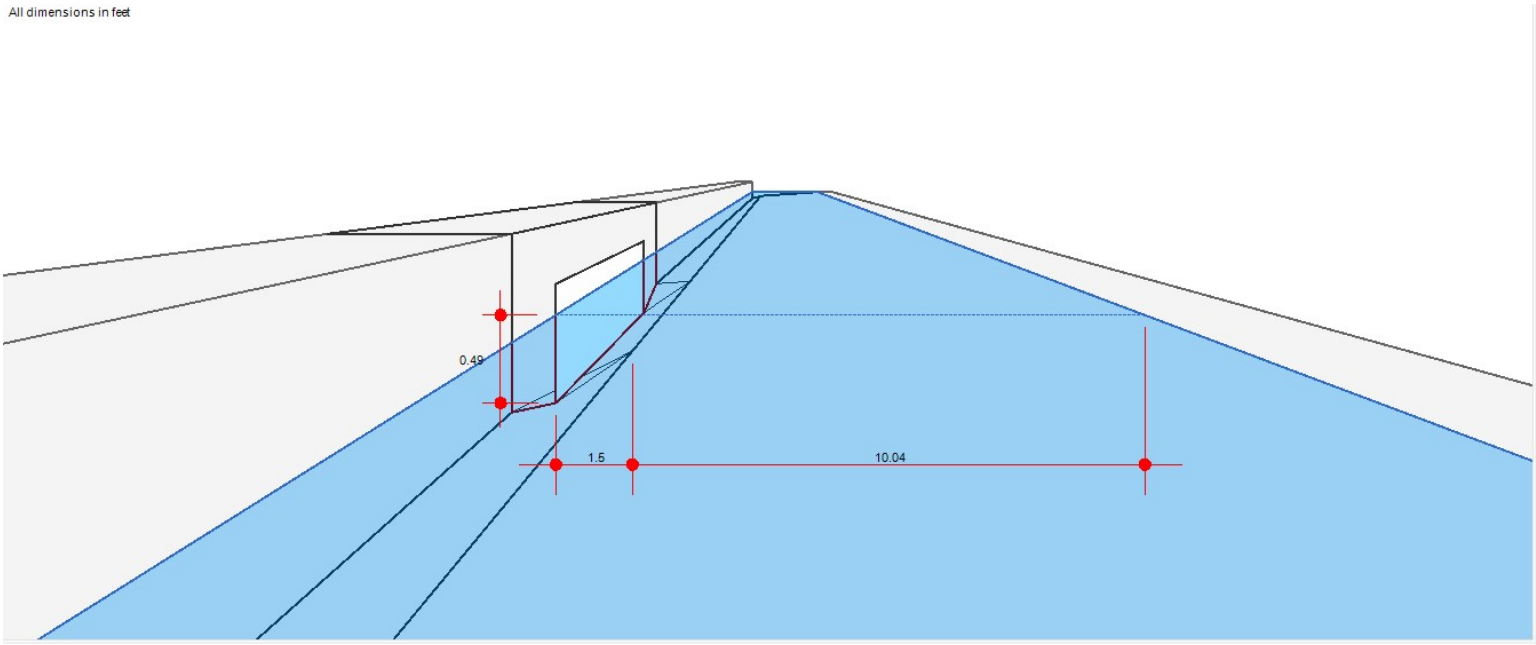
Slope, Sw (ft/ft)	= 0.083
Slope, Sx (ft/ft)	= 0.020
Local Depr (in)	= 2.00
Gutter Width (ft)	= 1.50
Gutter Slope (%)	= -0-
Gutter n-value	= -0-

Calculations

Compute by:	Known Q
Q (cfs)	= 1.71

Highlighted

Q Total (cfs)	= 1.71
Q Capt (cfs)	= 1.71
Q Bypass (cfs)	= -0-
Depth at Inlet (in)	= 5.90
Efficiency (%)	= 100
Gutter Spread (ft)	= 11.54
Gutter Vel (ft/s)	= -0-
Bypass Spread (ft)	= -0-
Bypass Depth (in)	= -0-



Inlet Report

CI-A2 (25 YEAR)

Curb Inlet

Location	= On grade
Curb Length (ft)	= 4.00
Throat Height (in)	= 4.00
Grate Area (sqft)	= -0-
Grate Width (ft)	= -0-
Grate Length (ft)	= -0-

Gutter

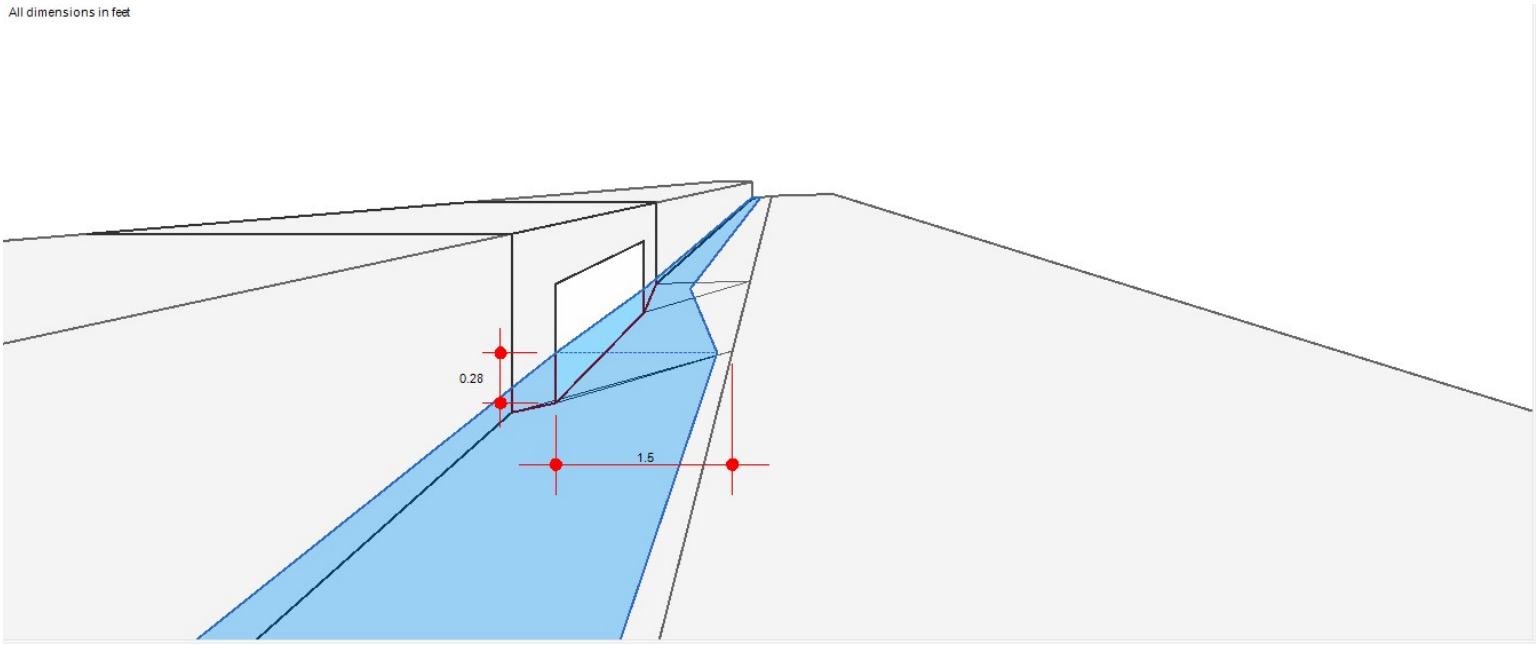
Slope, Sw (ft/ft)	= 0.083
Slope, Sx (ft/ft)	= 0.020
Local Depr (in)	= 2.00
Gutter Width (ft)	= 1.50
Gutter Slope (%)	= 8.40
Gutter n-value	= 0.016

Calculations

Compute by:	Known Q
Q (cfs)	= 0.37

Highlighted

Q Total (cfs)	= 0.37
Q Capt (cfs)	= 0.32
Q Bypass (cfs)	= 0.05
Depth at Inlet (in)	= 3.36
Efficiency (%)	= 86
Gutter Spread (ft)	= 1.37
Gutter Vel (ft/s)	= 4.75
Bypass Spread (ft)	= 0.65
Bypass Depth (in)	= 0.65



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Wednesday, Jul 23 2025

Pipe A1 (25 YEAR)

Circular

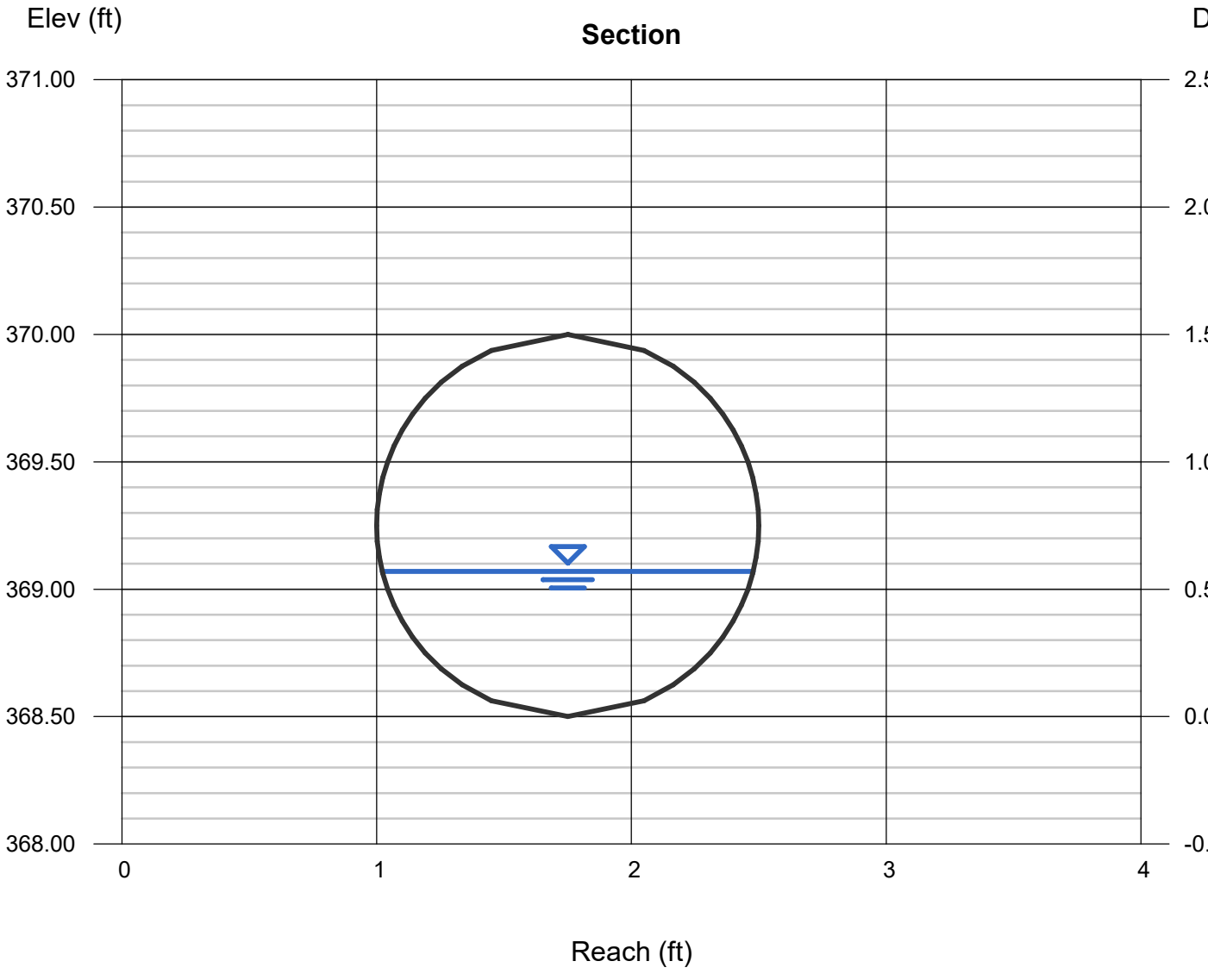
Diameter (ft)	= 1.50
Invert Elev (ft)	= 368.50
Slope (%)	= 0.26
N-Value	= 0.012

Calculations

Compute by:	Known Q
Known Q (cfs)	= 1.73

Highlighted

Depth (ft)	= 0.57
Q (cfs)	= 1.730
Area (sqft)	= 0.62
Velocity (ft/s)	= 2.80
Wetted Perim (ft)	= 1.99
Crit Depth, Yc (ft)	= 0.50
Top Width (ft)	= 1.46
EGL (ft)	= 0.69



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Wednesday, Jul 23 2025

Pipe A2 (25 YEAR)

Circular

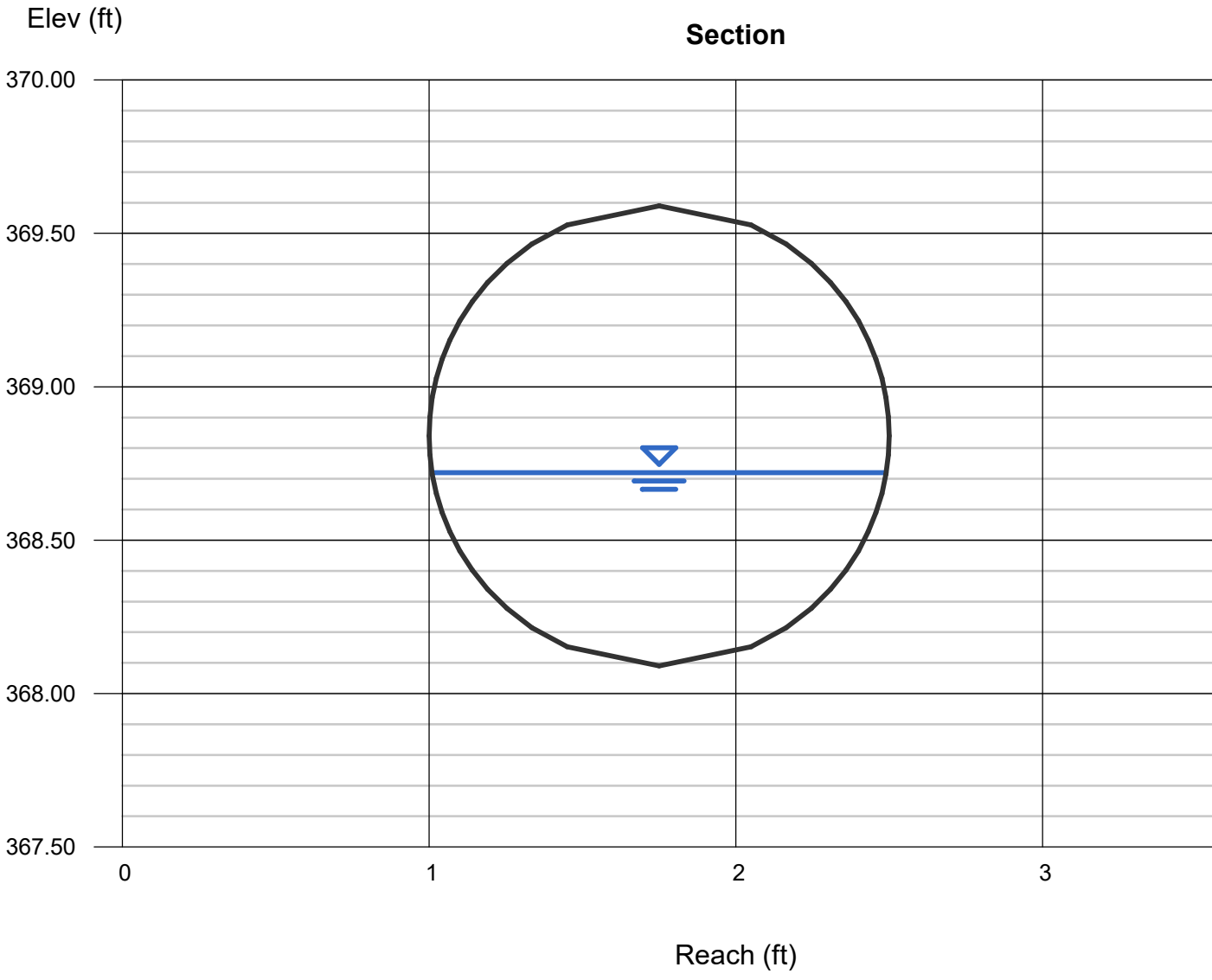
Diameter (ft)	= 1.50
Invert Elev (ft)	= 368.09
Slope (%)	= 0.26
N-Value	= 0.012

Calculations

Compute by:	Known Q
Known Q (cfs)	= 2.11

Highlighted

Depth (ft)	= 0.63
Q (cfs)	= 2.110
Area (sqft)	= 0.71
Velocity (ft/s)	= 2.97
Wetted Perim (ft)	= 2.12
Crit Depth, Yc (ft)	= 0.55
Top Width (ft)	= 1.48
EGL (ft)	= 0.77



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Wednesday, Jul 23 2025

Existing Pipe C1 (25 YEAR)

Circular

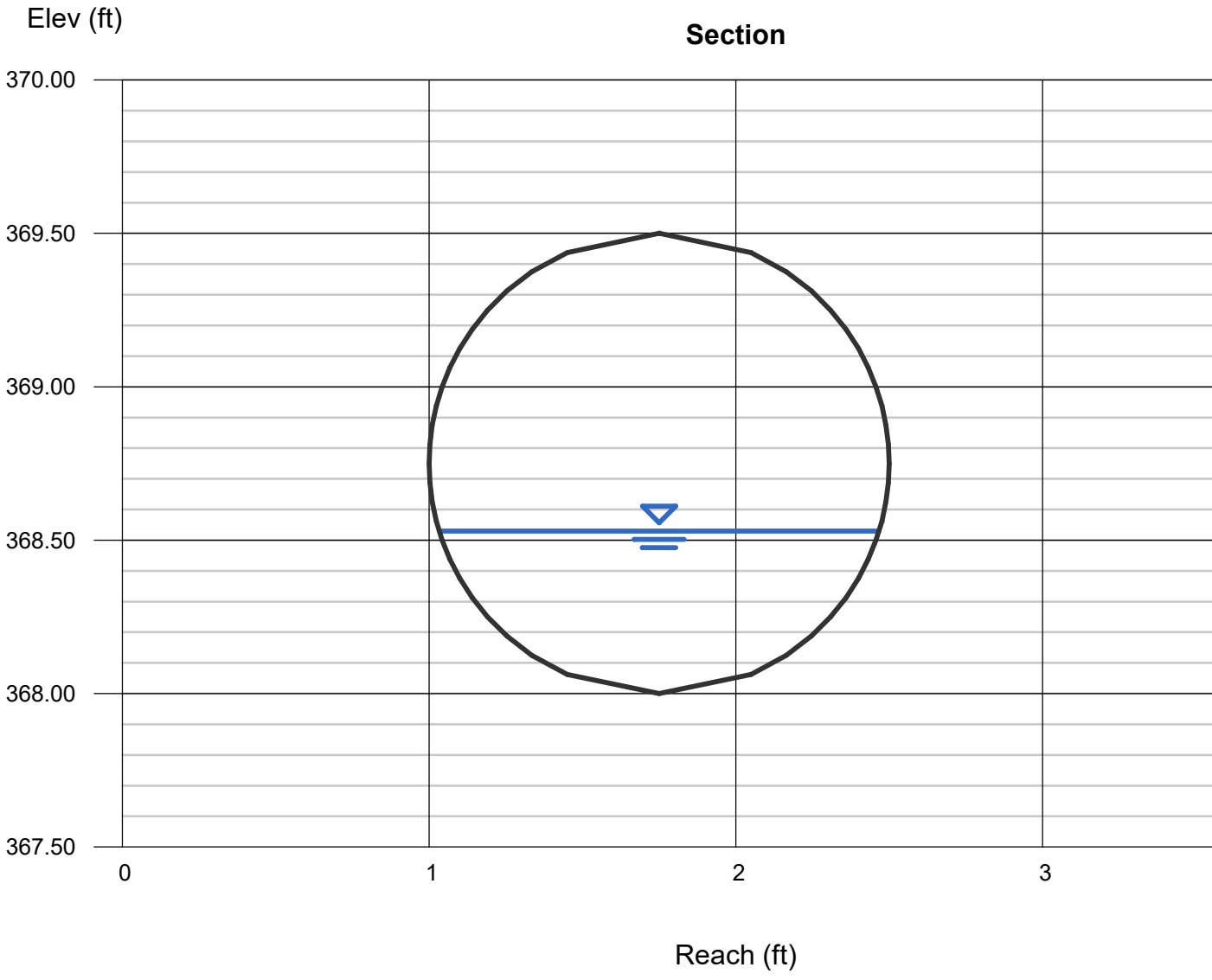
Diameter (ft)	= 1.50
Invert Elev (ft)	= 368.00
Slope (%)	= 1.14
N-Value	= 0.012

Calculations

Compute by:	Known Q
Known Q (cfs)	= 3.24

Highlighted

Depth (ft)	= 0.53
Q (cfs)	= 3.240
Area (sqft)	= 0.56
Velocity (ft/s)	= 5.78
Wetted Perim (ft)	= 1.91
Crit Depth, Yc (ft)	= 0.69
Top Width (ft)	= 1.43
EGL (ft)	= 1.05



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Wednesday, Jul 23 2025

Existing Pipe C2 (25 YEAR)

Circular

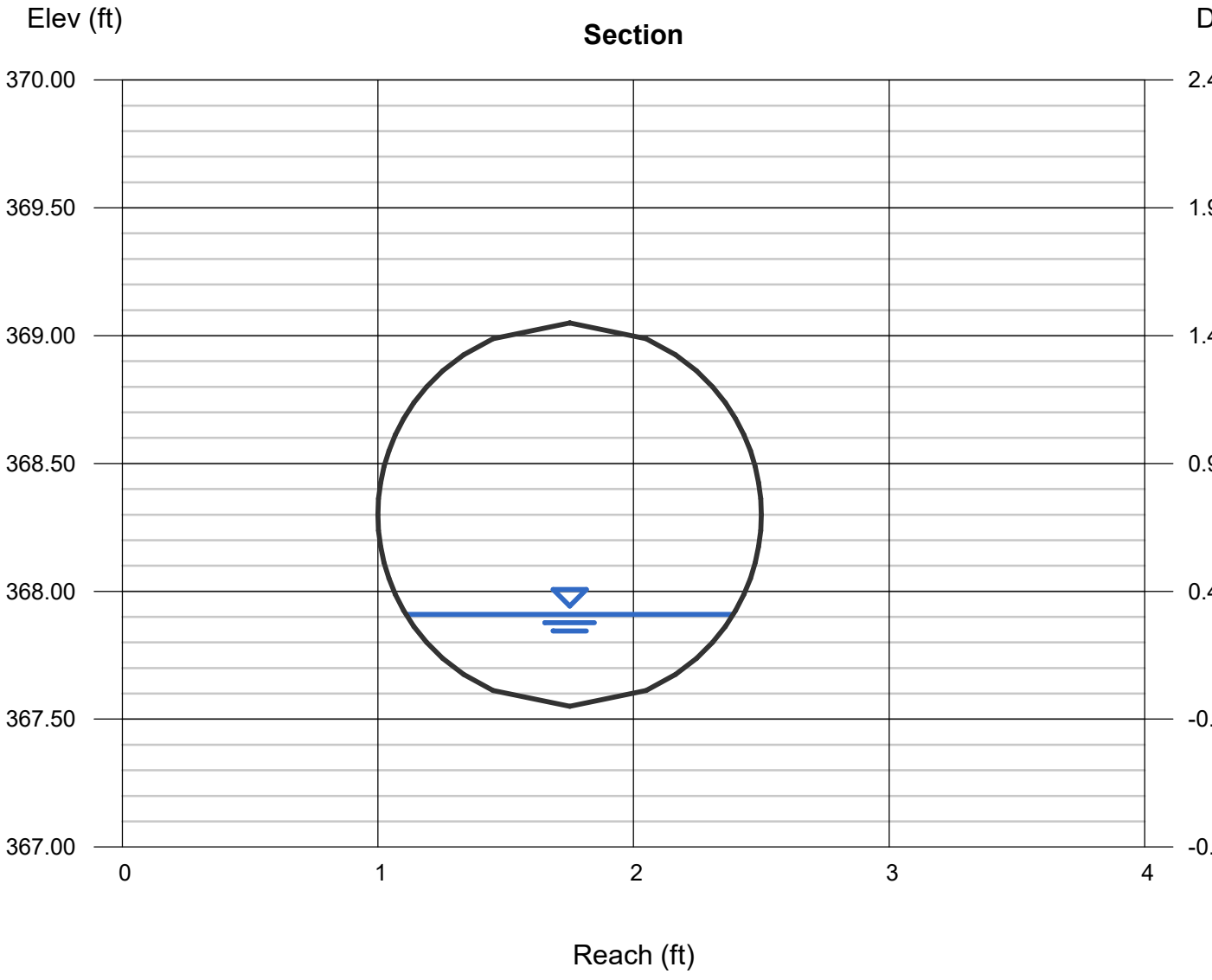
Diameter (ft)	= 1.50
Invert Elev (ft)	= 367.55
Slope (%)	= 6.06
N-Value	= 0.012

Calculations

Compute by:	Known Q
Known Q (cfs)	= 3.39

Highlighted

Depth (ft)	= 0.36
Q (cfs)	= 3.390
Area (sqft)	= 0.33
Velocity (ft/s)	= 10.37
Wetted Perim (ft)	= 1.54
Crit Depth, Yc (ft)	= 0.71
Top Width (ft)	= 1.28
EGL (ft)	= 2.03



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Wednesday, Jul 23 2025

Existing Pipe C3 (25 YEAR)

Circular

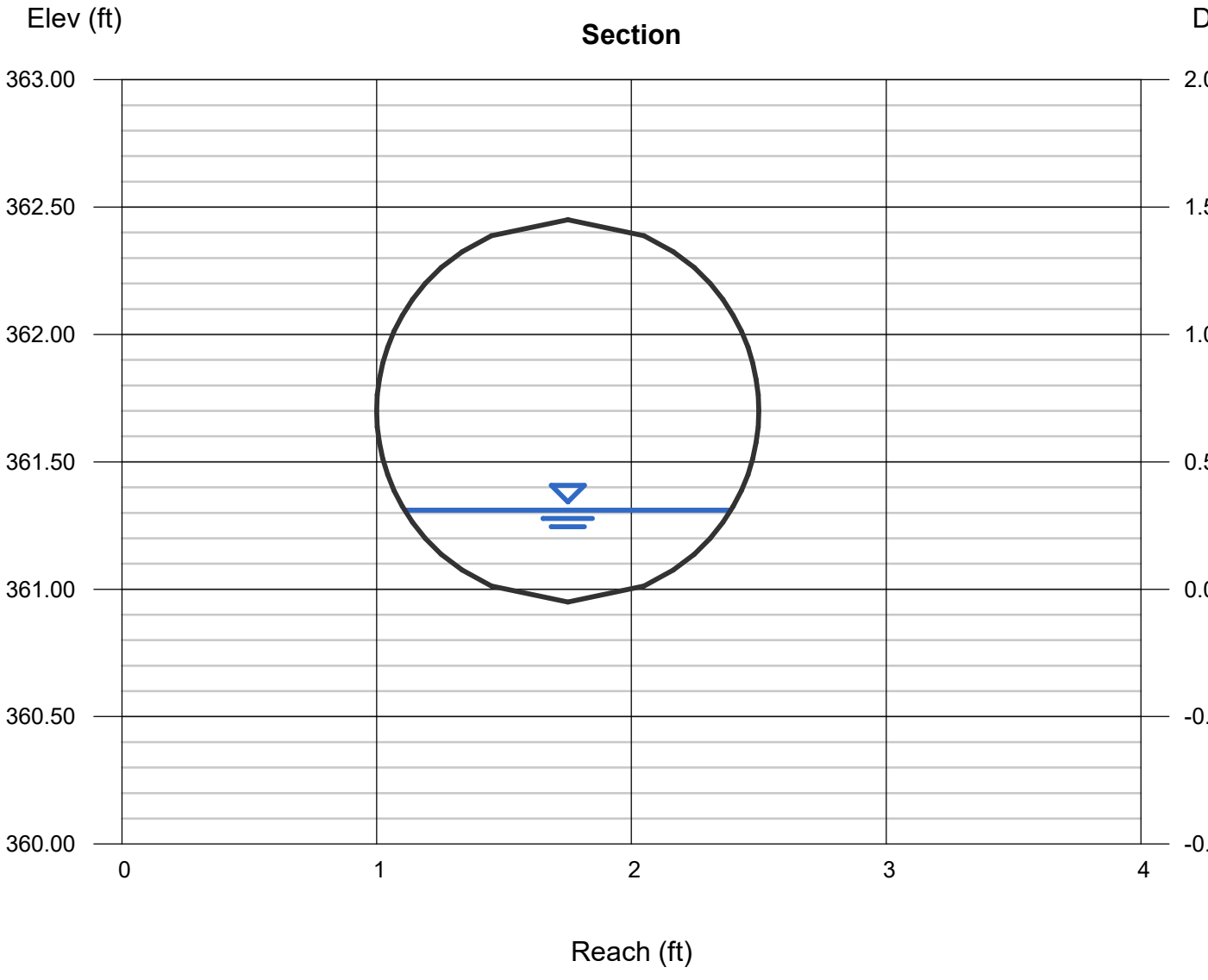
Diameter (ft)	= 1.50
Invert Elev (ft)	= 360.95
Slope (%)	= 5.91
N-Value	= 0.012

Calculations

Compute by:	Known Q
Known Q (cfs)	= 3.39

Highlighted

Depth (ft)	= 0.36
Q (cfs)	= 3.390
Area (sqft)	= 0.33
Velocity (ft/s)	= 10.37
Wetted Perim (ft)	= 1.54
Crit Depth, Yc (ft)	= 0.71
Top Width (ft)	= 1.28
EGL (ft)	= 2.03



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Wednesday, Jul 23 2025

Existing Pipe C4 (25 YEAR)

Circular

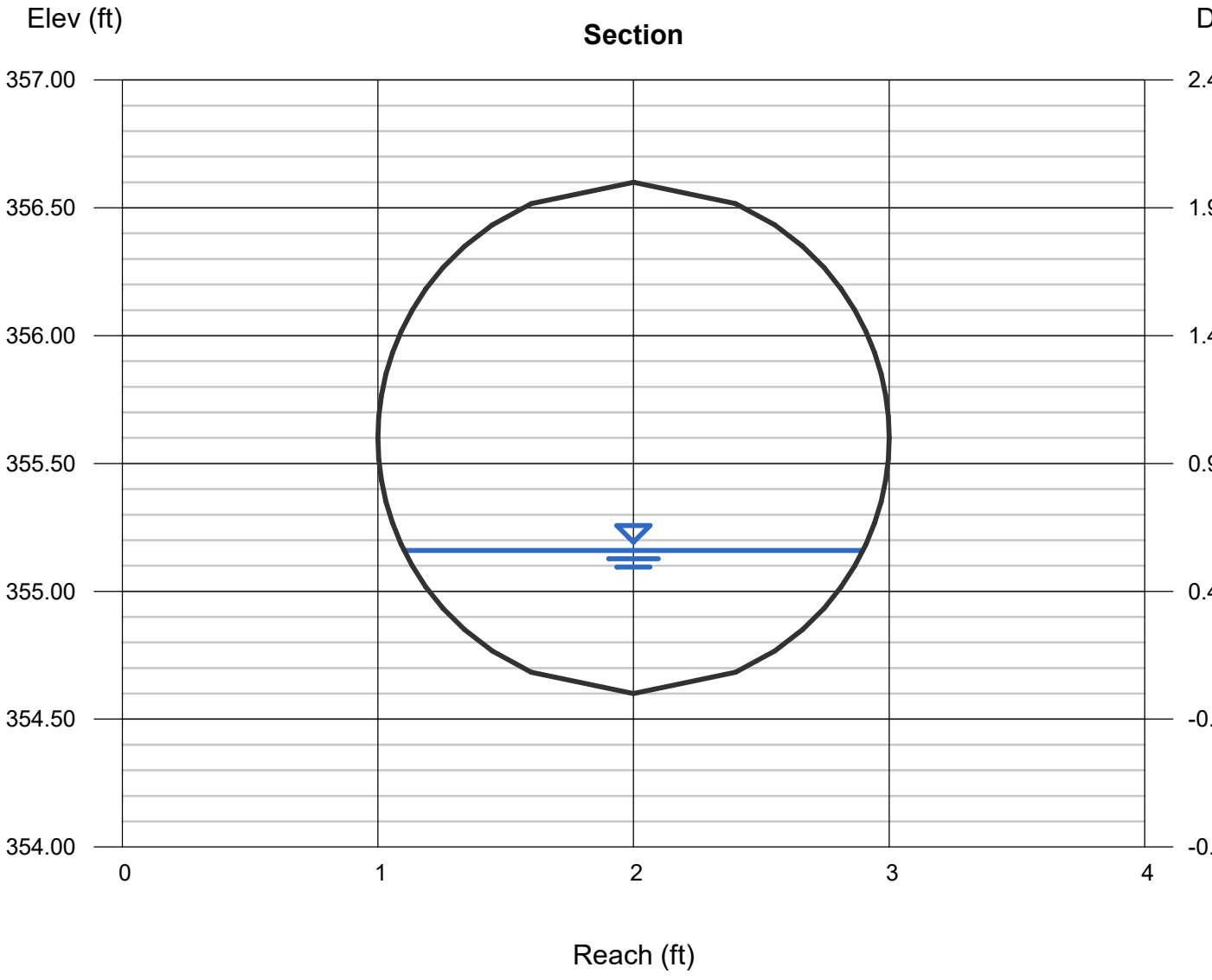
Diameter (ft)	= 2.00
Invert Elev (ft)	= 354.60
Slope (%)	= 2.91
N-Value	= 0.012

Calculations

Compute by:	Known Q
Known Q (cfs)	= 7.21

Highlighted

Depth (ft)	= 0.56
Q (cfs)	= 7.210
Area (sqft)	= 0.73
Velocity (ft/s)	= 9.91
Wetted Perim (ft)	= 2.24
Crit Depth, Yc (ft)	= 0.95
Top Width (ft)	= 1.80
EGL (ft)	= 2.09



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Wednesday, Jul 23 2025

Existing Pipe C5 (25 YEAR)

Circular

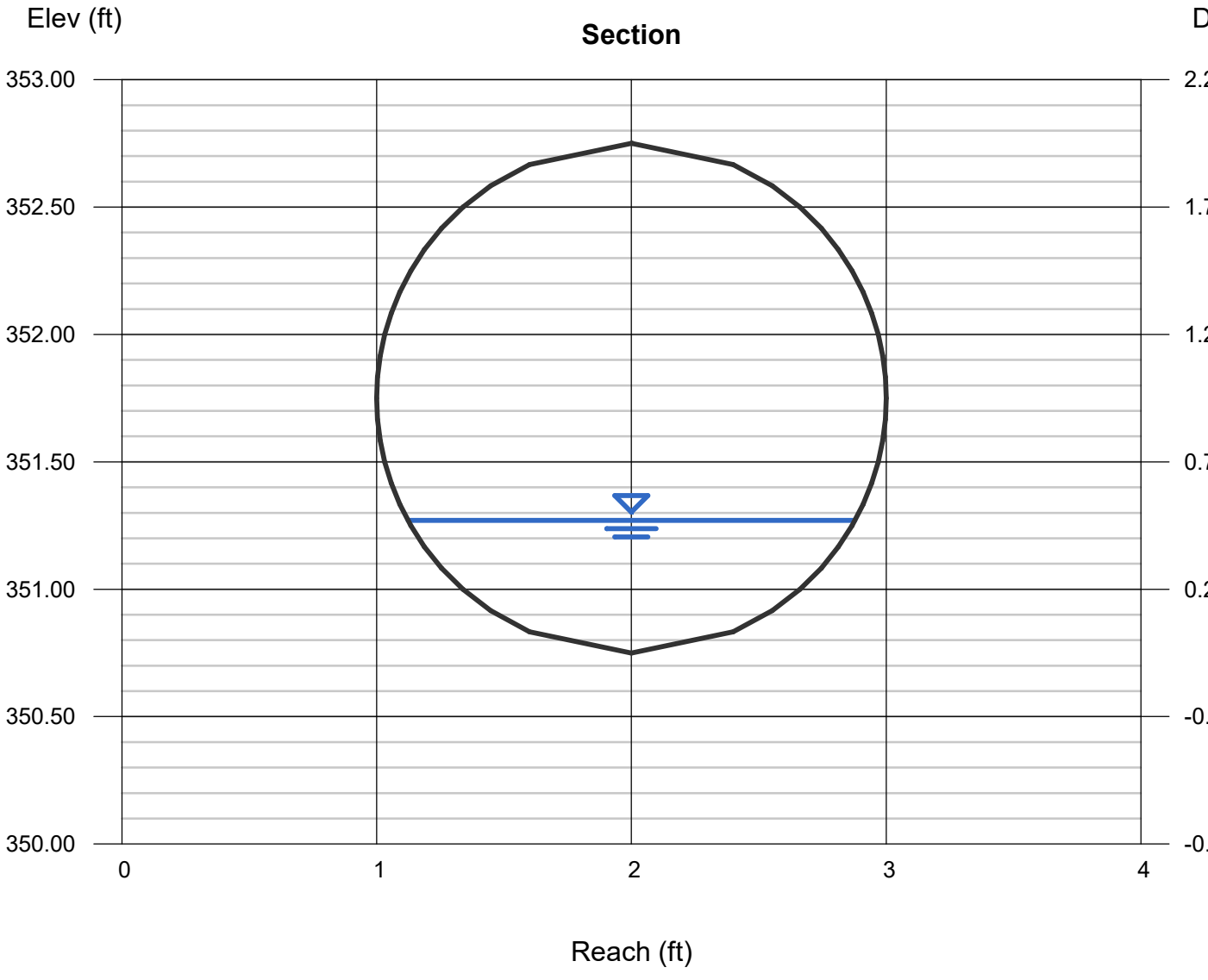
Diameter (ft)	= 2.00
Invert Elev (ft)	= 350.75
Slope (%)	= 7.25
N-Value	= 0.012

Calculations

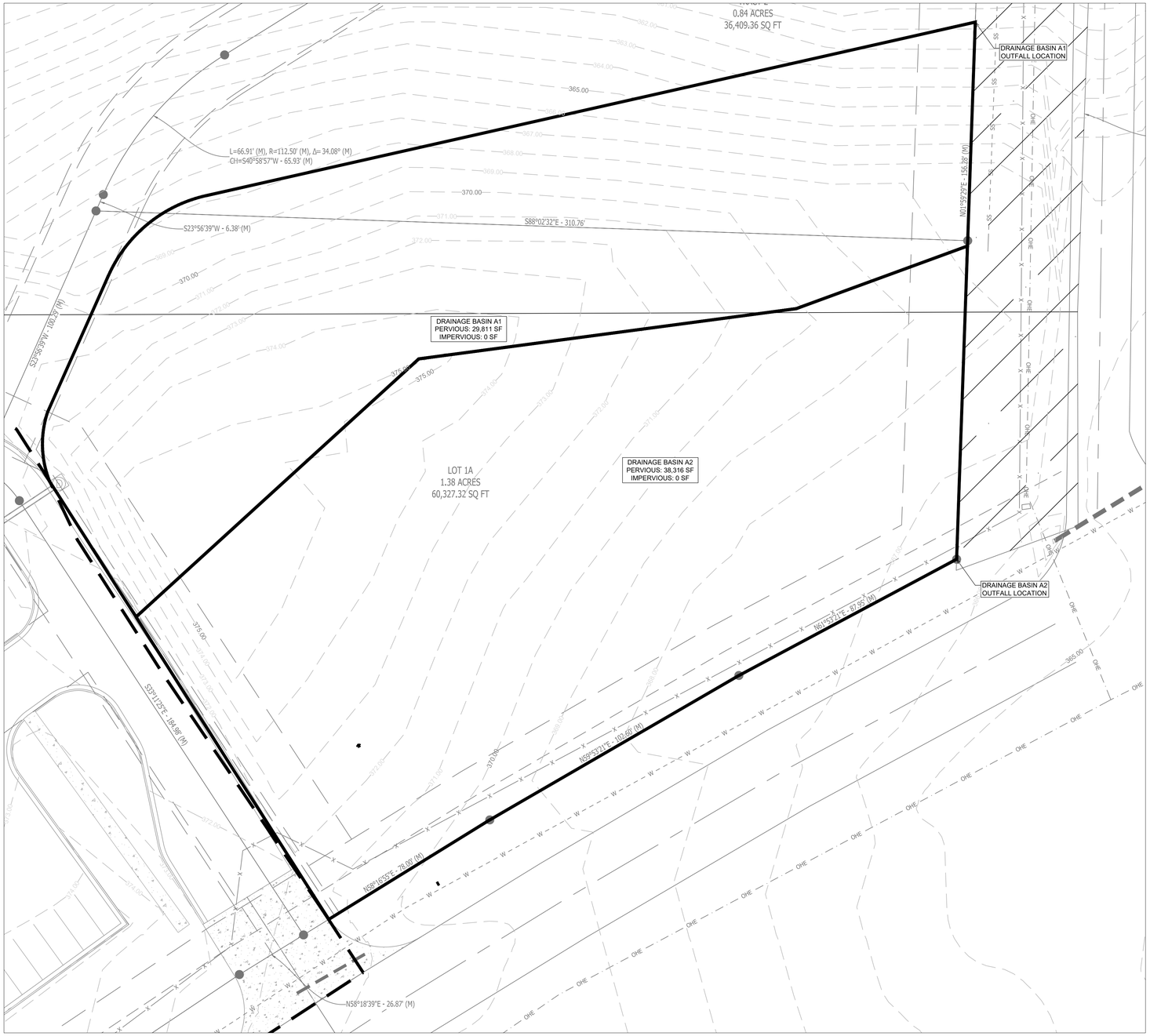
Compute by:	Known Q
Known Q (cfs)	= 9.59

Highlighted

Depth (ft)	= 0.52
Q (cfs)	= 9.590
Area (sqft)	= 0.66
Velocity (ft/s)	= 14.61
Wetted Perim (ft)	= 2.15
Crit Depth, Yc (ft)	= 1.11
Top Width (ft)	= 1.76
EGL (ft)	= 3.84



DRAINAGE BASIN MAPS



PRE-DEVELOPMENT DRAINAGE BASIN PLAN

SCALE 1" = 20'

PHILLIP LEWIS ENGINEERING, INC.
Structural + Civil Consultants
23620 Interstate 30 | Bryant, Arkansas
PH: 501-350-9840

REVISION:

NEW BEGINNINGS
HIGHWAY 5
BRYANT, ARKANSAS

CERTIFICATE OF AUTHORIZATION
PHILLIP LEWIS ENGINEERING, INC.
No. 566
ARKANSAS ENGINEER

STATE OF ARKANSAS
REGISTERED PROFESSIONAL ENGINEER
No. 9540
PHILLIP A. LEWIS

N

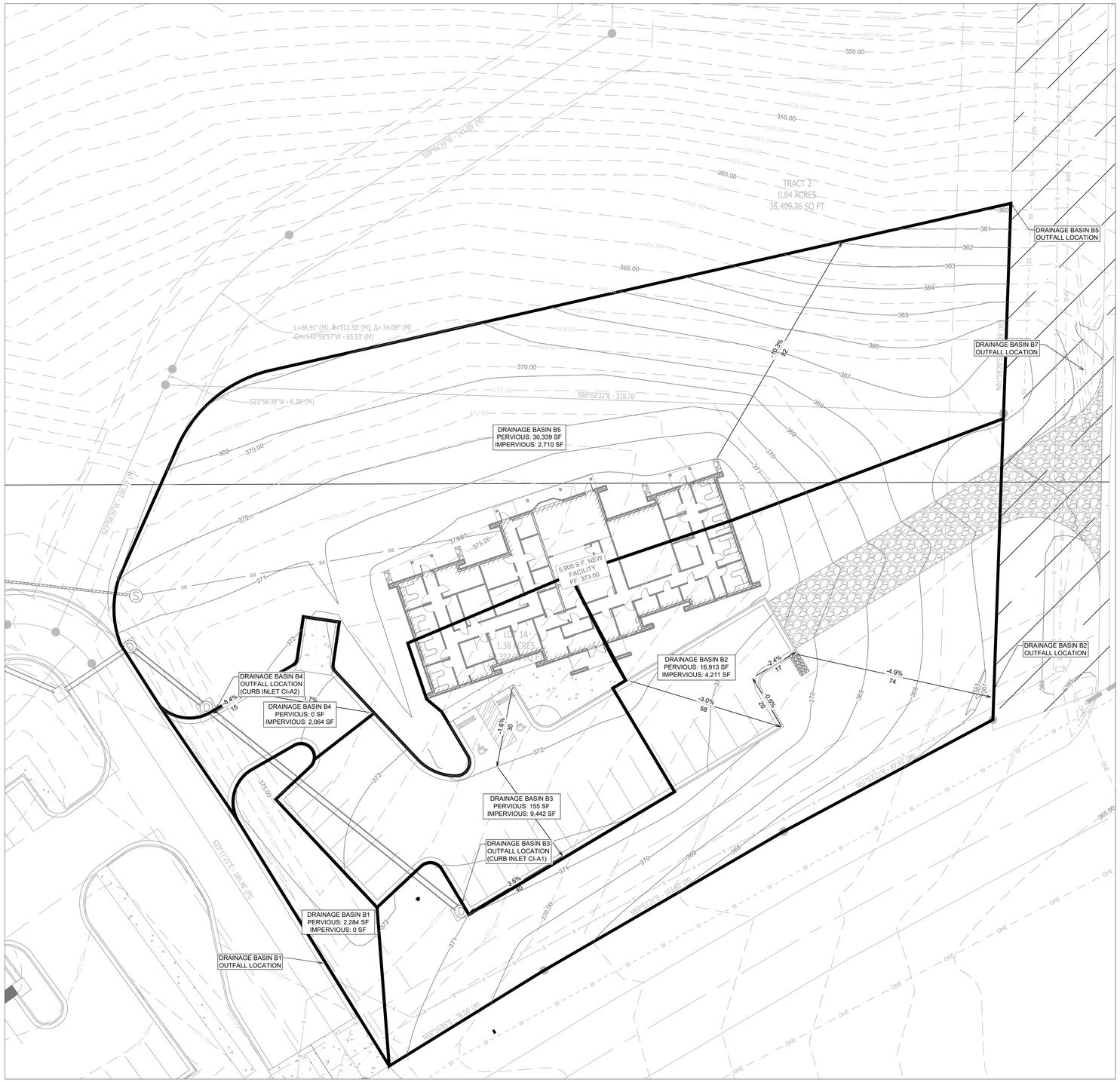
0 20'
1" = 20'-0"

PROJECT NUMBER:

SHEET ISSUE DATE:
08-06-2025

PAGE TITLE:
PRE-DEV DRAINAGE

SHEET NUMBER:
C1.10



POST-DEV DRAINAGE

SCALE 1" = 20'

N

0 10'

1" = 10'-0"

PHILLIP LEWIS ENGINEERING, INC.
Structural + Civil Consultants
23620 Interstate 30 | Bryant, Arkansas
PH: 501-350-9840

REVISION:

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No. 9540
PHILLIP LEWIS

PROJECT NUMBER:

SHEET ISSUE DATE:
08-06-2025

PAGE TITLE:
POST-DEV DRAINAGE

SHEET NUMBER:
C1.11

SOIL CLASSIFICATION MAPS

United States Department of Agriculture

NRCS

Natural Resources Conservation Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Saline County, Arkansas

August 30, 2024

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report
Soil Map

Map Scale: 1:2,340 if printed on A landscape (11" x 8.5") sheet.

N

Meters
0 30 60 120 180

Feet
0 100 200 400 600

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 15N WGS84

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Unit Polygons
- Soil Map Unit Lines
- Soil Map Unit Points

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other
- Special Line Features

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background
- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Saline County, Arkansas
Survey Area Data: Version 20, Sep 12, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 1, 2022—May 29, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
16	Ouachita silt loam, 0 to 1 percent slopes, frequently flooded	3.6	25.0%
22	Savannah fine sandy loam, 3 to 8 percent slopes	2.5	17.8%
27	Smithdale loamy sand, 8 to 12 percent slopes	4.9	34.4%
29	Tiak silt loam, 3 to 8 percent slopes	3.3	22.9%
Totals for Area of Interest		**14.2**	**100.0%**

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Saline County, Arkansas

16—Ouachita silt loam, 0 to 1 percent slopes, frequently flooded

Map Unit Setting

National map unit symbol: 30g3t
Elevation: 120 to 250 feet
Mean annual precipitation: 48 to 64 inches
Mean annual air temperature: 52 to 75 degrees F
Frost-free period: 225 to 290 days
Farmland classification: Farmland of statewide importance

Map Unit Composition

Ouachita, frequently flooded, brief duration, and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Ouachita, Frequently Flooded, Brief Duration

Setting

Landform: Flood plains
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Loamy alluvium

Typical profile

A - 0 to 4 inches: silt loam
Bw - 4 to 42 inches: silt loam
2C - 42 to 80 inches: very fine sandy loam

Properties and qualities

Slope: 0 to 1 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.57 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: Frequent
Frequency of ponding: None
Maximum salinity: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)
Available water supply, 0 to 60 inches: High (about 10.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 5w
Hydrologic Soil Group: C
Ecological site: F133BY017TX - Loamy Bottomland
Hydric soil rating: No

Minor Components

Ouachita, frequently flooded, long duration

Percent of map unit: 5 percent
Landform: Flood plains

Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F133BY017TX - Loamy Bottomland
Hydric soil rating: Yes

Aquents, frequently flooded
Percent of map unit: 5 percent
Landform: Depressions
Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Convex
Ecological site: F133BY012TX - Wet Terrace
Hydric soil rating: Yes

Una, frequently flooded
Percent of map unit: 3 percent
Landform: Flood plains
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F133BY018TX - Clayey Bottomland
Hydric soil rating: Yes

Guyton, frequently flooded
Percent of map unit: 2 percent
Landform: Flood plains
Landform position (three-dimensional): Talf
Down-slope shape: Linear
Across-slope shape: Linear
Ecological site: F133BY017TX - Loamy Bottomland
Hydric soil rating: Yes

22—Savannah fine sandy loam, 3 to 8 percent slopes

Map Unit Setting
National map unit symbol: 2tzrt
Elevation: 50 to 250 feet
Mean annual precipitation: 38 to 61 inches
Mean annual air temperature: 52 to 73 degrees F
Frost-free period: 220 to 260 days
Farmland classification: Farmland of statewide importance

Map Unit Composition
Savannah and similar soils: 95 percent
Minor components: 5 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Savannah

Setting

Landform: Interfluves
Landform position (three-dimensional): Riser
Down-slope shape: Convex
Across-slope shape: Linear
Parent material: Loamy marine deposits

Typical profile

Ap - 0 to 9 inches: fine sandy loam
Bt - 9 to 24 inches: loam
Btx - 24 to 59 inches: loam
BC - 59 to 72 inches: sandy loam

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: 16 to 32 inches to fragipan
Drainage class: Moderately well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.57 in/hr)
Depth to water table: About 16 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 3.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2e
Hydrologic Soil Group: C
Ecological site: F133BY005TX - Loamy Upland
Hydric soil rating: No

Minor Components

Amy

Percent of map unit: 5 percent
Landform: Stream terraces
Landform position (three-dimensional): Tread
Down-slope shape: Concave
Across-slope shape: Linear
Ecological site: F133BY017TX - Loamy Bottomland
Hydric soil rating: Yes

27—Smithdale loamy sand, 8 to 12 percent slopes

Map Unit Setting

National map unit symbol: m06n
Elevation: 70 to 620 feet

Mean annual precipitation: 44 to 61 inches
Mean annual air temperature: 49 to 74 degrees F
Frost-free period: 185 to 230 days
Farmland classification: Not prime farmland

Map Unit Composition
Smithdale and similar soils: 100 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Smithdale

Setting
Landform: Interfluves
Down-slope shape: Convex
Across-slope shape: Linear
Parent material: Loamy marine deposits

Typical profile
A - 0 to 6 inches: loamy sand
BA - 6 to 15 inches: fine sandy loam
Bt1 - 15 to 26 inches: sandy clay loam
Bt2 - 26 to 102 inches: fine sandy loam
BC - 102 to 123 inches: loamy fine sand

Properties and qualities
Slope: 8 to 12 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 1.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Moderate (about 8.0 inches)

Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: B
Ecological site: F133BY005TX - Loamy Upland
Hydric soil rating: No

29—Tiak silt loam, 3 to 8 percent slopes

Map Unit Setting
National map unit symbol: m06q
Elevation: 70 to 570 feet
Mean annual precipitation: 44 to 61 inches
Mean annual air temperature: 49 to 74 degrees F
Frost-free period: 185 to 230 days
Farmland classification: Not prime farmland

Map Unit Composition

Tiak and similar soils: 100 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Tiak

Setting

Landform: Interfluves
Down-slope shape: Convex
Across-slope shape: Linear
Parent material: Loamy and clayey marine deposits

Typical profile

A - 0 to 7 inches: silt loam
E - 7 to 9 inches: loam
Bt1 - 9 to 32 inches: clay
Bt2 - 32 to 72 inches: clay

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Moderately well drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 12 to 24 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: High (about 9.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: C/D
Ecological site: F133BY002TX - Seasonally Wet Upland
Hydric soil rating: No

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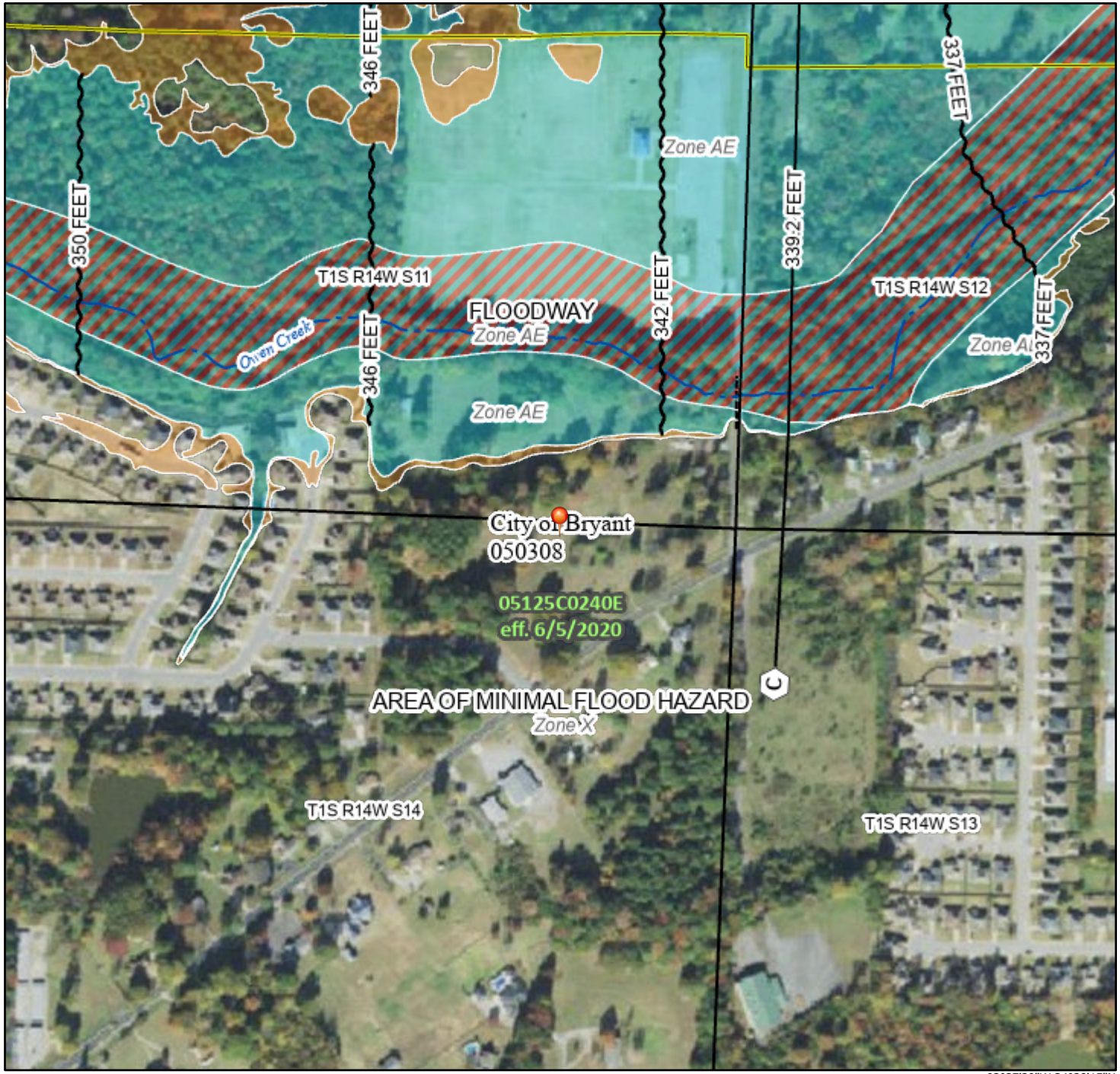
FEMA FLOOD INSURANCE RATE MAP

National Flood Hazard Layer FIRMette

FEMA

92°28'7"W 34°38'45"N



92°27'30"W 34°38'15"N

0 250 500 1,000 1,500 2,000 Feet

1:6,000

Basemap Imagery Source: USGS National Map 2023

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
- Area with Reduced Flood Risk due to Levee. See Notes. *Zone X*
- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard *Zone X*
- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- 20.2 Cross Sections with 1% Annual Chance
- 17.5 Water Surface Elevation
- Coastal Transect
- Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 10/9/2024 at 5:29 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

PHILLIP LEWIS ENGINEERING
Structural + Civil Consultants
23620 Interstate 30 | Bryant, AR
PH: 501-350-9840

New Beginnings – Soil Loss Calculations

July 24, 2025

Soil Loss Calculations without Controls

E (Soil Loss) = (R)(K)(LS)(C)(P) = (300)(0.43)(1.37)(0.45)(1) = 79.53 Tons/Acre/Year

R = 300 (Rainfall Factor)

K = 0.43 (Soil Type (Ap))

LS = 1.37 (10% slope for 100ft)

C = 0.45 (No Canopy)

P = 1 (No conservation Practice)

Soil Loss Calculations with Controls

E (Soil Loss) = (R)(K)(LS)(C)(P) = (300)(0.43)(1.37)(0.12)(0.6) = 12.72 Tons/Acre/Year

R = 300 (Rainfall Factor)

K = 0.43 (Soil Type (Ap))

LS = 1.37 (10% slope for 100ft)

C = 0.12 (2 tons/acre Loose Straw or Hay)

P = 0.6 (Silt Fence)

Article 2. Anticipated revenues shall be established at $16,888.89 in fund 3506-7010 State Grants.
Article 3. It is deemed necessary for the smooth operation of Saline County Government that this ordinance be approved.
DATE: JULY 21, 2025. APPROVED: MATT BRUMLEY, SALINE COUNTY JUDGE
This publication paid for by the Saline County's Judge Financial Mgt. Dept. and cost $98.10.

Legal Notices

EMERGENCY ORDINANCE NO. 2025 – 11
BE IT ORDAINED BY THE QUORUM COURT OF SALINE COUNTY, STATE OF ARKANSAS, AN ORDINANCE TO BE ENTITLED: AN ORDINANCE TO ESTABLISH A SUB-FUND OF COUNTY GENERAL TO BE CALLED THE SUT GENERAL FUND; AND TO DECLARE AN EMERGENCY.
Article 1. Affirmation. It comes before this Court that there is a need to establish a fund on the books of the county as a sub fund of the County General Fund No. 1000 to track the revenues, expenditures and/or appropriated transfers of additional SUT funds as received by the county from the State of Arkansas. This Court recognizes and affirms the need for such a fund to property account for and control all such revenues received and expenditures made in compliance with all applicable laws.
Article 2. Establishment of Fund. There is hereby created on the books of the Saline County Treasurer and the books of the Saline County Clerk or Comptroller a fund to be known as the SUT General Fund with a fund number of 1801 as assigned by Arkansas Legislative Audit. The revenue code for revenues received in such fund will be assigned in accordance with the County Financial Management System Manual revised by Legislative Audit in October 2022.
Article 3. Operation of Fund. The SUT General Fund is subject to all the normal county budgeting, appropriation and expenditure regulations of Arkansas Code Annotated, Title 14 and the County Financial Management System. As a sub fund of the County General Fund any balance in the fund is considered accruable to County General and is part of the general fund balance in aggregate as defined in A.C.A. 14-15-805(3). Any revenue received is unrestricted county revenue and may be expended for any legal county expense.
Article 4. Emergency Clause. It is found by this Court that the revenue related to such fund makes it necessary to establish the SUT General Fund, a sub fund of County General in order to be able to properly track the revenue, appropriated expenditures and/or appropriated transfers. Therefore, an emergency is declared to exist and this ordinance shall be in full force and effect from the date of passage and approval.
Dated: JULY 21, 2025 APPROVED: MATT BRUMLEY, SALINE COUNTY JUDGE
This publication paid for by the Saline County Judge's Financial Mgt. Dept. and cost $148.20.

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Business Op-

Going out of Business Sale! 3 Buildings full of resale merchandise. NO Clothes. Must buy out right! Call. 501-672-6444

CONSIDERED AT THE PLANNING COMMISSION MEETING ON SEPTEMBER 2, 2025. FOR INFORMATION, CALL (501) 860-6893.

Looking for love in all the wrong places???? Check out the Freebie section in today's classifieds. You will find unconditional love there FREE! Furry & Free!!

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Legal Notices

NOTICE OF PUBLIC HEARING
A public hearing will be held by City of Bryant, AR Planning Commission on Monday, Sept. 8, 2025 at 6:00 P.M. at the Bryant City Office Complex, 210 Southwest 33rd Street, for the purpose of public comment on the application for David Harris to obtain a Conditional Use for the purpose of adding a side shed to existing building with a zone at the site of 20 Tanglewood Dr. A legal description of this property can be obtained by contacting the Bryant Planning and Development Department at 501-943-0488.

Legal Notices

LEGAL NOTICE?
REQUEST FOR QUALIFICATIONS (RFQ)?
Saline County- Airport Engineer
Saline County is requesting Statements of Qualifications from qualified firms to provide professional engineering services for The Saline County Regional Municipal Airport, located in Saline County, AR.
Interested parties may obtain the full RFQ documents and instructions by visiting www.salinecounty.org.
Submittals must be received no later than Au-

NO. 63PR-25-359-IV IN THE CIRCUIT COURT OF SALINE COUNTY ARKANSAS, PROBATE DIVISION.

IN THE MATTER OF THE ESTATE OF ROBERT EARL BALLENTINE, DECEASED.
Last known address of decedent: 708 Sheffield Drive, Bryant, AR 72022. Date of Death: June 3, 2025.

A Petition for Appointment of Administrator was admitted to probate on July 9, 2025, and the undersigned, Terry Yazz, has been appointed Administrator thereunder. A contest of the probate of the estate can be effected only by filing a petition within the time provided by law.

All persons having claims against the estate must exhibit them, duly verified, to the undersigned within six (6) months from the date of the first publication of this notice, or they shall be forever barred and precluded from any benefit in the estate.

This notice first published the 22nd day of July 2025.

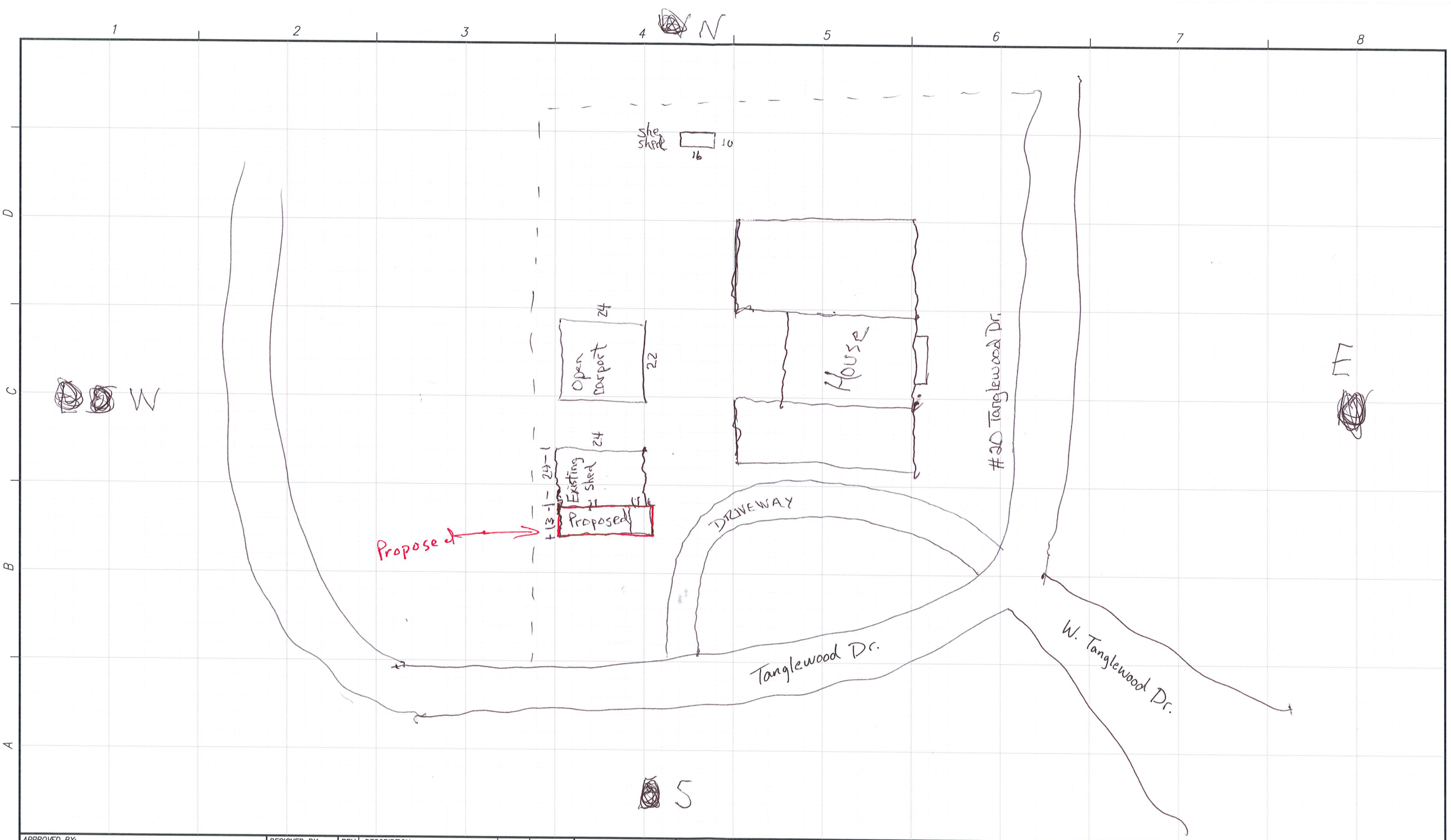
Terry Yazza, Administrator

Colin C. Heaton
Heaton & Harris LLP
Attorneys at Law
P.O. Box 111
Hot Springs, AR 71902-0111

Legal Notices

EMERGENCY ORDINANCE NO. 2025 – 10
BE IT ORDAINED BY THE QUORUM COURT OF SALINE COUNTY, STATE OF ARKANSAS, AN ORDINANCE TO BE ENTITLED: AN ORDINANCE TO ESTABLISH A SPECIAL REVENUE FUND TO BE CALLED THE PASSPORT ACCEPTANCE FACILITY FUND; AND TO DECLARE AN EMERGENCY.
Article 1. Affirmation. It comes before this Court that there is a need to establish a special revenue fund on the books of the county to track the revenues, expenditures and/or appropriated transfers of passport fees collected by a County office that has been authorized as a passport acceptance facility. This Court recognizes and affirms the need for such a fund to properly account for and control all such revenues received and expenditures made in compliance with all applicable laws and guidance by from the State of Arkansas.
Article 2. Establishment of Fund. There is hereby created on the books of the Saline County Treasurer and the books of the Saline County Comptroller and Financial Management Department a special revenue fund to be known as the Passport Acceptance Facility Fund with a fund number of 3049 as assigned by the Saline County Quorum Court. The revenue code for the collections from passport fees received will continue to be "Passport Application Fees – Revenue Code 7617."
Article 3. Operation of Fund. The Passport Acceptance Facility Fund is

Business & Service Directory



APPROVED BY:

TITLE	SIGNATURE	DATE
TITLE	SIGNATURE	DATE
TITLE	SIGNATURE	DATE

DESIGNED BY:

DRAWN BY:

CHECKED BY:

REV	DESCRIPTION	BY	DATE

NORTH:

INDICATE NORTH ABOVE

SCALE:

0' ' '

GRAPHIC SCALE

INDICATE SCALE OF GRID ABOVE IN 1' PER FEET EXAMPLE 1" = 50'

NAME: David Harris

ADDRESS: 20 Tanglewood Dr.

STREET:

PHONE:

BRYANT, ARKANSAS
72022

CITY OF BRYANT Est. 1892 ARKANSAS

STRUCTURE TYPE:

SQUARE FT:

LOT SIZE:

SUBMITTAL DATE:

WORK ORDER NO.:

SHEET NO.: OF

XXX####XXX.DWG

City of Bryant, Arkansas
Planning and Development
210 SW 3rd Street Bryant, AR 72022
501-943-0943

Conditional Use Permit Application

Applicants are advised to read the Conditional Use Permit section of Bryant Zoning Code prior to completing and signing this form. The Zoning Code is available at www.cityofbryant.com under the Planning and Community Development tab.

Date: 7-28-25

Applicant or Designee:

Name David Harris
Address 20 Tanglewood Dr

Phone 501-860-8907
Email: drdavidharris@hotmail.com

Project Location:

Property Address 20 Tanglewood Dr

Parcel Number 840-09528-000
Zoning Classification R-F

Property Owner (If different from Applicant):

Name
Phone
Address
Email Address

Additional Information:

Subdivision Lot and Block Number or Legal Description (Attach Legal Description to Application)
Lot 20 - Tanglewood Acres. Lot is .88ac in size

Current Use of Property Primary Residence

Description of Conditional Use Request / Proposed Use of Property (Attach any necessary drawings or images)
side shed to existing building → 13'x24'

Application Checklist

Requirements for Submission

- ☐ Letter stating request of Conditional Use and reasoning for request
- ☐ Completed Conditional Use Permit Application
- ☐ Submit Conditional Use Permit Application Fee ($125)
- ☐ Submit Copy of completed Public Notice [Attachment 1]
- ☐ Submit one (1) copy of the Development Plan (Site Plan) showing:
  - Location, size, and use of buildings/signs/land or improvements
  - Location, size, and arrangement of driveways and parking. Ingress/Egress
  - Existing topography and proposed grading
  - Proposed and existing lighting
  - Proposed landscaping and screening
  - Use of adjacent properties
  - Scale, North Arrow, Vicinity Map
  - Additional information that may be requested by the administrative official due to unique conditions of the site.
- ☐ Public Notice Requirements: **NOTE: Failure to provide notice in the following manners shall require delay of the public hearing until notice has been properly made.**
  - Publication: Public Notice shall be published at least one (1) time **fifteen (15) days prior to the public hearing** at which the variance will be heard. A copy of the public notice is provided on last page of application. [Attachment 1] Once published, the proof of publication must be provided to the Planning and Development office.
  - Posting of Property: The city shall provide a sign to post on the property involved for the **fifteen (15) consecutive days leading up to the public hearing.** One (1) sign is required for every two hundred (200) feet of street frontage.

Once the application is received, the material will be reviewed to make sure all the required information is provided. The applicant will be notified if additional information is required. The application will then go before the Development and Review Committee (DRC) for a recommendation to the Planning Commission. A public hearing will be held at this meeting for comments on the Conditional Use. After the public hearing, the Planning Commission will make a decision on the use.

READ CAREFULLY BEFORE SIGNING

I _____, do hereby certify that all information contained within this application is true and correct. I further certify that the owner of the property authorizes this proposed application. I understand that I must comply with all City Codes and that it is my responsibility to obtain all necessary permits required.

David Harris
20 Tanglewood Dr
Bryant, AR 72022

City of Bryant
Planning and Development
210 SW 3rd Street
Bryant, AR 72022

July 30, 2025

Greetings:

The purpose of this request for a conditional use permit is to be allowed to add a side shed to an existing building for storage and personal use.

Thank you for your consideration.