

Bryant Development and Review Committee Meeting

Boswell Municipal Complex - City Hall Conference Room 210 SW 3rd Street

Date: October 30, 2025 - Time: 9:00 AM

Call to Order

Old Business

New Business

1. Shoppes at Dogwood Springs - Phase 1 - 4707 Bryant Parkway

Richardson Engineering - Requesting Site Plan Approval

- · 0998-PLN-01.pdf
- 0998-DRN-01.pdf

2. Midtown Bryant - 12 Paisley Park and Block 2 - Minor Modifications from Midtown Code

HD Homes - Requesting Minor Modifications from Midtown Code for sidewalk location at 12 Paisley Park and change to frontage type at Block 2 for A Street

- · 0999-PLN-2b.pdf
- · 0999-PLN-01b.pdf
- <u>0999-PLN-01a.jpg</u>

3. 4014 Robinwood Cir - Conditional Use Permit - Accessory Structure

David Moulin - Requesting Recommendation for Approval of CUP for Additional Squarefootage for Accessory Structure

- 1000-APP-01.pdf
- 4. Gassy's Phillips 66 6101 HWY 5 Sign Permit

Arkansas Sign and Neon - Requesting Sign Permit Approval

• 94247-SGNAPP-02.pdf

Staff Approved

5. Therapeds - 2208 N Reynolds Rd - Sign Permit

Ace Sign Company - Requesting Sign Permit Approval - STAFF APPROVED

94275-SGNAPP-01.pdf

Permit Report

Adjournments

DETAILED PLANS:

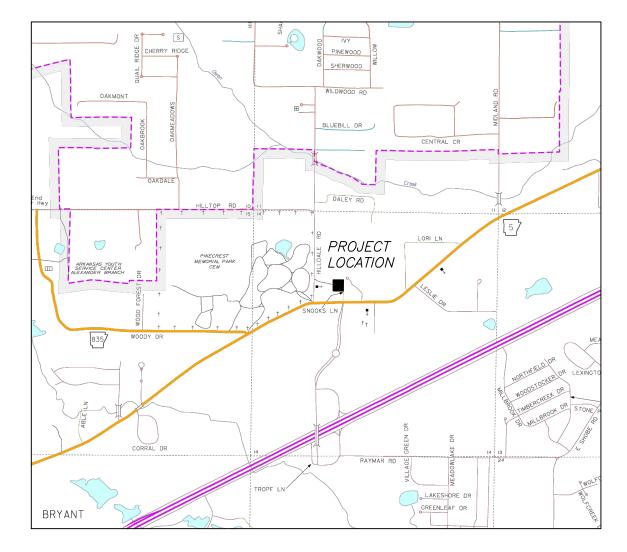
SHOPPES AT DOGWOOD SPRINGS PHASE 1

BRYANT PARKWAY
BRYANT, ARKANSAS

LEGAL DESCRIPTION

LOT 1 AND 2 OF THE SHOPPES AT DOGWOOD SPRINGS, A REPLAT OF ALL OF BLOCK 4, THE EAST HALF BLOCK 5, LOT 1 OF BLOCK 8, AND LOTS 1 AND 2 OF BLOCK 9, TOGETHER WITH A PORTION OF NORTHERN AVENUE, NORTH STREET, AND LIBERTY STREET ABANDONED RIGHT OF WAYS ADJACENT TO SAID BLOCKS, AS SHOWN ON THE PLAT OF THE TOWN OF COLLEGEVILLE, DATED JUNE 21, 1838.

VICINITY MAP



Prepared By:



10/20/2025

PREPARED FOR:

JON MARTIN

STATE OF ARKANSAS

LICENSED PROFESSIONAL ENGINEER

O No. 21954

SCOTT PARTY OF ARKANSAS

PRE-CONSTRUCTION COPY -

PLANS FOR BIDDING PURPOSES.

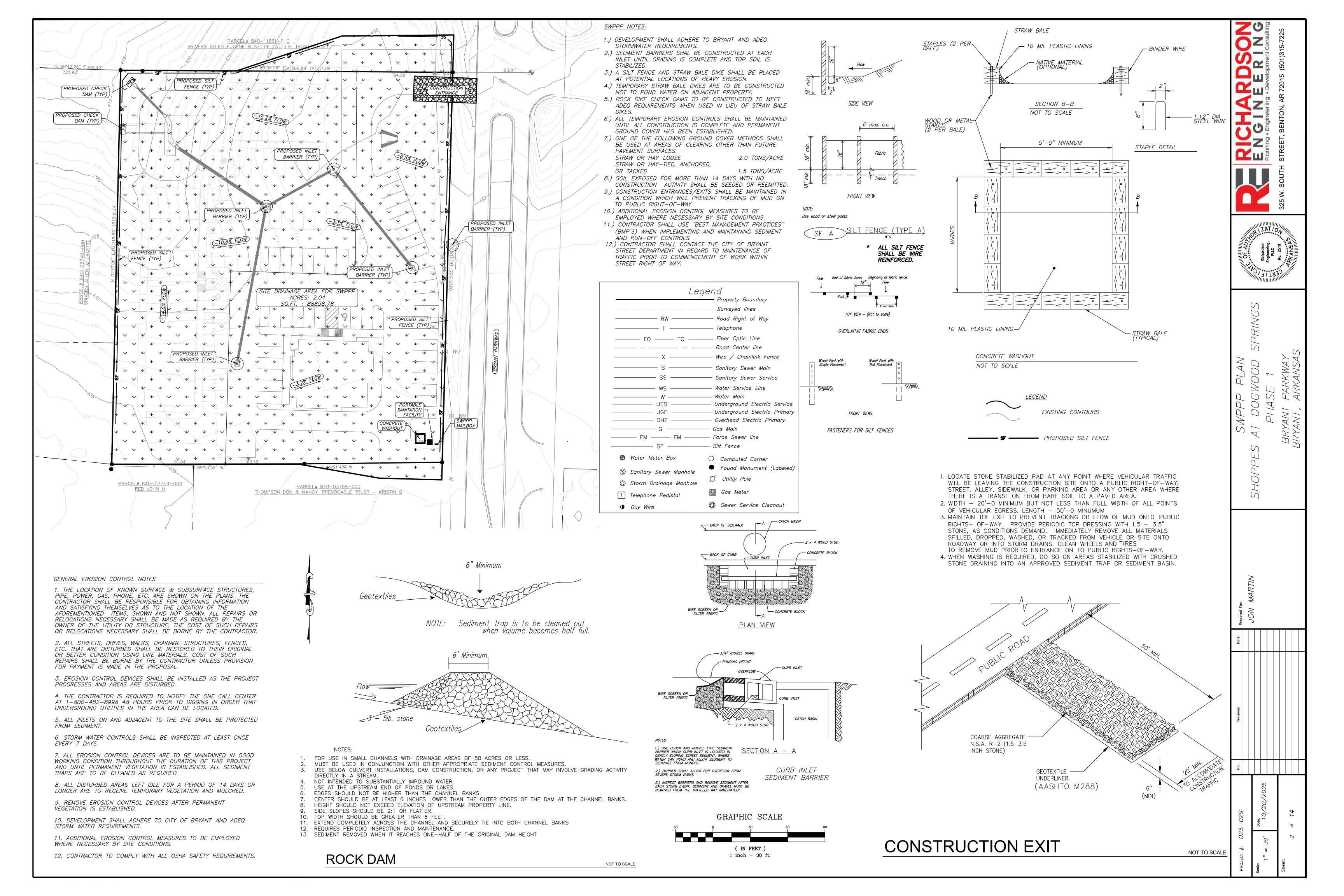
QUANTITIES TO BE VERIFIED PRIOR

TO CONSTRUCTION. CONTRACTOR

TO VERIFY GRADES WITH ENGINEER

PRIOR TO CONSTRUCTION.

			INDEX OF SHEETS	5
			COVER SHEET	1
			SWPPP	2
			DEMOLITION PLAN	3
			SITE PLAN	4
			SITE DIMENSION PLAN	5
	Revisions	Date	GRADING PLANS	6-7
			UTILITY PLANS	8-9
			DETAILS	10-12
1			LANDSCAPE PLAN	13
			PHASE PLAN	14



GENERAL CONSTRUCTION NOTES

A. THE CONTRACTOR WILL BE HELD SOLELY RESPONSIBLE FOR DAMAGES OCCURRING TO ANY PROPERTY DURING THE CONSTRUCTION OF THIS PROJECT. SAID CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PREVENT PROPERTY DAMAGE.

B. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL SOLELY AND COMPLETELY BE RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND WILL NOT BE LIMITED TO NORMAL WORKING HOURS.

C. ALL SITE AND UTILITY IMPROVEMENTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST REVISION TO THE BRYANT STANDARD SPECIFICATIONS.

D. CONTRACTOR IS TO REMOVE AND DISPOSE OF ALL DEBRIS, RUBBISH, AND OTHER MATERIALS RESULTING FROM PREVIOUS AND CURRENT DEMOLITION OPERATIONS. DISPOSAL WILL BE IN ACCORDANCE WITH ALL LOCAL, STATE, AND/OR FEDERAL REGULATIONS GOVERNING SUCH OPERATIONS.

E. PRIOR TO INSTALLATION OF ANY UTILITIES, THE CONTRACTOR IS TO EXCAVATE, VERIFY, AND CALCULATE ALL CROSSINGS AND INFORM ANY IMPACTED UTILITY OWNERS OF ANY CONFLICTS PRIOR TO CONSTRUCTION.

F. CONSTRUCTION SHALL NOT START ON ANY WATER UTILITY TIE-INS UNTIL APPROVAL IS GIVEN BY BRYANT. CONTRACTOR SHALL NOT OPERATE ANY VALVE, HYDRANT, OR WATER UTILITY APPURTENANCE NOR SHALL HE ATTACH TO OR TAP ANY WATER UTILITY MAIN WITHOUT APPROVAL FROM BRYANT. THE CONTRACTOR SHALL BEAR THE COST AND CONSEQUENCE OF ANY DISRUPTION OF UTILITY OPERATION CAUSED BY CONSTRUCTION.

G. FIBER OPTIC CABLE ON AND/OR ADJACENT TO THIS SITE WERE NOT LOCATED BY THE SURVEY AND ARE NOT SHOWN. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ANY FIBER OPTIC CABLES AT TO NEAR THIS SITE AND TAKE ALL NECESSARY AND REQUIRED PRECAUTIONS TO PROTECT ANY EXISTING FIBER OPTIC CABLES. CONTRACTORS SHALL COORDINATE ALL EFFORTS WITH OWNER OF FIBER OPTIC CABLES OR THEIR DESIGNATED REPRESENTATIVE.

H. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING "ONECALL" SERVICE TO MARK ALL UTILITIES PRIOR TO ANY DEMOLITION, EARTHWORK, OR UTILITY WORK ON THIS SITE.

I. ANY LAND CLEARING, CONSTRUCTION, OR DEVELOPMENT INVOLVING THE MOVEMENT OF MATERIALS, UTILITIES OR GROUND EXCAVATION SHALL BE IN ACCORDANCE WITH THE STORMWATER POLLUTION PREVENTION PLAN.

J. THE APPROXIMATE LOCATION OF KNOWN SURFACE AND SUBSURFACE STRUCTURES, PIPES, POWER, GAS, PHONE, ETC. ARE SHOWN ON THE DESIGN DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE EXACT LOCATION OF THE AFOREMENTIONED ITEMS, SHOWN AND NOT SHOWN.

K. CONTRACTOR TO ADHERE TO CURRENT OSHA REGULATIONS, INCLUDING EXCAVATION & TRENCH SAFETY.

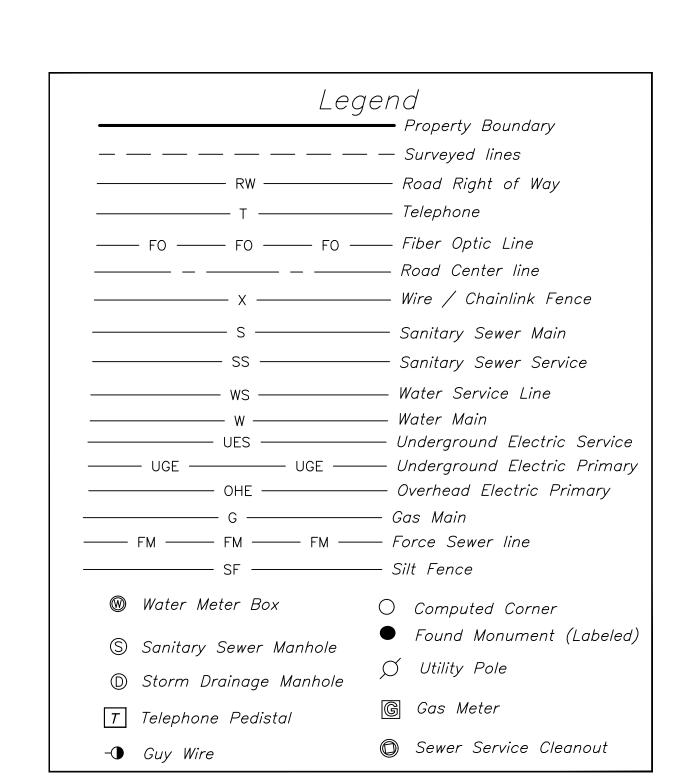
L. CONTRACTOR SHALL CONTACT THE CITY OF BRYANT STREET DEPARTMENT IN REGARD TO MAINTENANCE OF TRAFFIC PRIOR TO COMMENCEMENT OF WORK WITHIN STREET RIGHT OF

M. CONTRACTOR SHALL CONTACT CITY OF BRYANT PRIOR TO COMMENCEMENT OF UTILITY CONSTRUCTION OR TIE-INS.

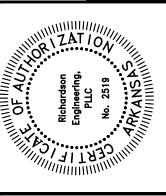


DEMO NOTES:

1) QUANTITATIVE AREAS ARE APPROXIMATE, TO BE VERIFIED BY THE DEMOLITION CONTRACTOR.







SHO

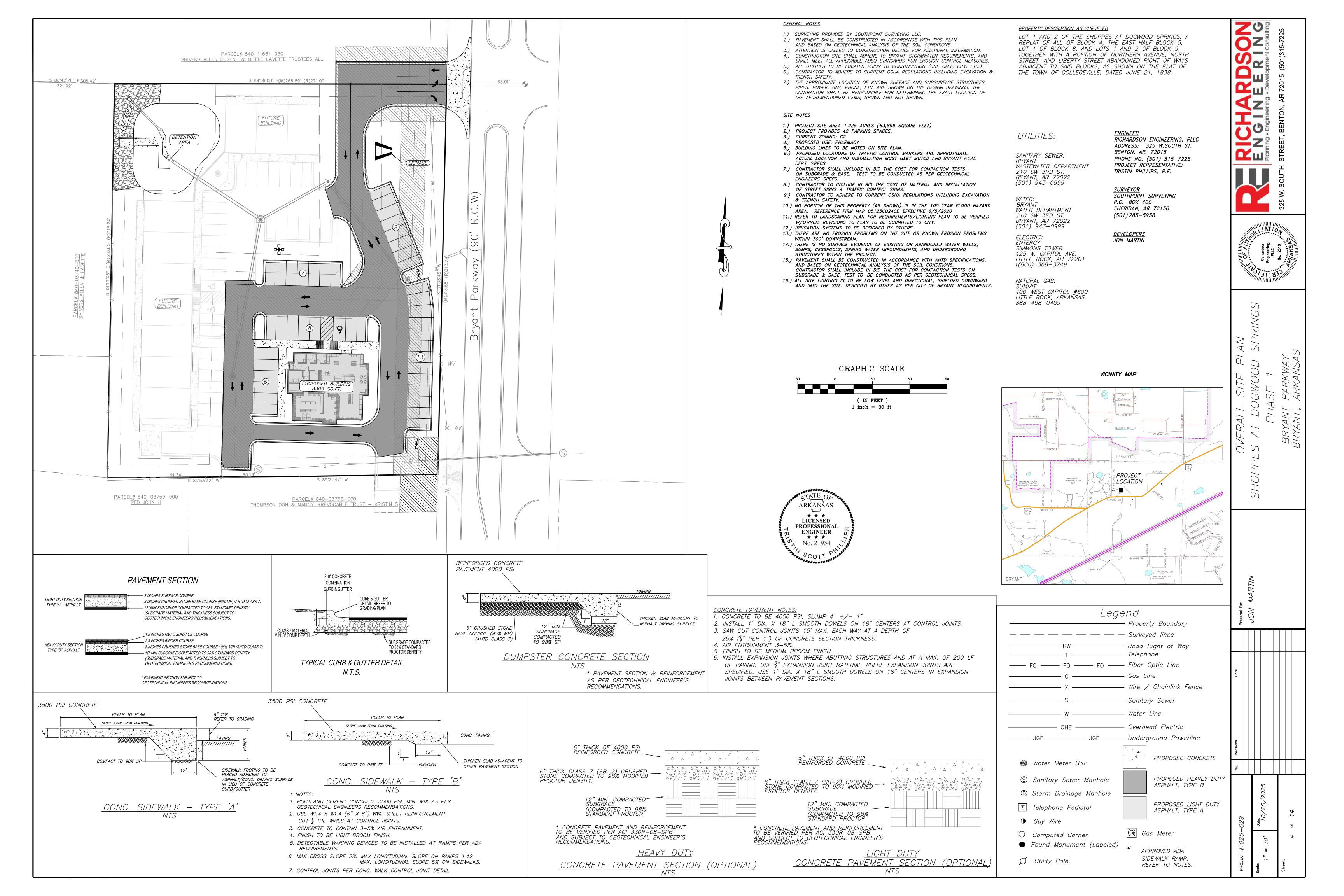
<u>UTILITIES:</u>

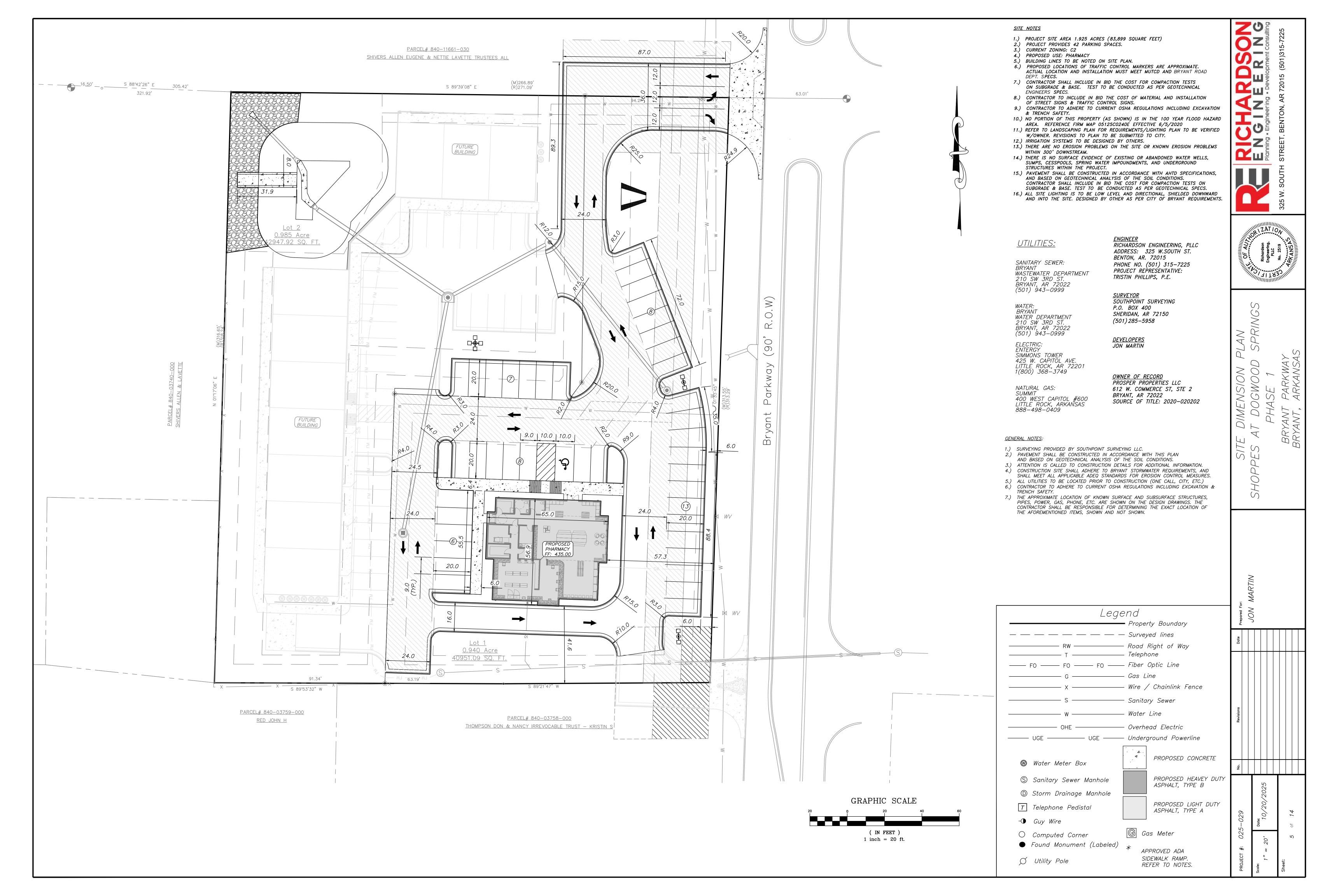
SANITARY SEWER: BRYANT WASTEWATER DEPARTMENT 210 SW 3RD ST. BRYANT, AR 72022 (501) 943-0999

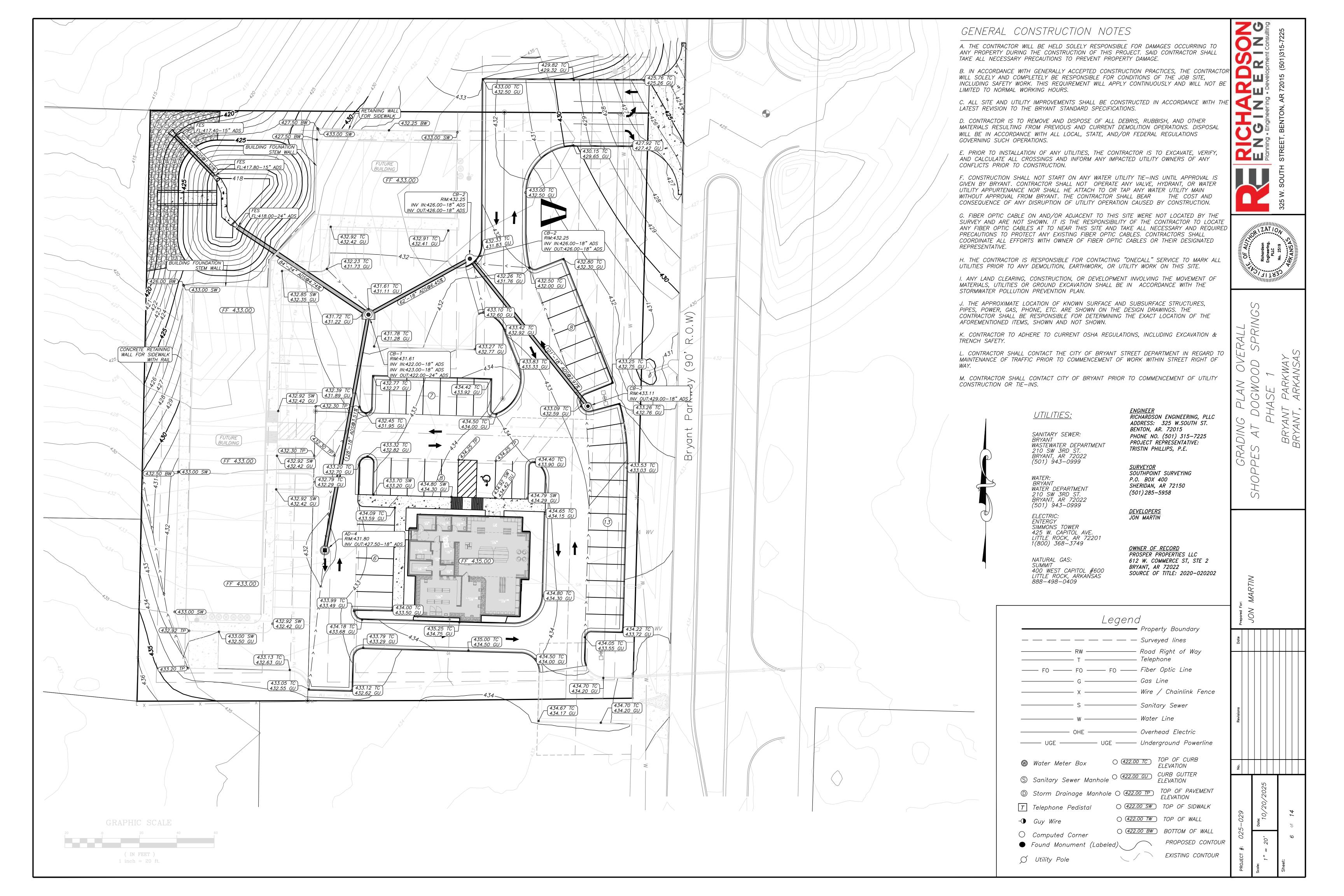
WATER: BRYANT WATER DEPARTMENT 210 SW 3RD ST. BRYANT, AR 72022 (501) 943—0999

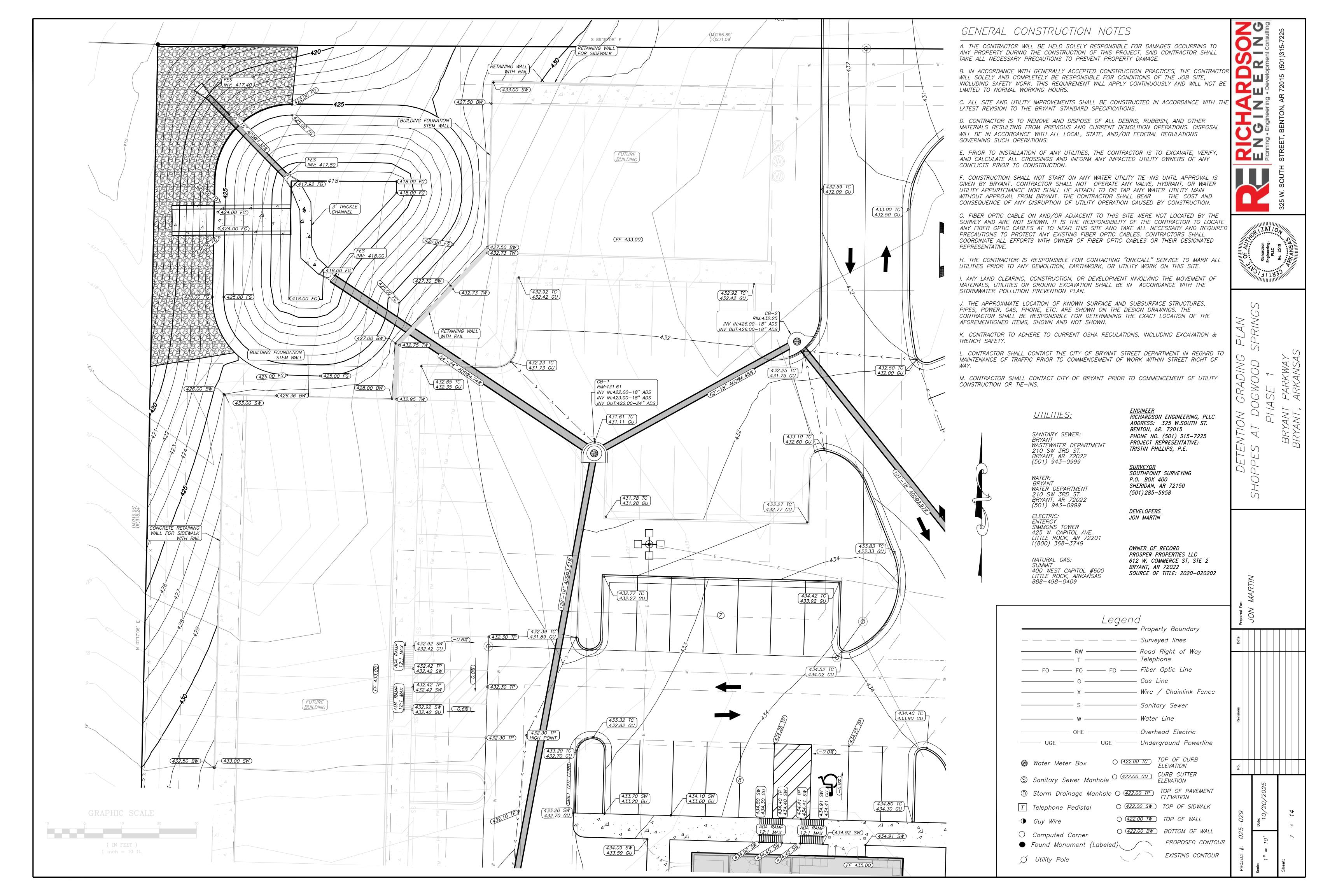
ELECTRIC: ENTERGY SIMMONS TOWER 31MMON3 TOWER 425 W. CAPITOL AVE. LITTLE ROCK, AR 72201 1(800) 368-3749

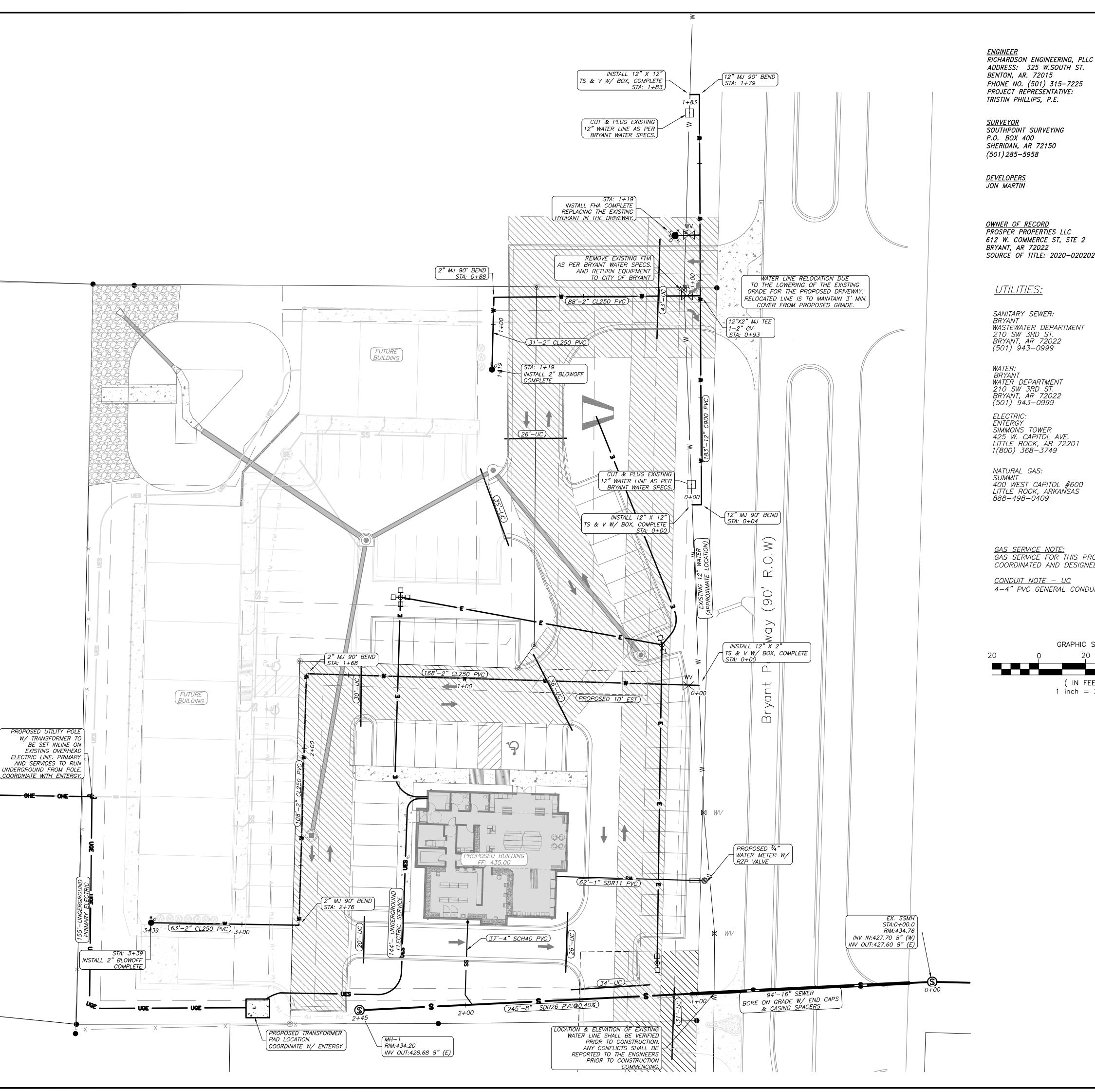
NATURAL GAS: SUMMIT 400 WEST CAPITOL #600 LITTLE ROCK, ARKANSAS 888-498-0409











GENERAL NOTES:

- 1.) ALL CONSTRUCTION AND MATERIALS TO MEET OR EXCEED CITY OF BRYANT SPECIFICATIONS.
- 2.) ALL SEWER GRAVITY MAINS SHALL BE SDR-26 PVC, UNLESS OTHERWISE SHOWN ON PLAN. 3.) ALL WATER MAINS SHALL BE SDR21 PVC PIPE UNLESS OTHERWISE SHOWN ON PLAN.
- 4.) ATTENTION IS CALLED TO WATER, SEWER, AND STREET LAYOUT FOR ADDITIONAL INFORMATION. 5.) ATTENTION IS CALLED TO CONSTRUCTION DETAILS FOR ADDITIONAL INFORMATION. 6.) ALL UTILITIES TO BE LOCATED PRIOR TO CONSTRUCTION (ONE CALL, CITY, UTILITY OWNER, ETC.)
- 7.) MINIMUM HORIZONTAL SEPARATION BETWEEN WATERLINES AND SEWERLINES SHALL BE 10'. 8.) MINIMUM VERTICAL SEPARATION BETWEEN WATERLINES AND SEWERLINES SHALL BE 18" (WATER ON TOP).
- 9.) CONTRACTOR TO ADHERE TO CURRENT OSHA REGULATIONS INCLUDING EXCAVATION & TRENCH SAFETY.
- 10.) BACKFILL MATERIAL FOR STREET CROSSINGS SHALL BE ACCORDING TO CITY OF BRYANT STREET DEPT SPECS AND COMPACTED TO 98% M.P. UNLESS OTHERWISE APPROVED IN WRITING BY THE ENGINEER AND UTILITY OWNER. 11.) THE APPROXIMATE LOCATION OF KNOWN SURFACE AND SUBSURFACE STRUCTURES, PIPES, POWER, GAS, PHONE, ETC. ARE SHOWN ON THE DESIGN DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE
- FOR DETERMINING THE EXACT LOCATION OF THE AFOREMENTIONED ITEMS, SHOWN AND NOT SHOWN. 12) CITY OF BRYANT MASTER SPECIFICATIONS APPLY. 13) CONTRACTOR SHALL CONTACT THE CITY OF BRYANT STREET DEPARTMENT IN REGARD TO MAINTENANCE
- OF TRAFFIC PRIOR TO COMMENCEMENT OF WORK WITHIN STREET RIGHT OF WAY. 14) CONTRACTOR SHALL CONTACT CITY OF BRYANT PRIOR TO COMMENCEMENT OF UTILITY CONSTRUCTION OR

SEWER NOTES:

1.) ALL CONSTRUCTION AND MATERIALS TO MEET OR EXCEED CITY OF BRYANT SPECIFICATIONS. 4) ATTENTION IS CALLED TO CONSTRUCTION DETAILS FOR ADDITIONAL INFORMATION.

SANITARY SEWER: WASTEWATER DEPARTMENT 210 SW 3RD ST. BRYANT, AR 72022

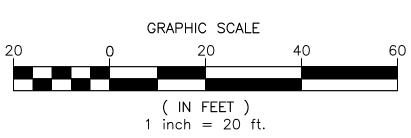
WATER DEPARTMENT 210 SW 3RD ST. BRYANT, AR 72022 (501) 943-0999

SIMMONS TOWER 425 W. CAPITOL AVE. LITTLE ROCK, AR 72201 1(800) 368–3749

400 WEST CAPITOL #600 LITTLE ROCK, ARKANSAS 888-498-0409

GAS SERVICE NOTE: GAS SERVICE FOR THIS PROJECT IS TO BE COORDINATED AND DESIGNED BY SUMMIT UTILITIES

<u>CONDUIT NOTE - UC</u> 4-4" PVC GENERAL CONDUITS

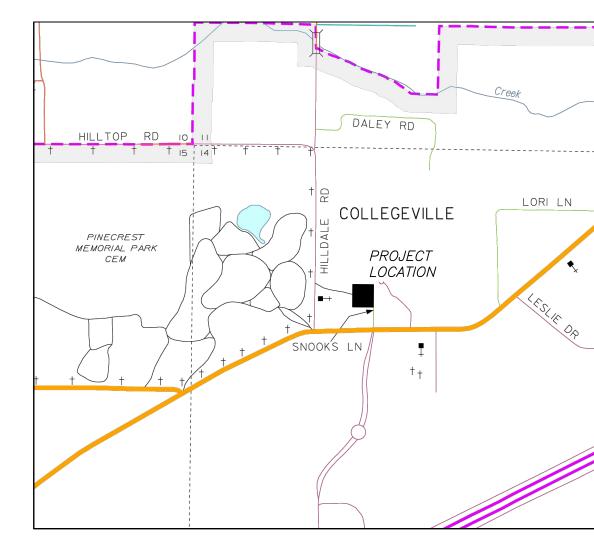


- 2.) ALL SERVICE LINES SHALL BE 4" SDR-21 OR SCH 40 PVC OR AS SPECIFIED ON THE DESIGN DRAWINGS. 3.) CONTRACTOR TO VERIFY METHOD OF CONNECTION WITH THE UTILITY OWNER PRIOR TO CONSTRUCTION. CONTRACTOR TO ADHERE TO CURRENT OSHA REGULATIONS INCLUDING EXCAVATION & TRENCH SAFETY. 6) BACKFILL FOR ALL DISTURBED (EXCAVATED) AREAS SHALL BE IN ACCORDANCE WITH
- CITY OF BRYANT STANDARD SPECS. 7) A 2-WAY CLEANOUT WITH BACKFLOW PREVENTER SHALL BE INSTALLED WITHIN 5 FEET OF THE BUILDING. DIRECTIONALY, THE CLEANOUT MUST SWEEP AWAY FROM THE VALVE TO PREVENT DAMAGE TO THE

WATER NOTES:

- 1.) ALL CONSTRUCTION AND MATERIALS TO MEET OR EXCEED CITY OF BRYANT SPECIFICATIONS.
- 2.) ALL SERVICE LINES AND METER SETTINGS SHALL BE AS PER CITY OF BRYANT SPECS. 3.) 12ga BLUE COATED COPPER TRACING WIRE TO BE INSTALLED WITH ALL WATERLINES (MAINS & SERVICES).
- 4.) ALL FITTINGS SHALL BE DUCTILE IRON M.J. (WHERE AVAILABLE). 5.) ATTENTION IS CALLED TO CONSTRUCTION DETAILS FOR ADDITIONAL INFORMATION.
- 6.) MINIMUM SEPARATION BETWEEN WATERLINES & SEWERLINES SHALL BE 10' 7.) MINIMUM VERTICAL SEPARATION BETWEEN WATERLINE & SEWERLINE CROSSINGS SHALL BE 18" (WATER ON TOP).
- 8.) CONTRACTOR SHALL ADHERE TO CURRENT OSHA REGULATIONS INCLUDING EXCAVATION & TRENCH SAFETY. 9.) STREET CROSSINGS TO MEET CITY OF BRYANT STREET DEPT SPECIFICATIONS.
- 10.) CONTRACTOR TO ADHERE TO AWWA SPECS FOR BLOCKING AND ANCHORING. 11.) ON-SITE FIRE HYDRANTS SHALL BE PER CITY OF BRYANT SPECIFICATIONS.

VICINITY MAP



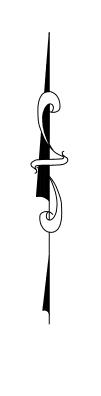
Lec	gend
_	Property Boundary
	— — Surveyed lines
RW	Road Right of Way
т	—— Telephone
— FO — FO — FO -	—— Fiber Optic Line
	—— Road Center line
X	—— Wire / Chainlink Fence
s	Sanitary Sewer Main
SS	— Sanitary Sewer Service
WS	— Water Service Line
w	
UES	— Underground Electric Service
	— Underground Electric Primary
——————————————————————————————————————	— Overhead Electric Primary
G	— Gas Main
—— FM ——— FM —	— Force Sewer line
SF	— Silt Fence
₩ Water Meter Box	O Computed Corner
S Sanitary Sewer Manhole	• Found Monument (Labeled)
_	◯ Utility Pole
	,-

T Telephone Pedistal

- Guy Wire

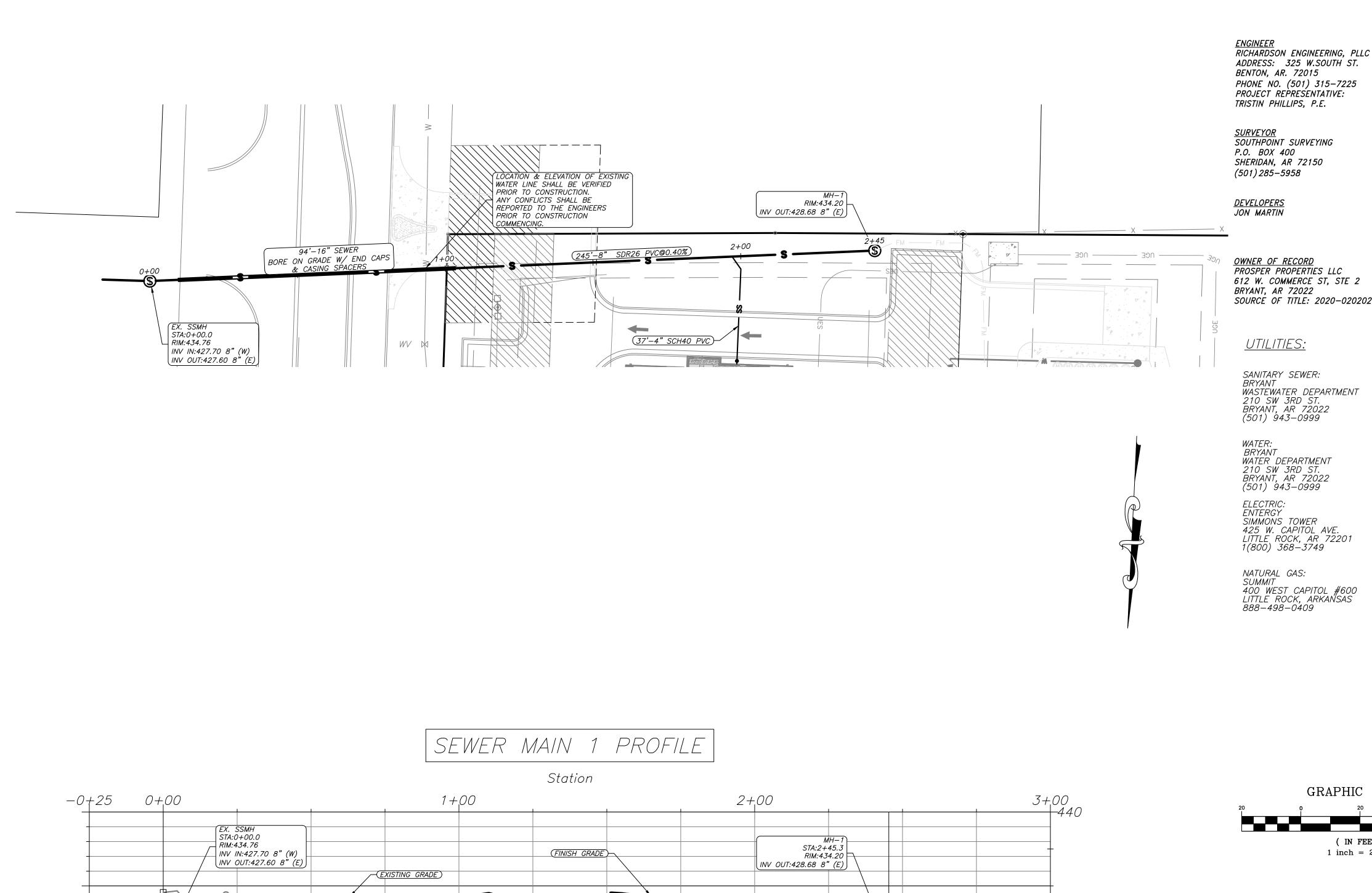
G Gas Meter

Sewer Service Cleanout



∞2~

ZATIOA



12" WATER LINE APPROX. LOCATION CONTRACTOR VERIFY

94'-16" SEWER BORE ON GRADE W/ END CAPS

1+00

& CASING SPACERS

0+75

0+00

0+25

0+50

245'-8" SDR26 PVC@0.40%

1+25

1+50

1 + 75

2+00

2+25

2+50

2+75

GENERAL NOTES:

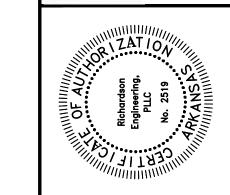
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- 12) CITY OF BRYANT MASTER SPECIFICATIONS APPLY. 13) CONTRACTOR SHALL CONTACT THE CITY OF BRYANT STREET DEPARTMENT IN REGARD TO MAINTENANCE

FOR DETERMINING THE EXACT LOCATION OF THE AFOREMENTIONED ITEMS, SHOWN AND NOT SHOWN.

OF TRAFFIC PRIOR TO COMMENCEMENT OF WORK WITHIN STREET RIGHT OF WAY. 14) CONTRACTOR SHALL CONTACT CITY OF BRYANT PRIOR TO COMMENCEMENT OF UTILITY CONSTRUCTION OR

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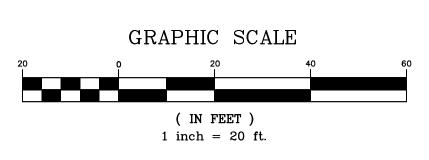


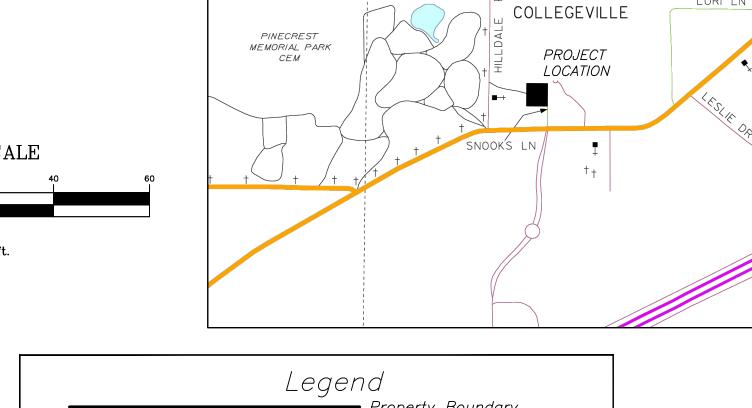
ON Consulting

SZ-

VICINITY MAP

DALEY RD LORI LN COLLEGEVILLE PINECREST MEMORIAL PARK PROJECT LOCATION





l ed	gend
9	—— Property Boundary
	– Surveyed lines
RW	Road Right of Way
т	—— Telephone
—— FO —— FO —— FO —	— Fiber Optic Line
	— Road Center line
X	—— Wire / Chainlink Fence
s	— Sanitary Sewer Main
SS	— Sanitary Sewer Service
———— WS ————	— Water Service Line
w	
	— Underground Electric Service
	— Underground Electric Primary
———— OHE ————	— Overhead Electric Primary
G	— Gas Main
—— FM —— FM —	— Force Sewer line
SF	— Silt Fence
₩ Water Meter Box	O Computed Corner
g weter meter ben	O Computed Corner
S Sanitary Sewer Manhole	 Found Monument (Labeled)
_	Utility Pole
Storm Drainage Manhole	
T Telephone Pedistal	

PROJECT REPRESENTATIVE: TRISTIN PHILLIPS, P.E. <u>SURVEYOR</u> SOUTHPOINT SURVEYING SHERIDAN, AR 72150 (501) 285-5958

<u>DEVELOPERS</u> JON MARTIN

OWNER OF RECORD PROSPER PROPERTIES LLC 612 W. COMMERCE ST, STE 2 BRYANT, AR 72022 SOURCE OF TITLE: 2020-020202

<u>UTILITIES:</u>

SANITARY SEWER: WASTEWATER DEPARTMENT 210 SW 3RD ST. BRYANT, AR 72022 (501) 943-0999

WATER DEPARTMENT 210 SW 3RD ST. BRYANT, AR 72022 (501) 943-0999

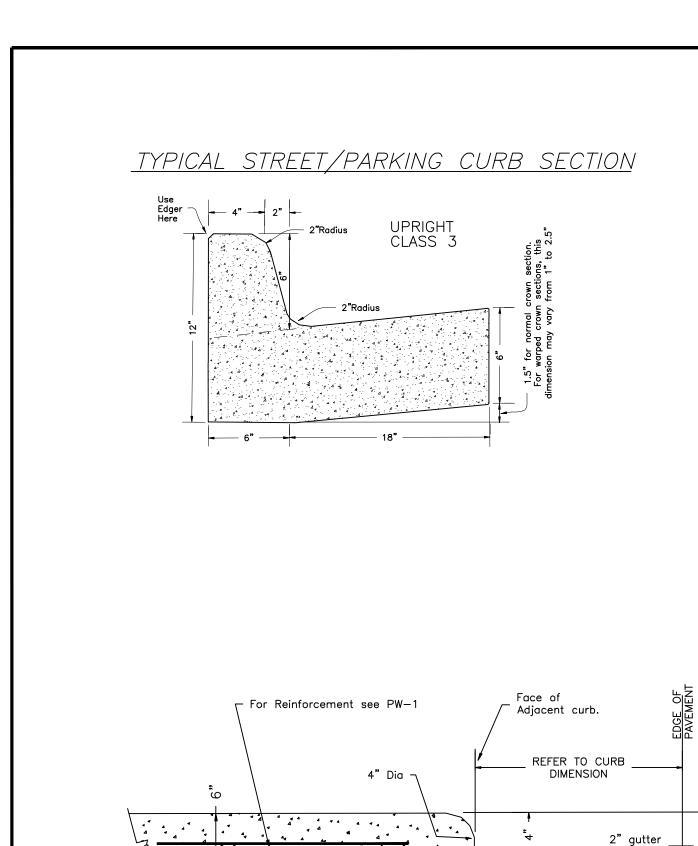
ENTERGY SIMMONS TOWER 425 W. CAPITOL AVE. LITTLE ROCK, AR 72201 1(800) 368–3749

NATURAL GAS: 400 WEST CAPITOL #600 LITTLE ROCK, ARKANSAS

T Telephone Pedistal

-**①** Guy Wire

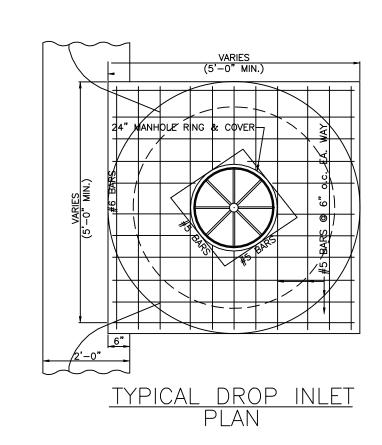
Sewer Service Cleanout

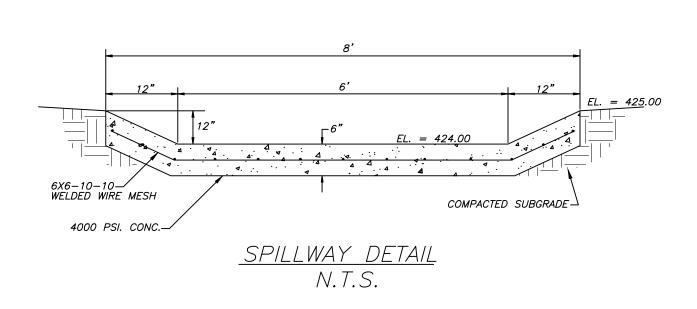


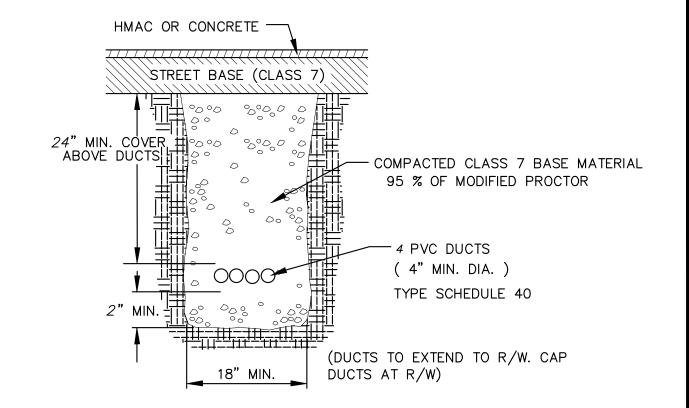
PLACE 4" DIA. STOOLS AT CENTERLINE OF INLET,

FROM CENTERLINE NOT TO EXCEED 4'.

THEN SPACE ADDITIONAL STOOLS EQUAL DISTANCE?







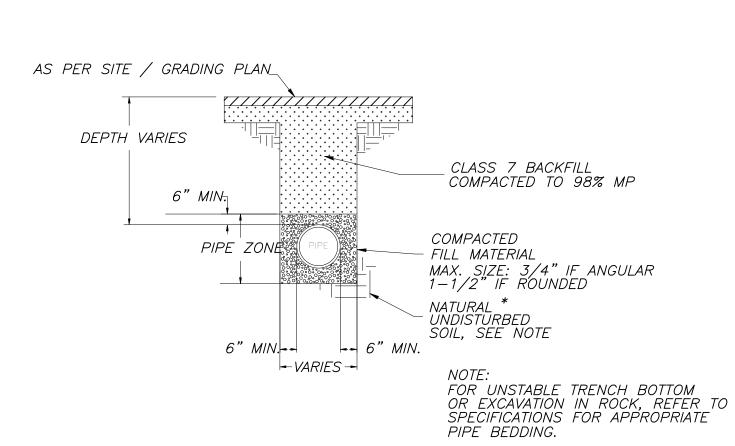


NOT TO SCALE

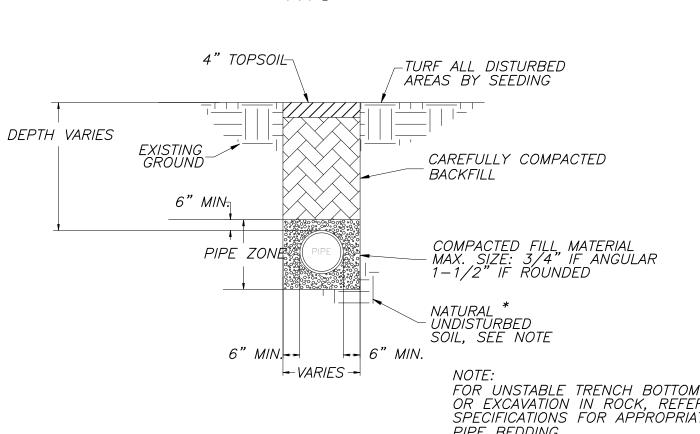
GENERAL NOTES

- 1.) ALL STOP SIGNS SHALL BE 30" X 30". 2.) PROPOSED LOCATIONS OF TRAFFIC CONTROL MARKERS ARE APPROXIMATE. ACTUAL LOCATION AND INSTALLATION MUST MEET
- MUTCD AND CITY OF BRYANT SPECS. 3.) PAVEMENT SHALL BE CONSTRUCTED IN ACCORDANCE WITH ENGINEER'S
- SPECIFICATIONS, AND BASED ON GEOTECHNICAL ANALYSIS OF THE SOIL CONDITIONS.
- 4.) CONTRACTOR SHALL INCLUDE IN BID THE COST FOR COMPACTION TESTS ON SUBGRADE & BASE. TEST TO BE CONDUCTED AS PER CITY OF BRYANT STREET DEPARTMENT SPECIFICATIONS.
- 5.) CURB INLETS CONSTRUCTED SO THAT POOLING OF WATER DOES NOT OCCUR AT INTERSECTIONS.
- 6.) ALL STORM BOX LIDS SHALL BE PER CITY OF BRYANT
- 7.) ALL SIDEWALKS AND CURB/GUTTER TO BE CONSTRUCTED PER CITY OF BRYANT SPECIFICATIONS.
- 8.) ALL SIDEWALKS TO MEET ADA REQUIREMENTS. 9.) * - INDICATES ADA APPROVED HANDICAP RAMP.
- 10.) DEVELOPMENT SHALL ADHERE TO CITY OF BRYANT STORMWATER REQUIREMENTS.
- 11.) THE APPROXIMATE LOCATION OF KNOWN SURFACE AND SUBSURFACE STRUCTURES, PIPES, POWER, GAS, PHONE, ETC. ARE SHOWN ON THE DESIGN DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE EXACT LOCATION OF THE AFOREMENTIONED ITEMS, SHOWN AND NOT SHOWN. ALL REPAIRS OR RELOCATIONS NECESSARY SHALL BE MADE AS REQUIRED BY THE UTILITY OR OWNER OF THE STRUCTURE. THE COST OF SAID REPAIRS OR RELOCATIONS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

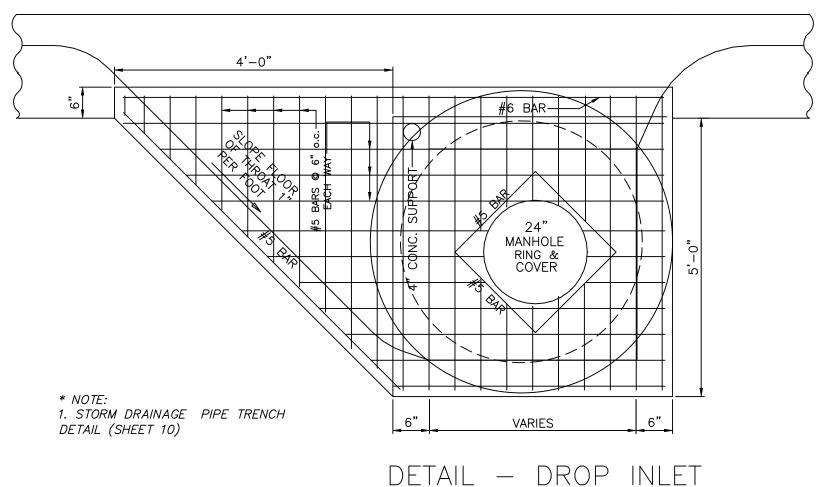
REFER TO CITY OF BRYANT, ARKANSAS PUBLIC WORKS STREET AND DRAINAGE STANDARD DETAILS FOR ADDITIONAL DETAILS AND INFORMATION.

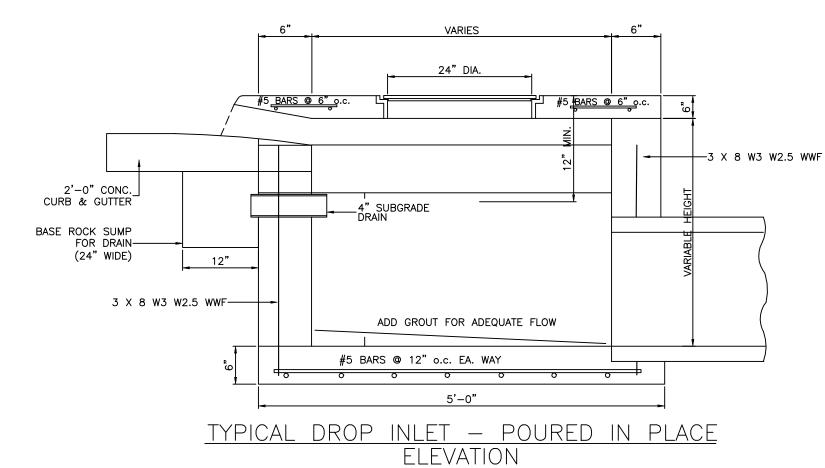




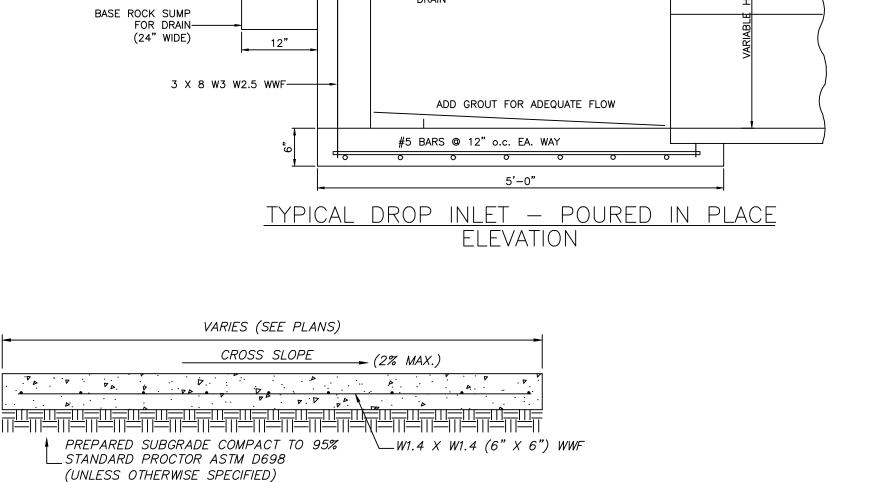


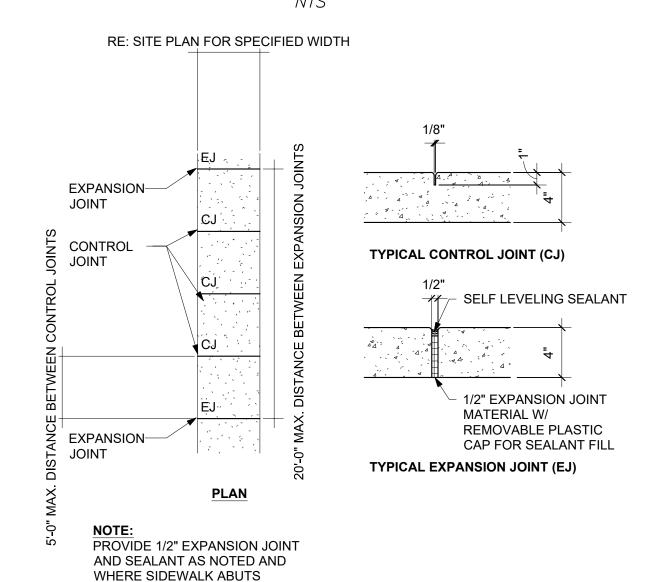
STORM DRAINAGE PIPE TRENCH (UNPAVED AREA





<u>with extended throat</u>





CONC. WALK/CONCRETE SWALE CONTROL JOINT DETAILS

REINFORCED

INLET THROAT BETWEEN STOOLS

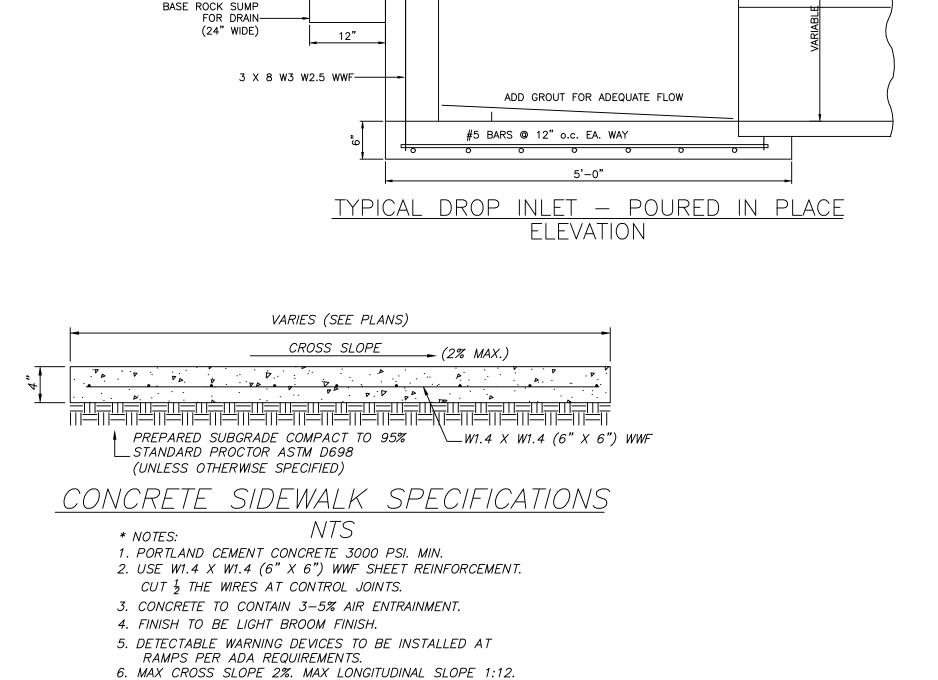
NOTE: This detail is not to be used for inlets in curb radius. See PW-10 for detail of inlet top in curb radius.

THROAT OPENING (NOT IN RADIUS)

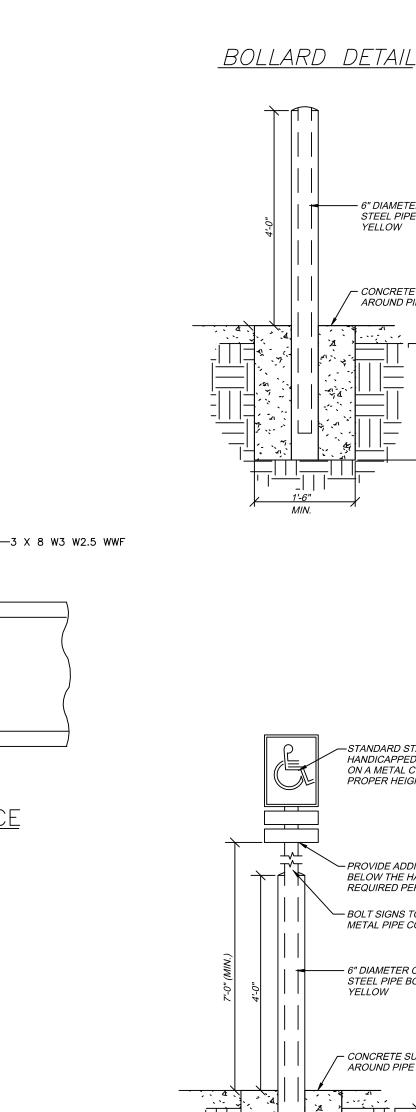
STRUCTURES OR OTHER

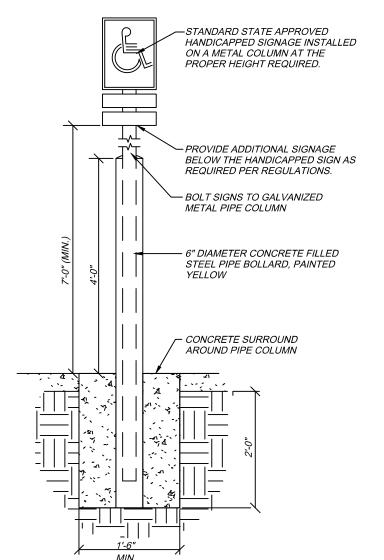
SIDEWALKS.

BOX REQUIRED



7. CONTROL JOINTS PER CONC. WALK CONTROL JOINT DETAIL.



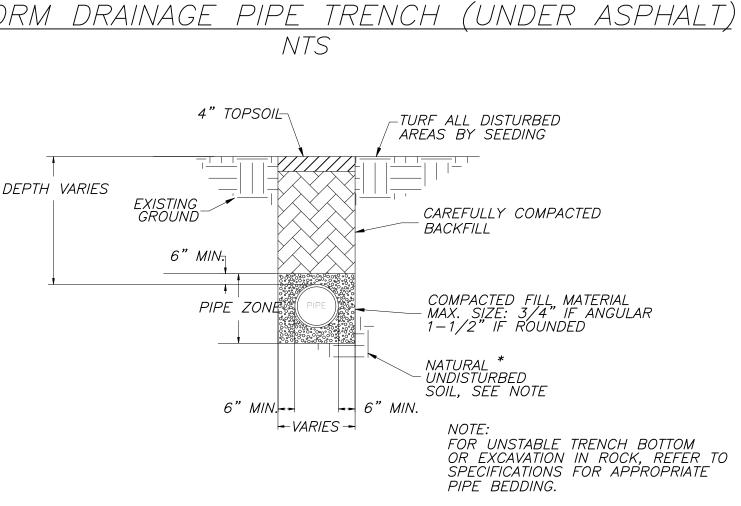


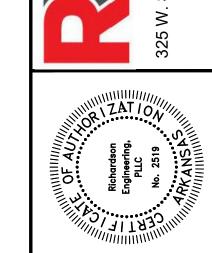
HANDICAP SIGN DETAIL

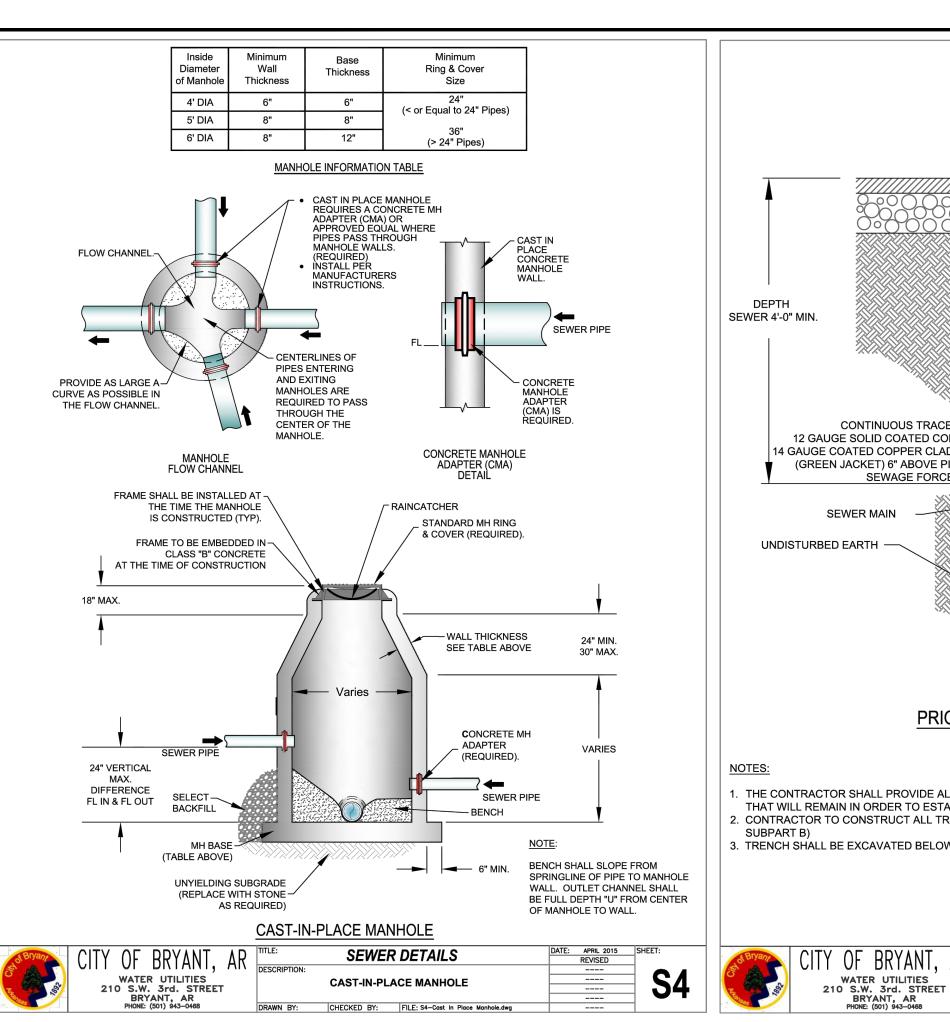
STEEL PIPE BOLLARD, PAINTED

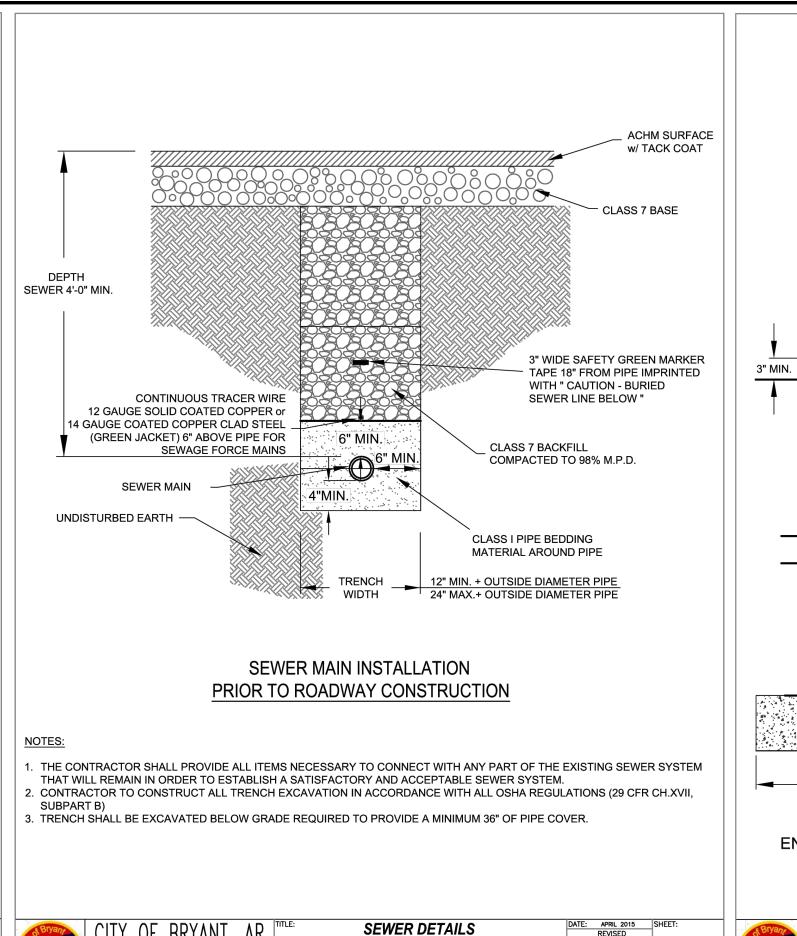
- CONCRETE SURROUND

AROUND PIPE COLUMN



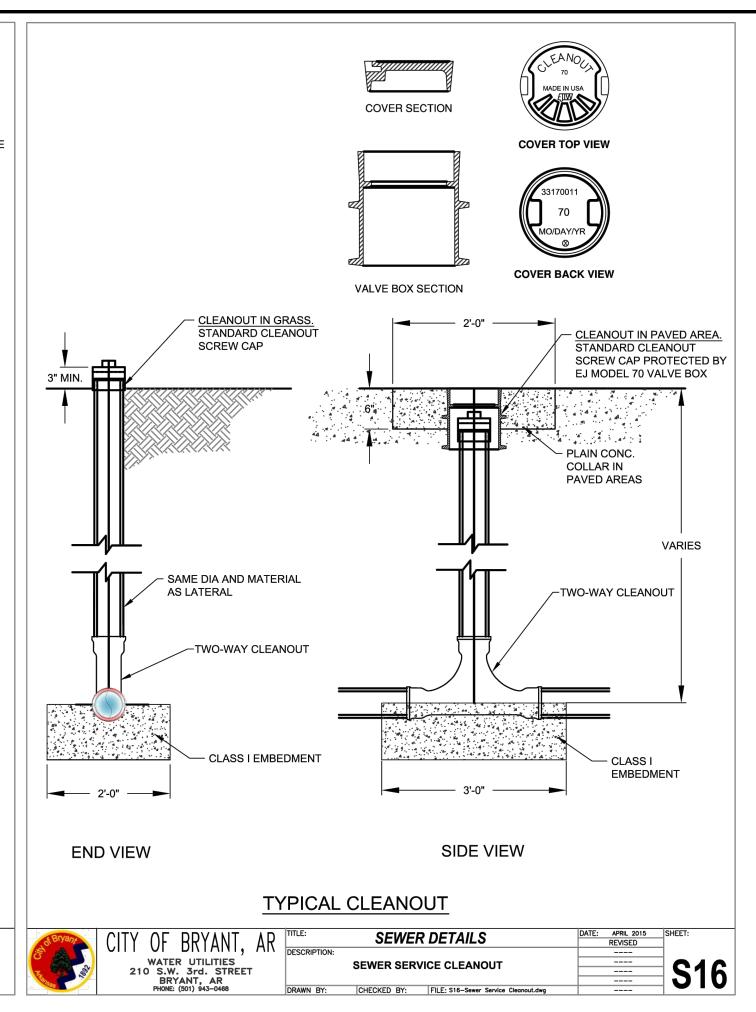


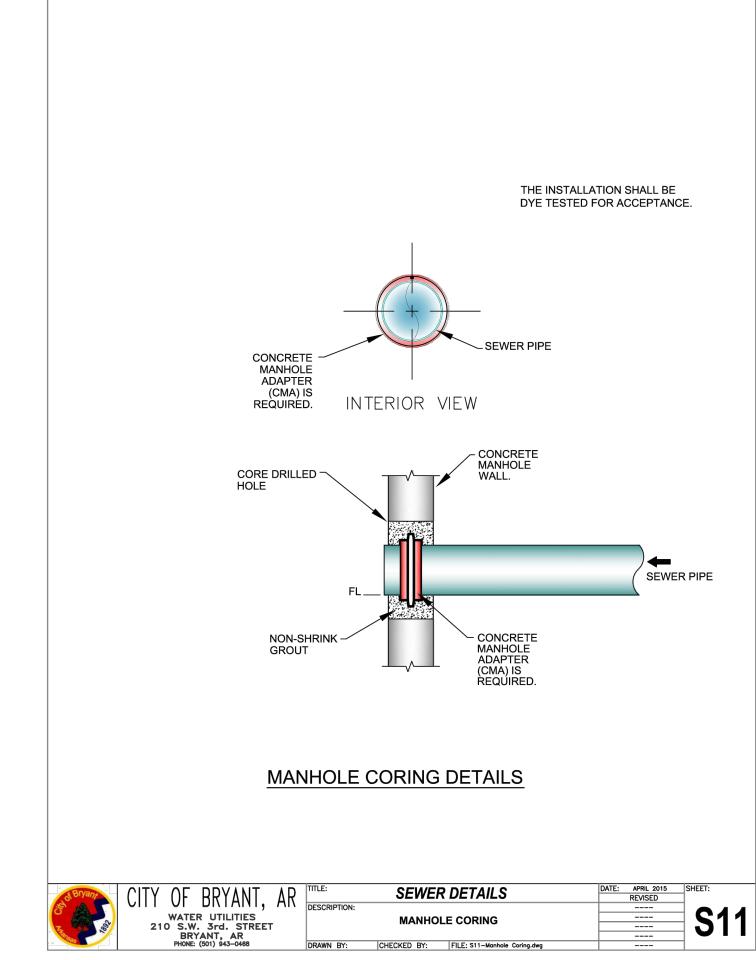




SEWER MAIN INSTALLATION

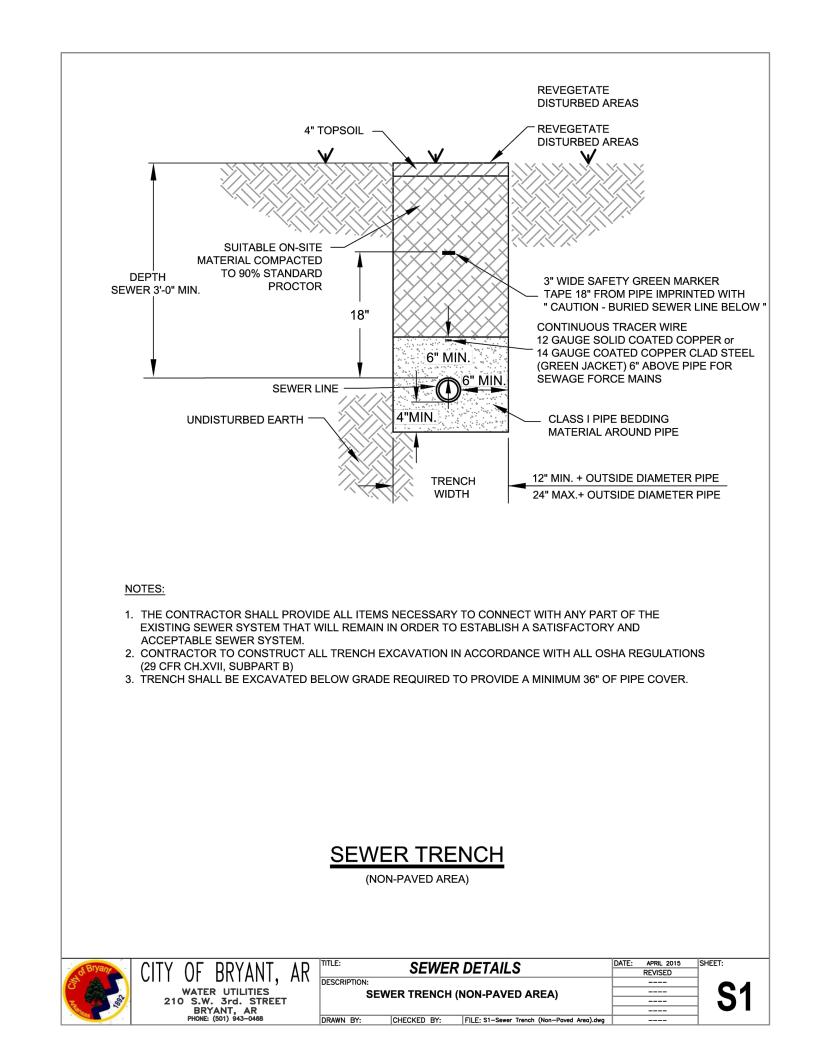
PRIOR TO ROADWAY CONSTRUCTION

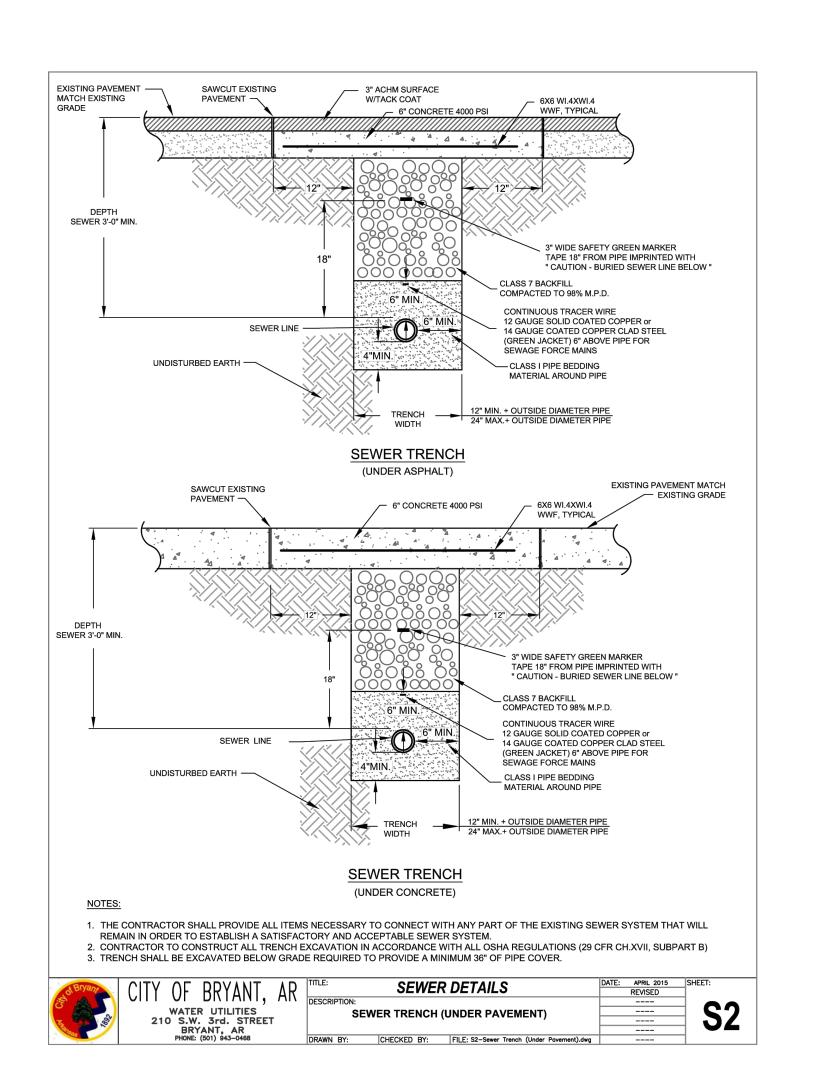


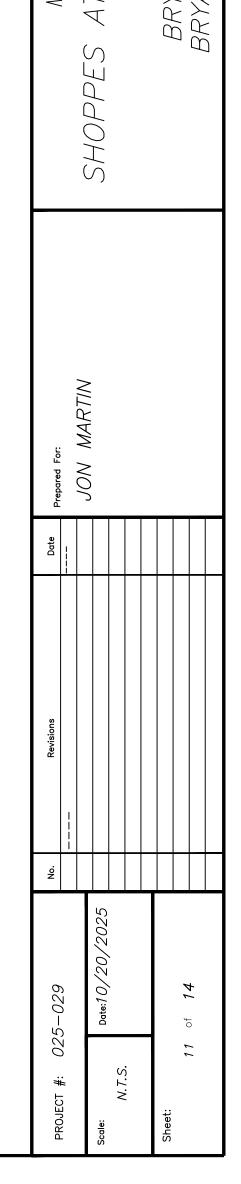


SEWER NOTES:

- 1.) ALL CONSTRUCTION AND MATERIALS TO MEET OR EXCEED CITY OF BRYANT SPECIFICATIONS. 2.) ALL SERVICE LINES SHALL BE 4" SDR—21 OR SCH 40 PVC OR AS SPECIFIED ON THE DESIGN DRAWINGS.
- 2.) ALL SERVICE LINES SHALL BE 4 SDR—21 OR SCH 40 PVC OR AS SPECIFIED ON THE DESIGN DRAWING 3.) CONTRACTOR TO VERIFY METHOD OF CONNECTION WITH THE UTILITY OWNER PRIOR TO CONSTRUCTION.
- 4) ATTENTION IS CALLED TO CONSTRUCTION DETAILS FOR ADDITIONAL INFORMATION. 5) CONTRACTOR TO ADHERE TO CURRENT OSHA REGULATIONS INCLUDING EXCAVATION & TRENCH SAFETY
- 6) BACKFILL FOR ALL DISTURBED (EXCAVATED) AREAS SHALL BE IN ACCORDANCE WITH CITY OF BRYANT STANDARD SPECS.
- 7) A 2-WAY CLEANOUT WITH BACKFLOW PREVENTER SHALL BE INSTALLED WITHIN 5 FEET OF THE BUILDING. DIRECTIONALY, THE CLEANOUT MUST SWEEP AWAY FROM THE VALVE TO PREVENT DAMAGE TO THE BACKFLOW PREVENTER.



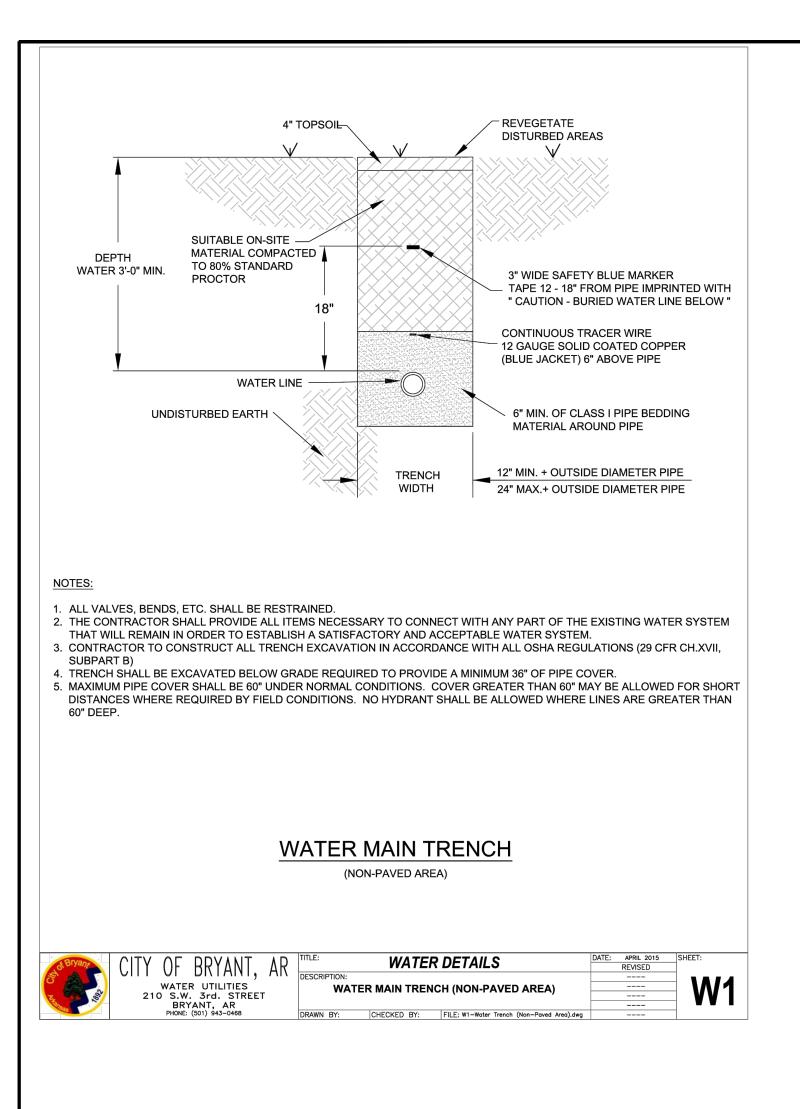


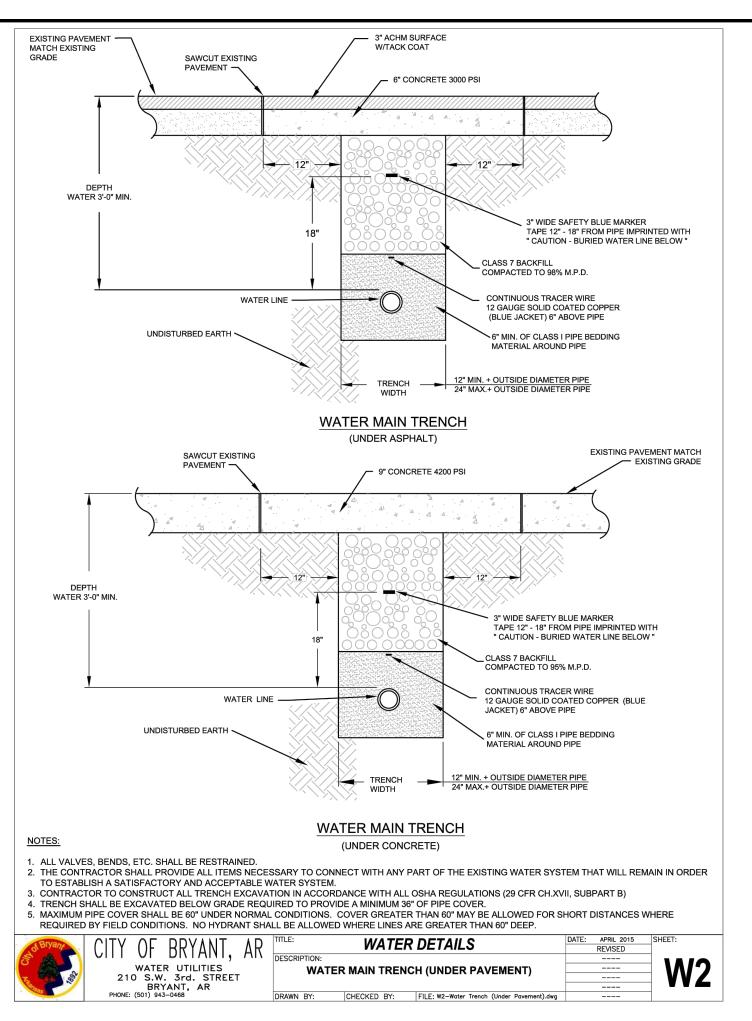


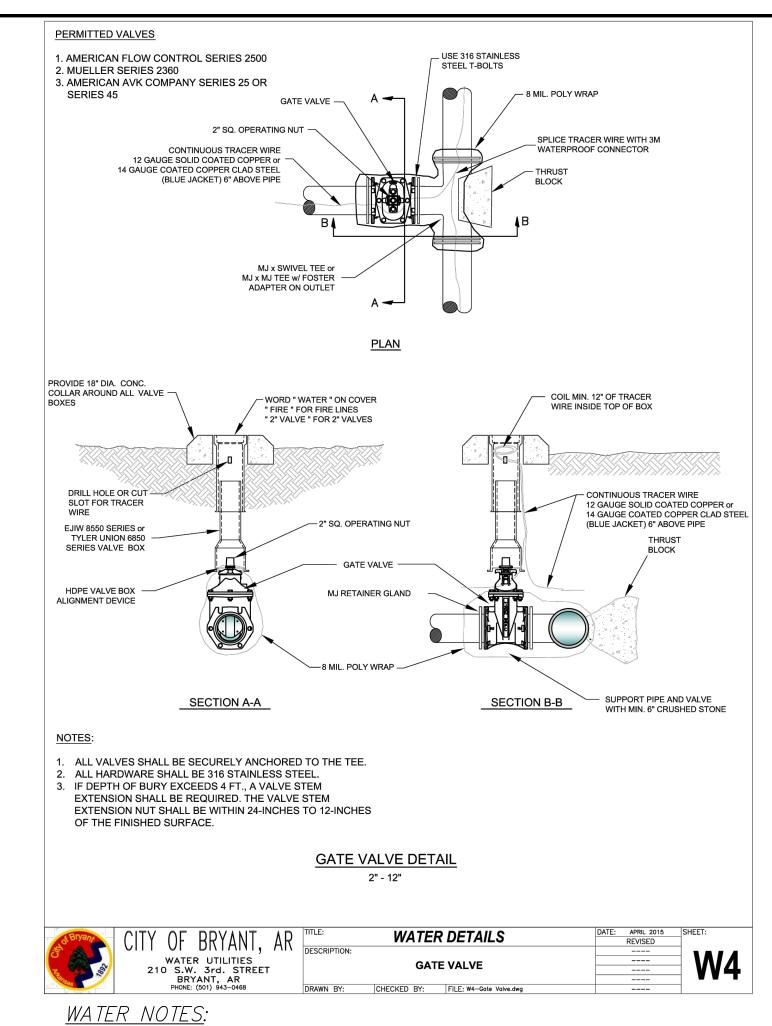
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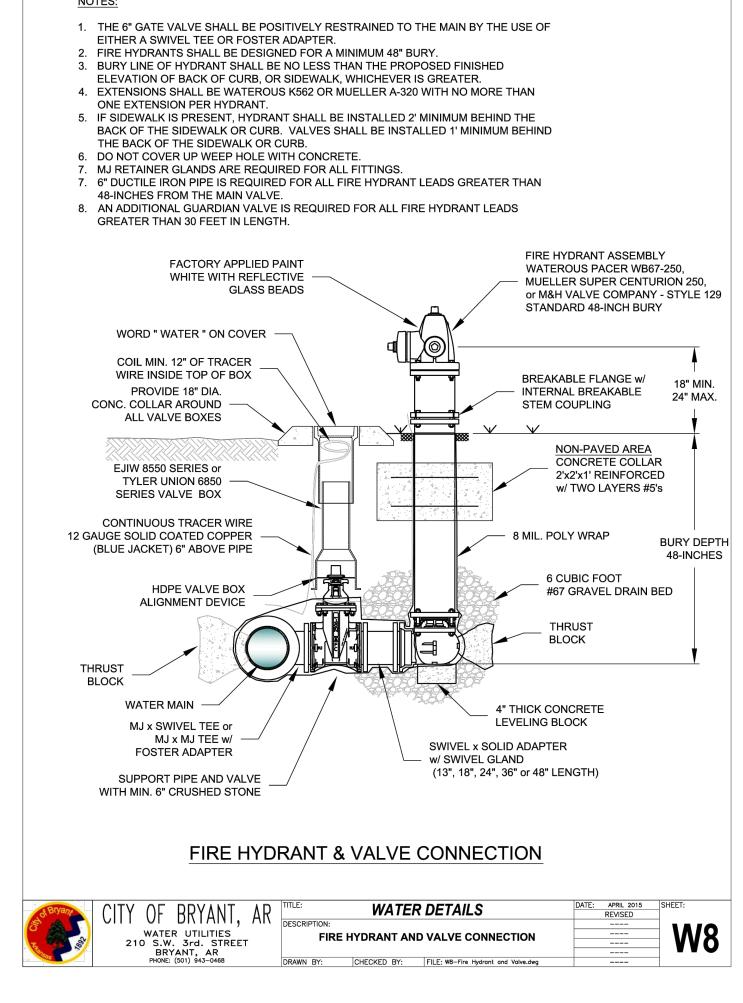
1.) ALL CONSTRUCTION AND MATERIALS TO MEET OR EXCEED CITY OF BRYANT SPECIFICATIONS.
2.) ALL SERVICE LINES AND METER SETTINGS SHALL BE AS PER CITY OF BRYANT SPECS.

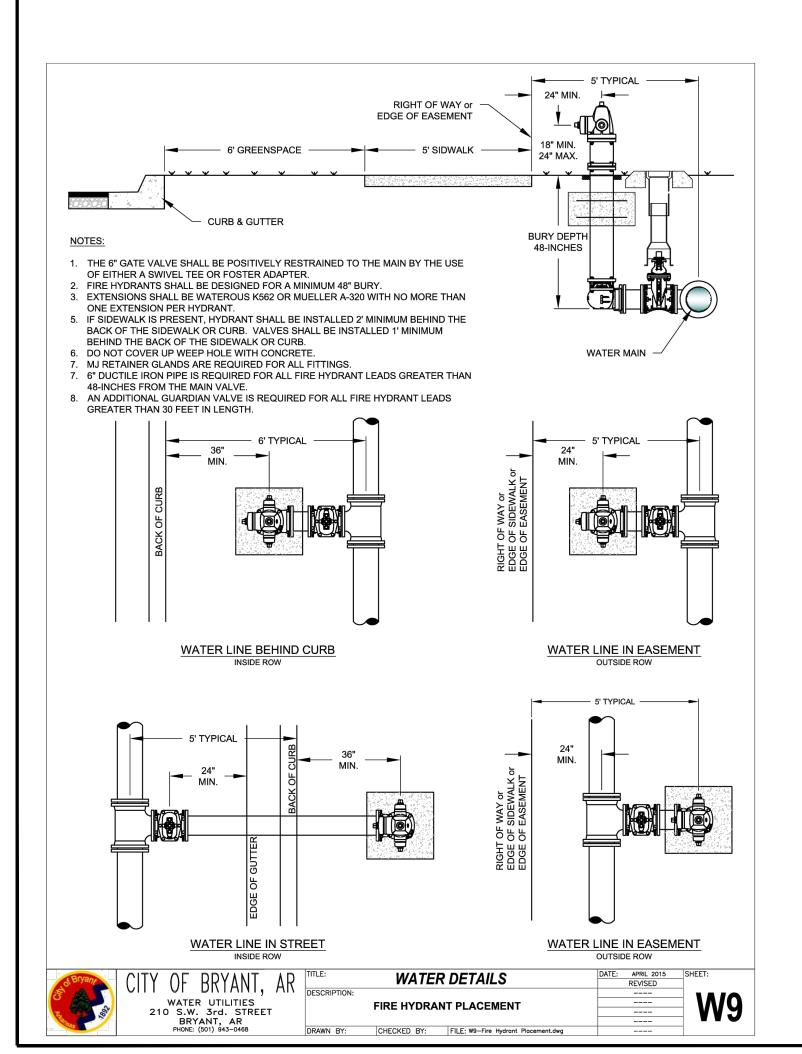
5.) ATTENTION IS CALLED TO CONSTRUCTION DETAILS FOR ADDITIONAL INFORMATION.
6.) MINIMUM SEPARATION BETWEEN WATERLINES & SEWERLINES SHALL BE 10'

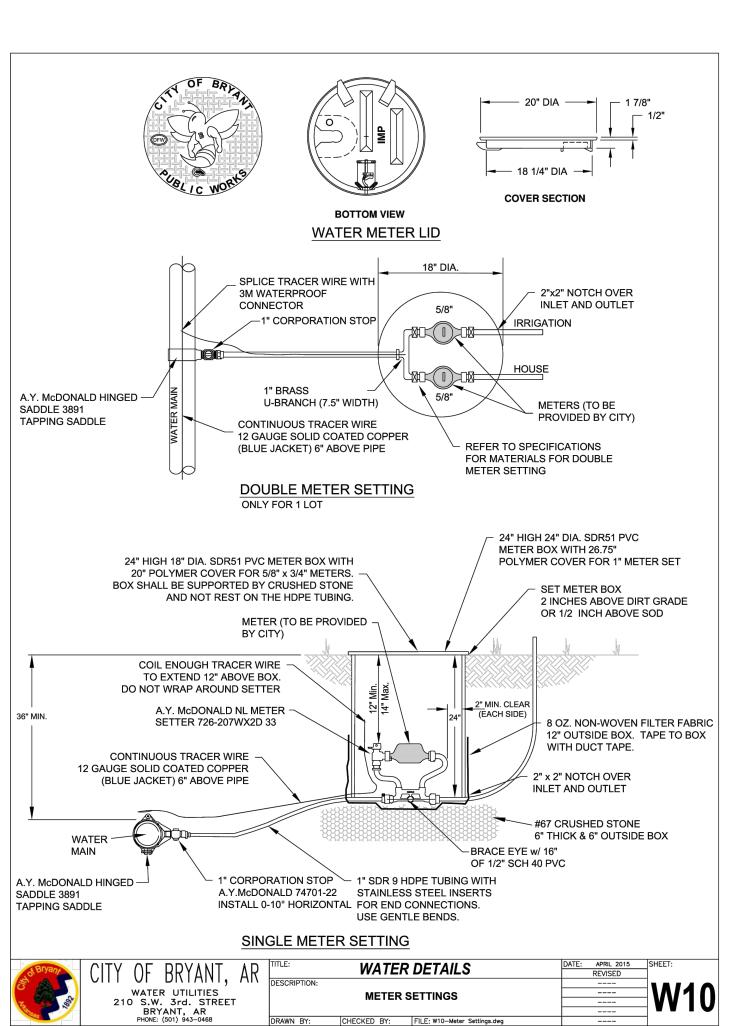
4.) ALL FITTINGS SHALL BE DUCTILE IRON M.J. (WHERE AVAILABLE).

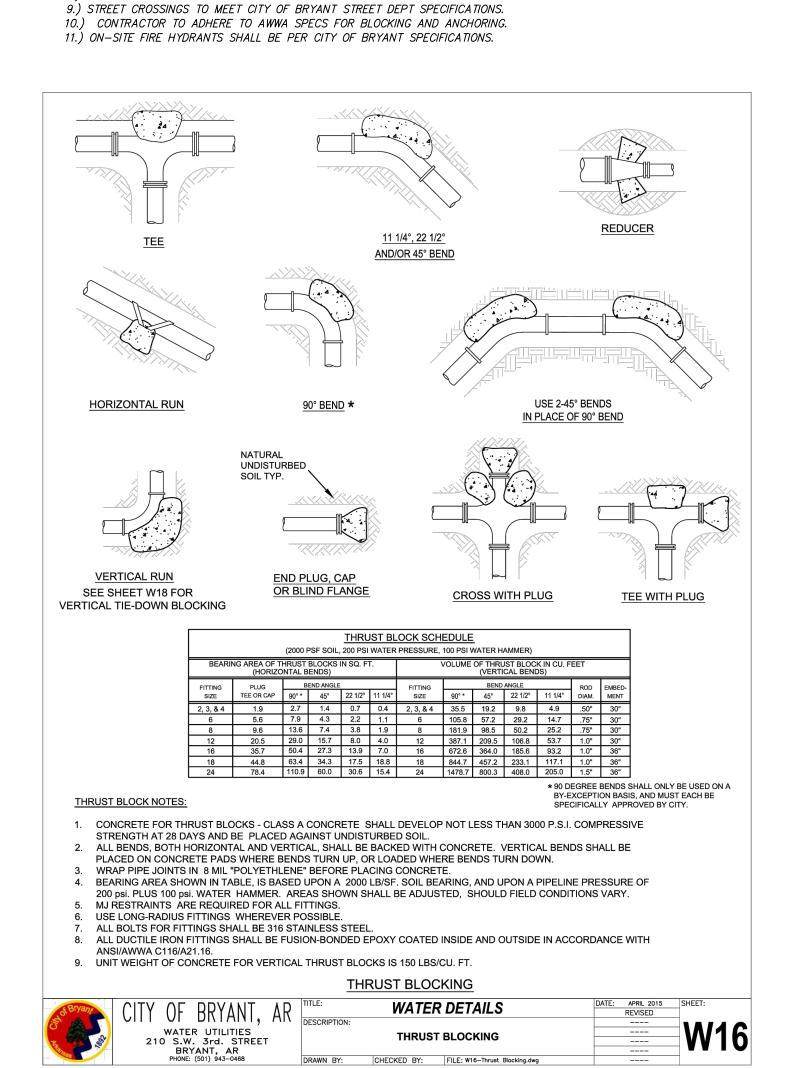
3.) 12ga BLUE COATED COPPER TRACING WIRE TO BE INSTALLED WITH ALL WATERLINES (MAINS & SERVICES).

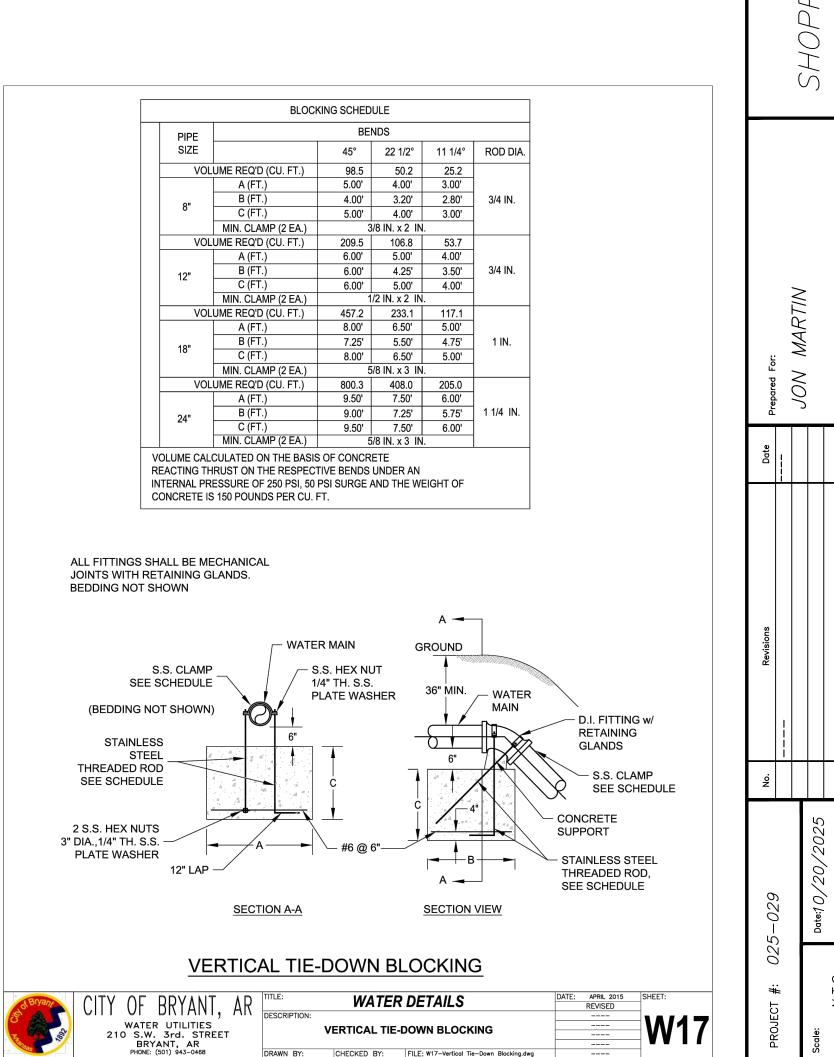
7.) MINIMUM VERTICAL SEPARATION BETWEEN WATERLINE & SEWERLINE CROSSINGS SHALL BE 18" (WATER ON TOP).
8.) CONTRACTOR SHALL ADHERE TO CURRENT OSHA REGULATIONS INCLUDING EXCAVATION & TRENCH SAFETY.

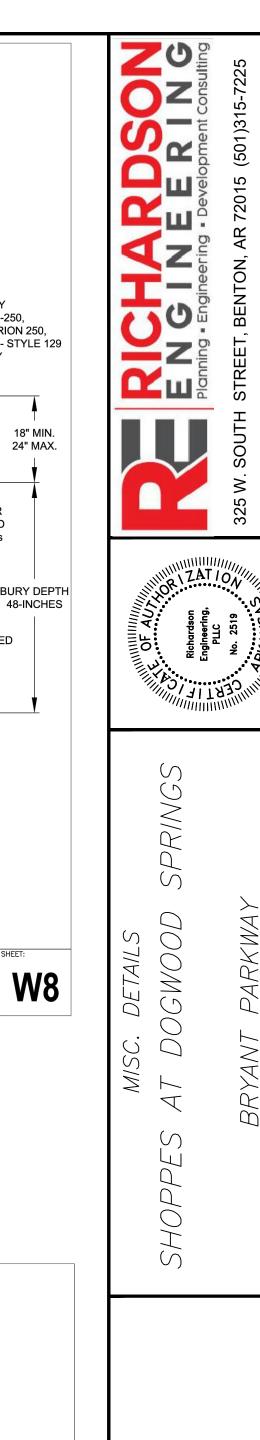


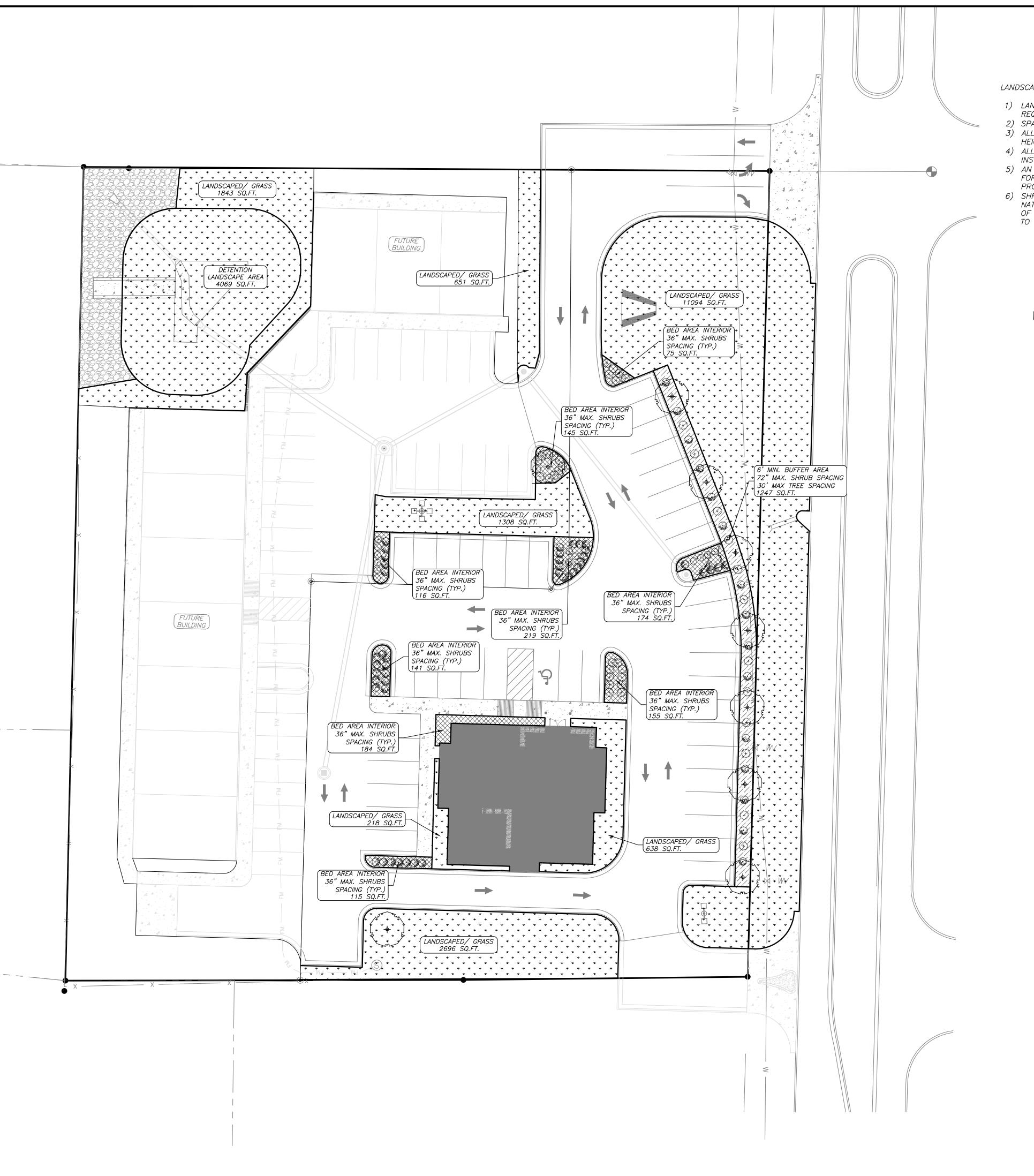










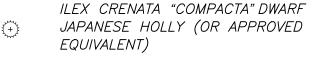


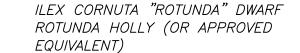
LANDSCAPE NOTES:

- 1) LANDSCAPING SHALL MEET CITY OF BRYANT LANDSCAPING
- REQUIREMENTS. 2) SPACING FOR SHRUBS IN BEDS AS NOTED.
- 3) ALL SHRUB MATERIAL MUST BE EIGHTEEN (18) INCHES IN HEIGHT AT INSTALLATION
- 4) ALL TREES MUST BE 2" CALIPER AT THE TIME OF INSTALLATION
- 5) AN AUTOMATIC IRRIGATION SYSTEM SHALL BE PROVIDED FOR ALL NEW LANDSCAPE AREAS. THIS SYSTEM WILL PROVIDE 100% COVERAGE FOR LANDSCAPED AREAS.
- 6) SHRUBS USED FOR SCREENING SHALL BE EVERGREEN IN NATURE AND BE AT LEAST 30 INCHES TALL AT THE TIME OF PLANTING. SPACING SHALL BE CLOSE ENOUGH SO AS TO CREATE A SEAMLESS ROW OF HEDGING.



ACER PALMATUM— JAPANESE MAPLE (OR APPROVED EQUIVALENT)







HYDRANGEA ARBORESCENS 'ANNABELLE' / ANNABELLE HYDRANGEA (OR APPROVED *EQUIVALENT)*



ACER SACCHARINUM - SILVER MAPLE (OR APPROVED *EQUIVALENT)*



PROPOSED BERMUDA



PROPOSED MULCH/BED AREA / INTERIOR LANDSCAPING



PROPOSED BUFFER AREAS

GRAPHIC SCALE

(IN FEET) 1 inch = 20 ft.

<u>UTILITIES:</u>

SANITARY SEWER: BRYANT WASTEWATER DEPARTMENT 210 SW 3RD ST. BRYANT, AR 72022 (501) 943-0999

BRYANT WATER DEPARTMENT 210 SW 3RD ST. BRYANT, AR 72022 (501) 943-0999

ENTERGY SIMMONS TOWER 425 W. CAPITOL AVE. LITTLE ROCK, AR 72201 1(800) 368–3749

NATURAL GAS: 400 WEST CAPITOL #600 LITTLE ROCK, ARKAŃSAS 888-498-0409

RICHARDSON ENGINEERING, PLLC ADDRESS: 325 W.SOUTH ST. BENTON, AR. 72015 PHONE NO. (501) 315-7225 PROJECT REPRESENTATIVE: TRISTIN PHILLIPS, P.E.

SOUTHPOINT SURVEYING P.O. BOX 400 SHERIDAN, AR 72150 (501) 285-5958

DEVELOPERS JON MARTIN

OWNER OF RECORD PROSPER PROPERTIES LLC 612 W. COMMERCE ST, STE 2 BRYANT, AR 72022 *SOURCE OF TITLE: 2020-020202*

<u>GENERAL NOTES:</u>

- 1.) SURVEYING PROVIDED BY SOUTHPOINT SURVEYING LLC. 2.) PAVEMENT SHALL BE CONSTRUCTED IN ACCORDANCE WITH THIS PLAN
- AND BASED ON GEOTECHNICAL ANALYSIS OF THE SOIL CONDITIONS. 3.) ATTENTION IS CALLED TO CONSTRUCTION DETAILS FOR ADDITIONAL INFORMATION.
- 4.) CONSTRUCTION SITE SHALL ADHERE TO BRYANT STORMWATER REQUIREMENTS, AND SHALL MEET ALL APPLICABLE ADEQ STANDARDS FOR EROSION CONTROL MEASURES.
- 5.) ALL UTILITIES TO BE LOCATED PRIOR TO CONSTRUCTION (ONE CALL, CITY, ETC.) 6.) CONTRACTOR TO ADHERE TO CURRENT OSHA REGULATIONS INCLUDING EXCAVATION &
- TRENCH SAFETY. 7.) THE APPROXIMATE LOCATION OF KNOWN SURFACE AND SUBSURFACE STRUCTURES, PIPES, POWER, GAS, PHONE, ETC. ARE SHOWN ON THE DESIGN DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE EXACT LOCATION OF THE AFOREMENTIONED ITEMS, SHOWN AND NOT SHOWN.

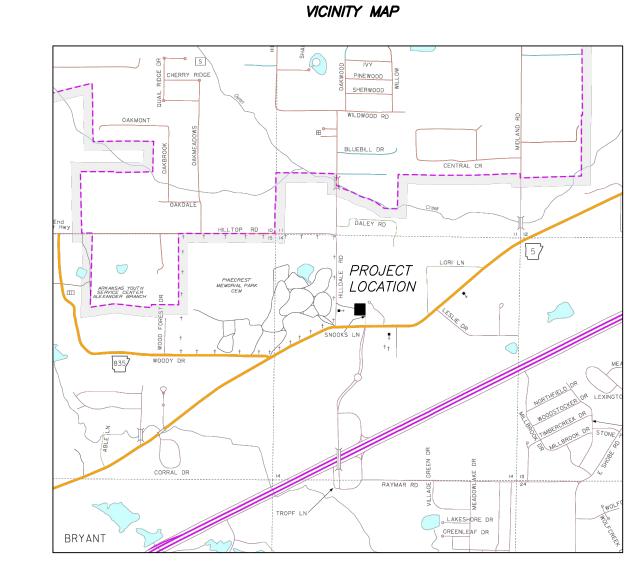
SITE NOTES

1.) PROJECT SITE AREA 1.925 ACRES (83,899 SQUARE FEET)

- 2.) PROJECT PROVIDES 42 PARKING SPACES. 3.) CURRENT ZONING: C2
- 4.) PROPOSED USE: PHARMACY
- 5.) BUILDING LINES TO BE NOTED ON SITE PLAN.
- 6.) PROPOSED LOCATIONS OF TRAFFIC CONTROL MARKERS ARE APPROXIMATE. ACTUAL LOCATION AND INSTALLATION MUST MEET MUTCD AND BRYANT ROAD
- 7.) CONTRACTOR SHALL INCLUDE IN BID THE COST FOR COMPACTION TESTS ON SUBGRADE & BASE. TEST TO BE CONDUCTED AS PER GEOTECHNICAL
- ENGINEERS **SPEC**S. 8.) CONTRACTOR TO INCLUDE IN BID THE COST OF MATERIAL AND INSTALLATION OF STREET SIGNS & TRAFFIC CONTROL SIGNS.
- 9.) CONTRACTOR TO ADHERE TO CURRENT OSHA REGULATIONS INCLUDING EXCAVATION
- & TRENCH SAFETY. 10.) NO PORTION OF THIS PROPERTY (AS SHOWN) IS IN THE 100 YEAR FLOOD HAZARD
- AREA. REFERENCE FIRM MAP 05125C0240E EFFECTIVE 6/5/2020 11.) REFER TO LANDSCAPING PLAN FOR REQUIREMENTS/LIGHTING PLAN TO BE VERIFIED W/OWNER. REVISIONS TO PLAN TO BE SUBMITTED TO CITY. 12.) IRRIGATION SYSTEMS TO BE DESIGNED BY OTHERS.
- 13.) THERE ARE NO EROSION PROBLEMS ON THE SITE OR KNOWN EROSION PROBLEMS WITHIN 300' DOWNSTREAM.
- 14.) THERE IS NO SURFACE EVIDENCE OF EXISTING OR ABANDONED WATER WELLS, SUMPS, CESSPOOLS, SPRING WATER IMPOUNDMENTS, AND UNDERGROUND STRUCTURES WITHIN THE PROJECT.
- 15.) PAVEMENT SHALL BE CONSTRUCTED IN ACCORDANCE WITH AHTD SPECIFICATIONS, AND BASED ON GEOTECHNICAL ANALYSIS OF THE SOIL CONDITIONS. CONTRACTOR SHALL INCLUDE IN BID THE COST FOR COMPACTION TESTS ON SUBGRADE & BASE. TEST TO BE CONDUCTED AS PER GEOTECHNICAL SPECS.

16.) ALL SITE LIGHTING IS TO BE LOW LEVEL AND DIRECTIONAL, SHIELDED DOWNWARD

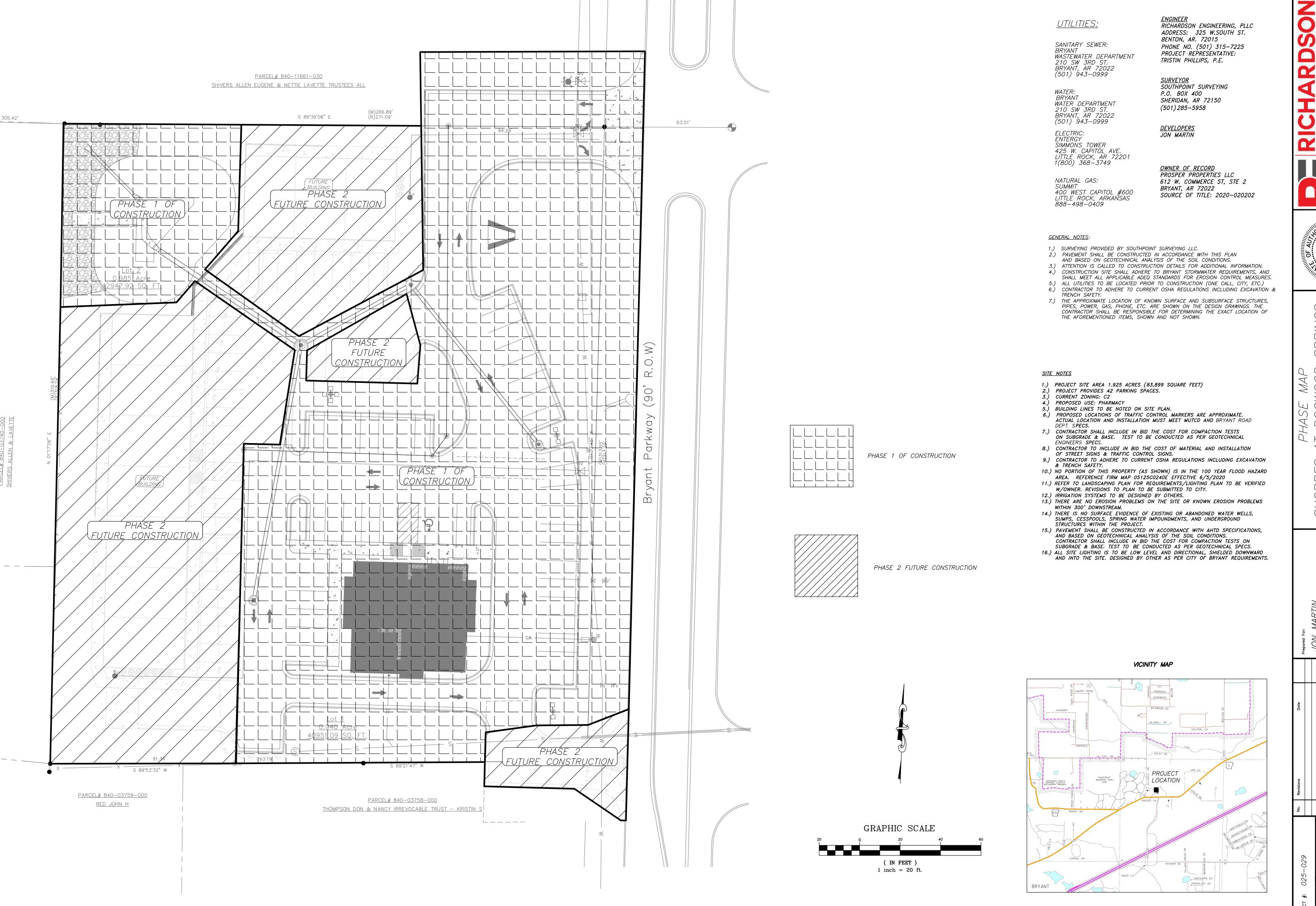
AND INTO THE SITE. DESIGNED BY OTHER AS PER CITY OF BRYANT REQUIREMENTS.



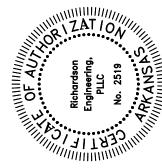
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PRING

Drainage Report

For

Bryant Pharmacy

Bryant, Saline County, Arkansas



September 5, 2025

Prepared by:

RICHARDSON ENGINEERING, PLLC

325 W. South St. Benton, AR 72015 501-315-7225

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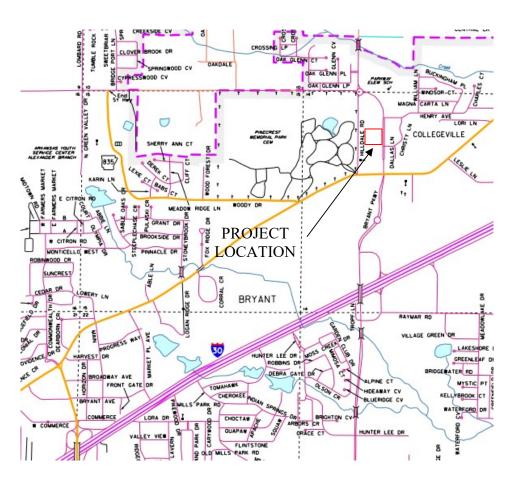
Title	Page Number
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Recommendations/Summary	6
Appendices:	7
Runoff Coefficient Calculation	8
NRCS Soil Report	9
Site Drainage Basin Maps	10
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2 Year Design Storm	12
10 Year Design Storm	13
25 Year Design Storm	14
50 Year Design Storm	15
100 Year Design Storm	16
Pre and Post Development Hydrographs (Hydrology	17
Studio)	

Project Owner Information

Jon Martin 5501 Lombard Road Alexander, AR 72002

Project Location and Description

The project is located on West side of the Bryant Parkway, part of the Southeast Quater of the Northwest Quarter, Section 14, Township 1-S, Range 14-W, Saline County, Arkansas.



Vicinity Map – N.T.S

This project is a proposed Commercial Development, located in the City of Bryant, Saline County.

Site Drainage

Pre-Development

The pre-developed runoff for the site flows to the East and West. The pre-development runoff condition is undeveloped/woods.

Post-Development

The site drainage starts on the South side of the project and flows to the North. The drainage is sheet flows across the proposed parking lot and intercepted by the proposed storm sewer system and is discharged into a proposed detention basin on the Northwest corner of the project. The proposed detention basin will utilize a culvert/weir discharge structure. Post-Development Basin A is the drainage basin that discharges water into the proposed detention basin and Post-Development Basin B and C are the grass tie back slopes from the proposed pavement to existing grade. These areas are not routed through the detention basin, so it was calculated by itself. The post-development runoff conditions changed from undeveloped/woods to commercial development.

Runoff Summary

Overall Basin Design Point

Development Drainage Study Area = 1.92 Ac

Post-Development Drainage Study Area = 1.97

Existing Condition runoff Coefficient: C = 0.56

Proposed runoff Coefficient: C = 0.95/0.58/0.51

Tc Undeveloped = 9/12 Minutes (TR55 Method)

Tc Developed = 5/10 Minutes

Detention Basin Required Volume: 7,811 CF

Detention Basin Volume: 12,635 CF

Maximum Storage: 1,163 CF Discharge Structure: Culvert/Weir

Pre-Development Flow Rate (cfs)	Post-Development Flow Rate (cfs)	Post-Development w/ Detention Flow Rate (cfs)
4.20	2.00	9.82
		13.13
		15.13
		15.08
8.02	6.79	17.91
	Flow Rate (cfs) 4.39 5.87 6.75 7.38	Flow Rate (cfs) Flow Rate (cfs) 4.39 3.90 5.87 5.25 6.75 5.77 7.38 6.26

Pre- Development Basin "A" Design Point

Drainage Study Area = 1.17 Ac Existing runoff Coefficient: C = 0.56 Tc Undeveloped = 9 Minutes

Design Storm	Post-Development Flow Rate (cfs)	
2-yr	3.08	
10-yr	4.13	
25-yr	4.75	
50-yr	5.19	
100-yr	5.64	

Pre- Development Basin "B" Design Point

Drainage Study Area = 0.75 Ac

Existing runoff Coefficient: C = 0.56

Tc Undeveloped = 12 Minutes

Design Storm	Post-Development Flow Rate (cfs)	
2	1.74	
2-yr	1.74	
10-yr	2.33	
25-yr	2.67	
50-yr	2.92	
100-yr	3.18	

Post- Development Basin "A" Design Point

Drainage Study Area = 1.50 Ac
Proposed runoff Coefficient: C = 0.95

Tc Developed = 5 Minutes

Design Storm	Post-Development Flow Rate (cfs)	
2-yr	8.75	
10-yr	11.70	
25-yr	13.44	
50-yr	14.68	
100-yr	15.96	

Post- Development Basin "B" Design Point Drainage Study Area = 0.22 Ac

Proposed runoff Coefficient: C = 0.58

Tc Developed = 10 Minutes

Design Storm	Post-Development Flow Rate (cfs)	
2-yr	0.57	
10-yr	0.77	
25-yr	0.88	
50-yr	0.96	
100-yr	1.05	

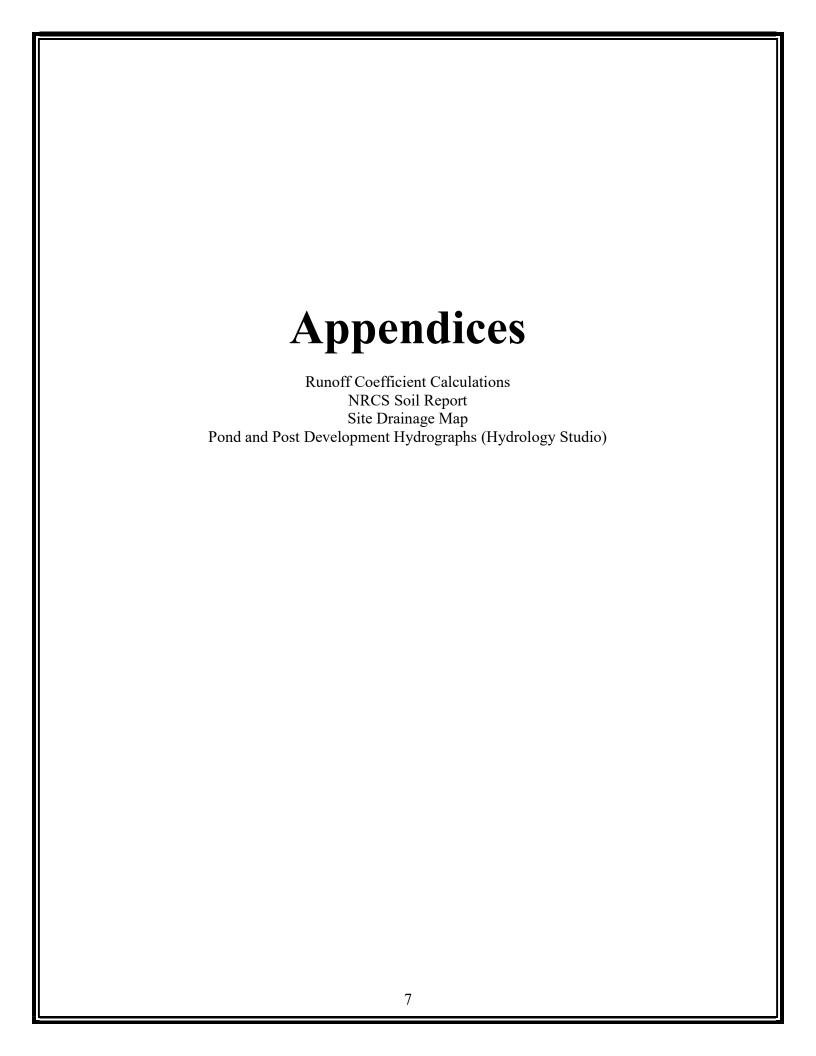
Post- Development Basin "C" Design Point Development Drainage Study Area = 0.25 Ac Proposed runoff Coefficient: C = 0.51

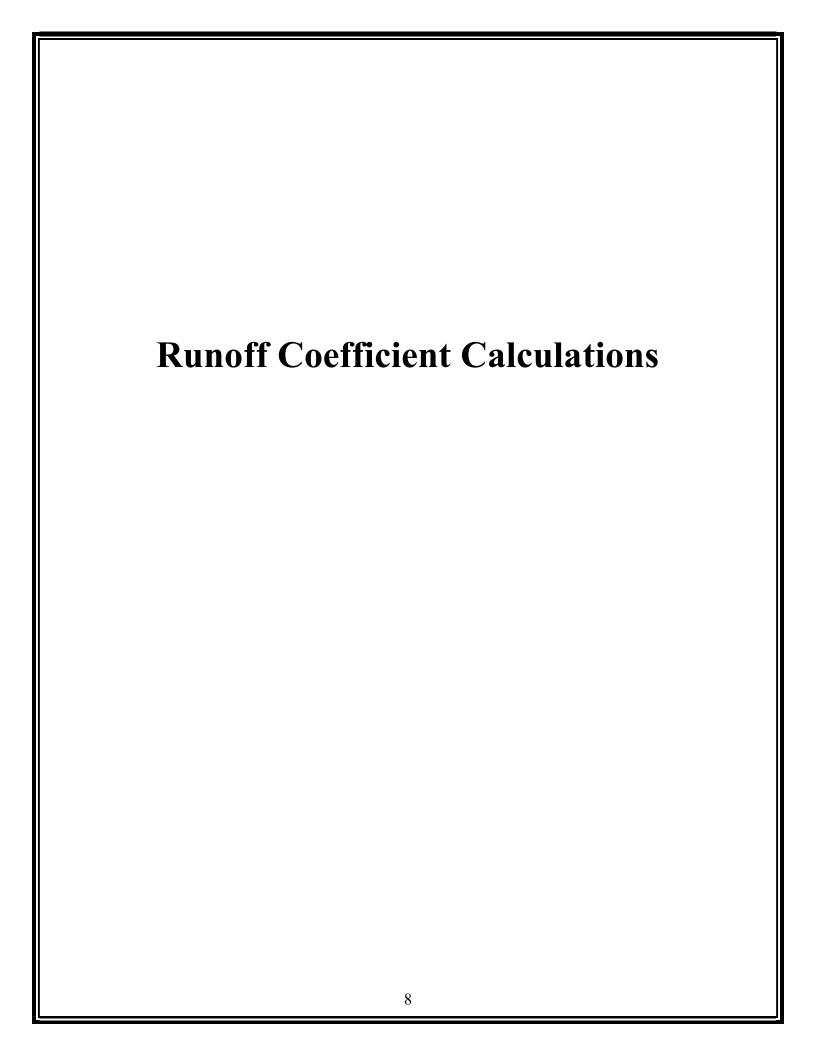
Tc Developed = 5 Minutes

Design Storm	Post-Development Flow Rate (cfs)	
2-yr	0.78	
10-yr	1.05	
25-yr	1.20	
50-yr	1.31	
100-yr	1.43	

Recommendations/Summary

The proposed drainage improvements include a storm sewer system and a detention basin on the Northwest corner of the project. The proposed detention basin releases the post development runoff at a lower rate than the pre-development condition.







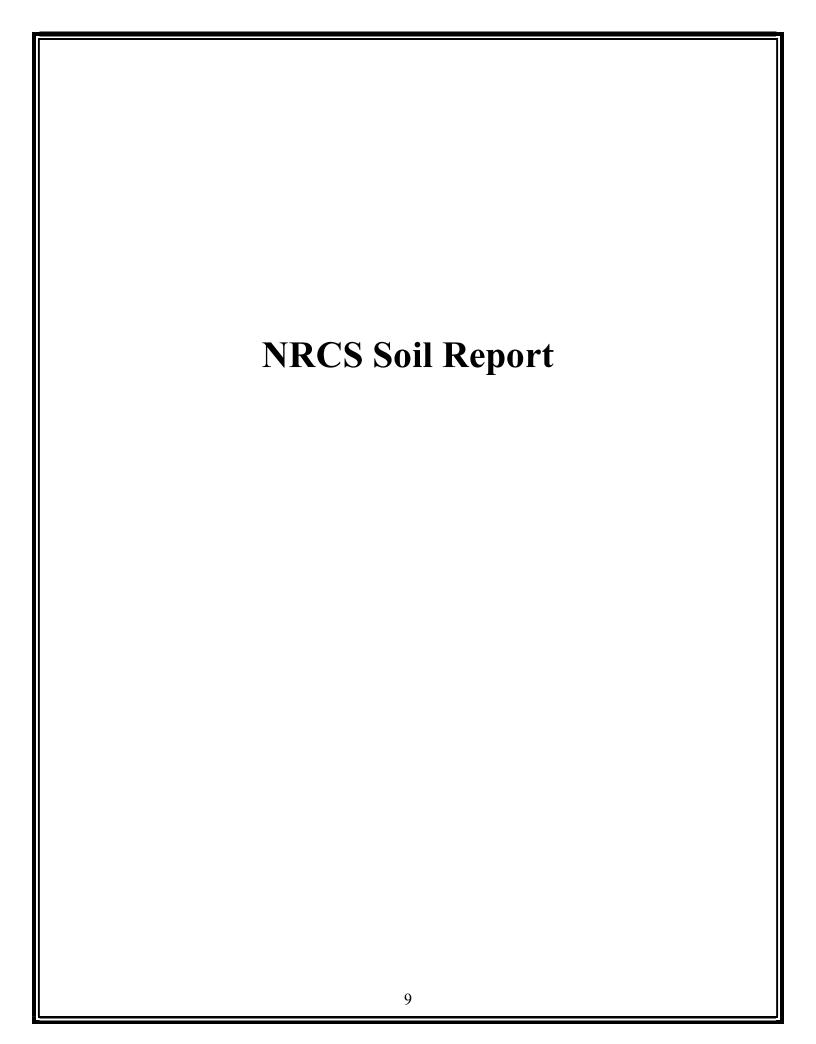
325 West South Street Benton, AR 72015 (501) 315-7225

(1/2) PROJECT 025 - DZ9 DRAINAGE ANALYSIS DATE 09/04/2025 PRE-DEVELOPMENT DRAWAGE AREA: 1.92 AC CLAY SOIL 50% - 2%-7% 6:0.50 6= (0.960)(0.5) + (0.960)(0.62) 1.92 = 0.56



325 West South Street Benton, AR 72015 (501) 315-7225 (2/2)

PROJECI_	025-029	DRAIN ALOE	ANALYSIS	DATE 09/04/2025
	P087 - DE	VELOPMENT		
BA	5/A A			
A.	REA= 1.5	Ac		
	E 0,75			
BA	5/2 ' 3 "			
An	LEA = 0,22	AE (966)	I SEV	
PE	2 VIDUS : 80	192 or C	= 0.51 /62753 = 6	LOTHUAD GOOD
MP	PERVIOUS: 151	69 3 8 2	= 0.81 (GA483 = 6.	
	<i>C</i> = (8092)(0.5	1) + (184) (6.95)	E 0.58
			966)	
3450	v 2 -			
ALE	A = 0, 25	Ac (11,08	18 SF)	
PEN	V1005 ; 6,7	92 55 6	C= 0.81	
)M ? 6	ELVIOUS & 4,2	96 54 (3 0.95	
G	= (6792	2)(0.51) +	(4296)(0.78) =	0,68
		11087	&	





MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons



Soil Map Unit Points

Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

... Gravelly Spot

Candfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

→ Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

Spoil Area

Stony Spot

Wery Stony Spot

Wet Spot
 Other
 Othe

Special Line Features

Water Features

Δ

Streams and Canals

Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Saline County, Arkansas Survey Area Data: Version 21, Sep 10, 2024

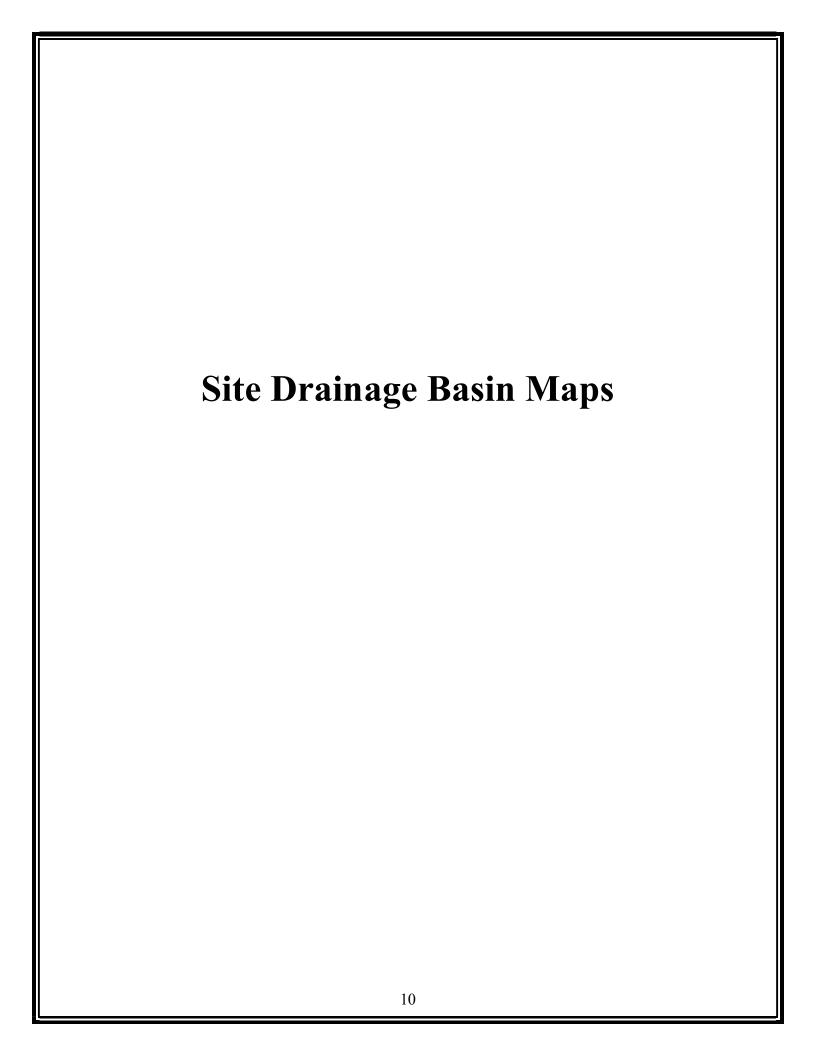
Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: May 1, 2022—May 29, 2022

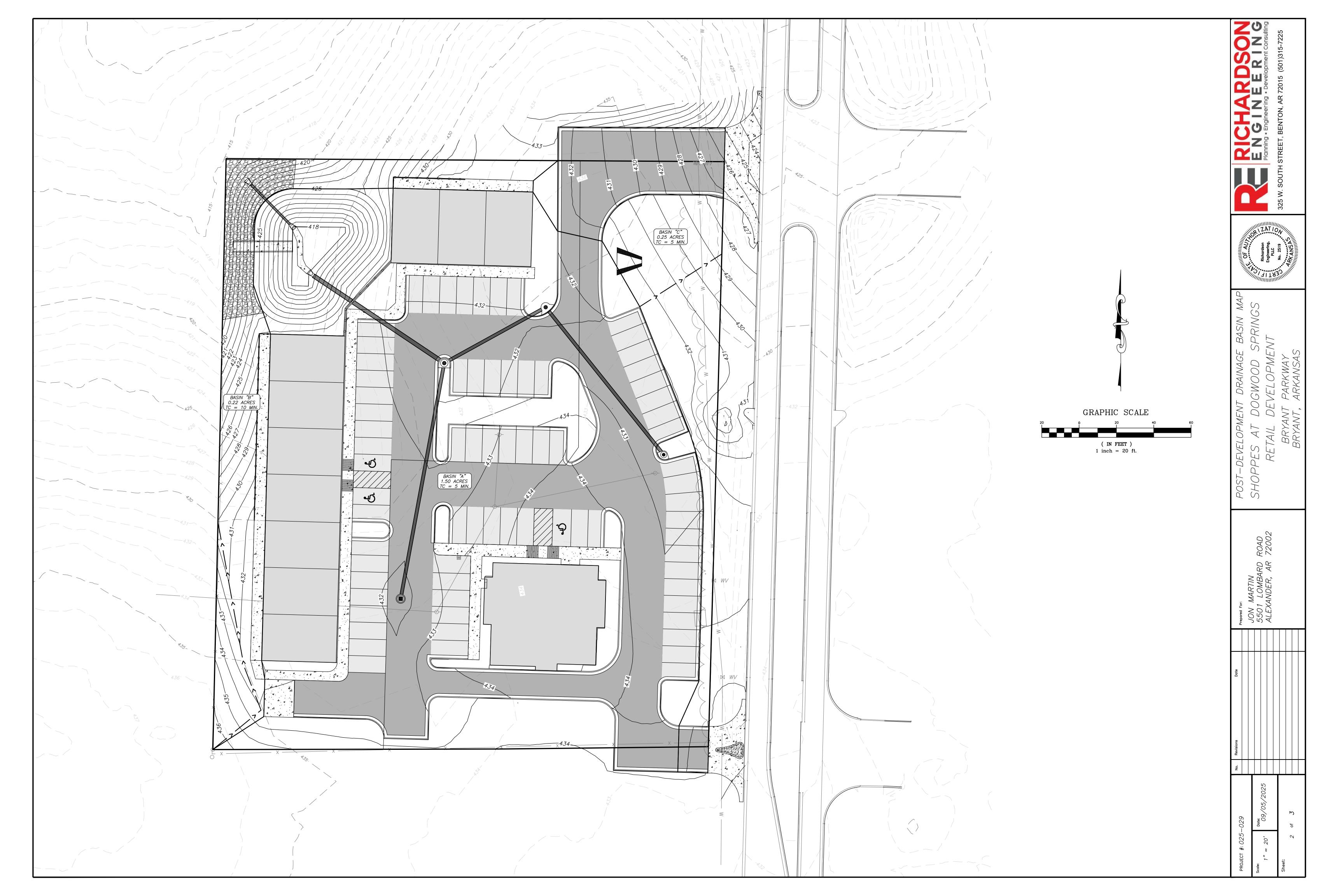
The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

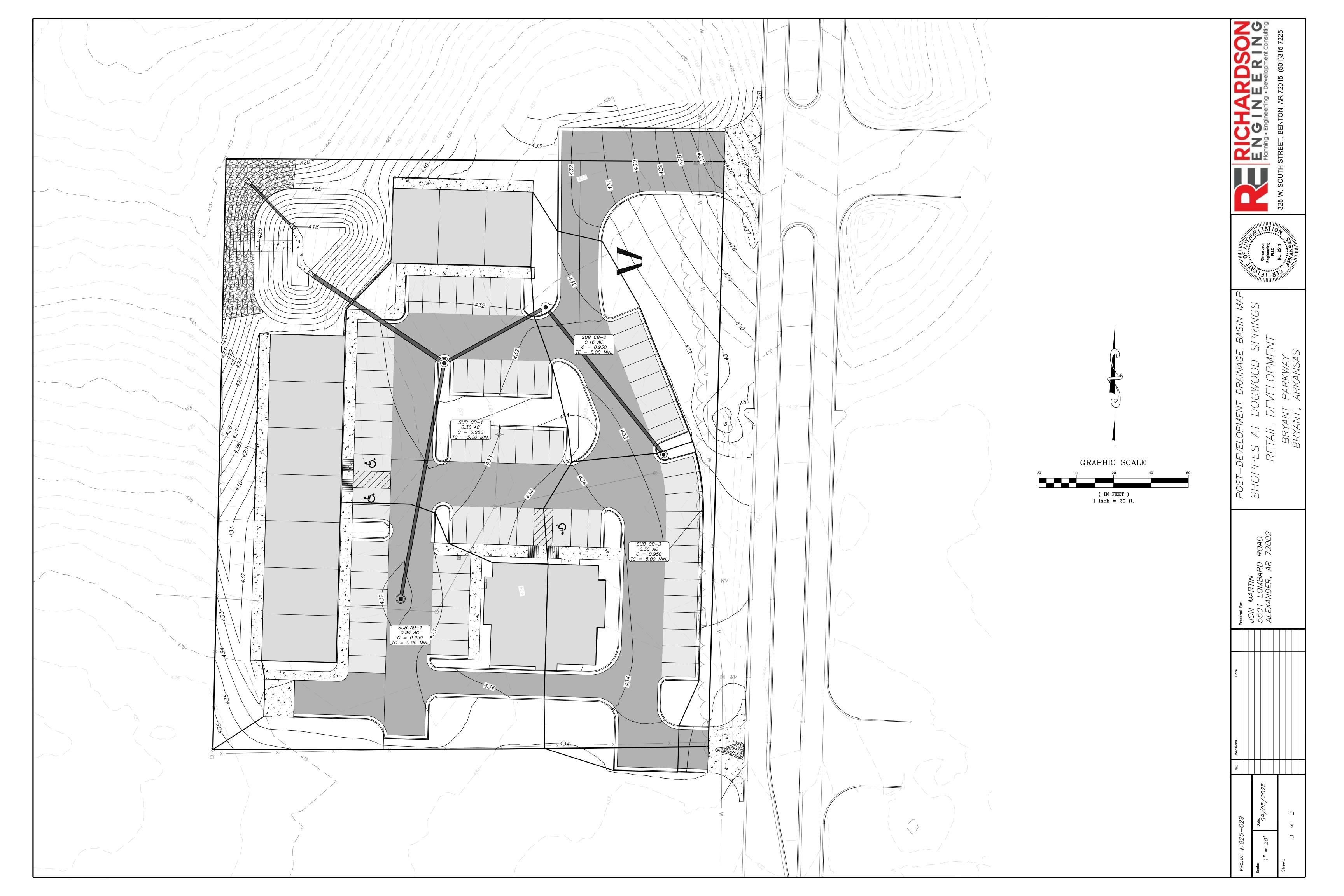
Map Unit Legend

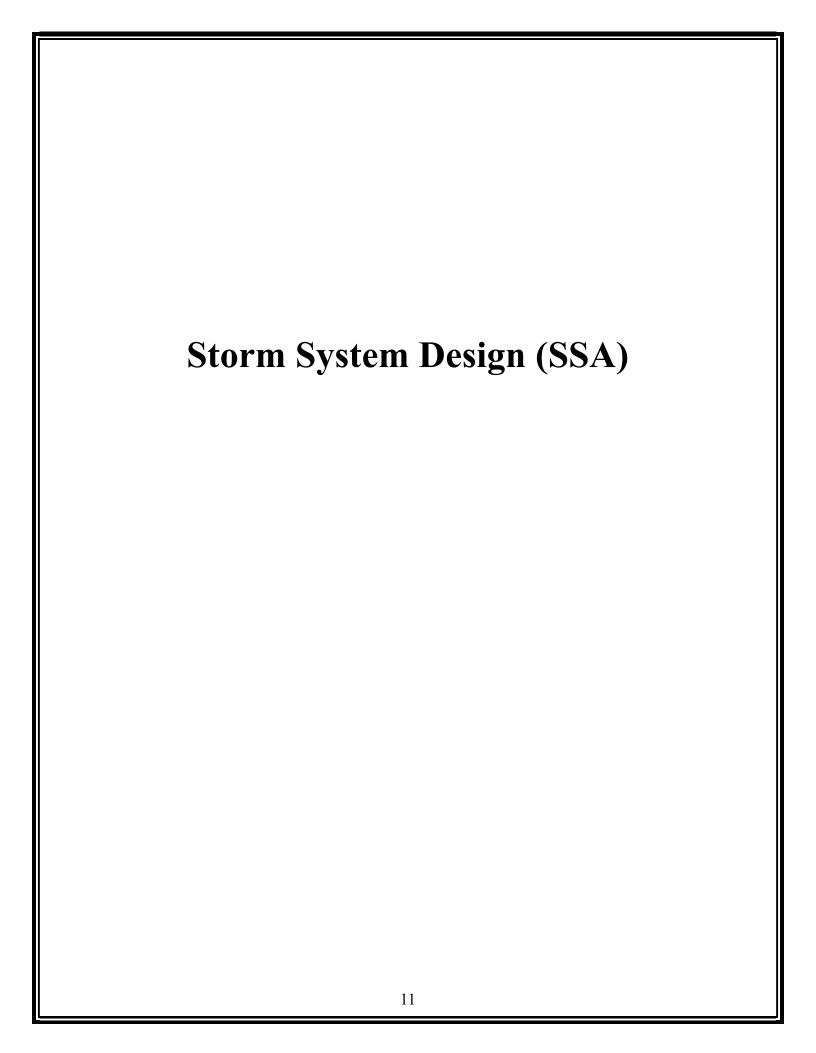
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI	
29	Tiak silt loam, 3 to 8 percent slopes	2.4	100.0%	
Totals for Area of Interest		2.4	100.0%	

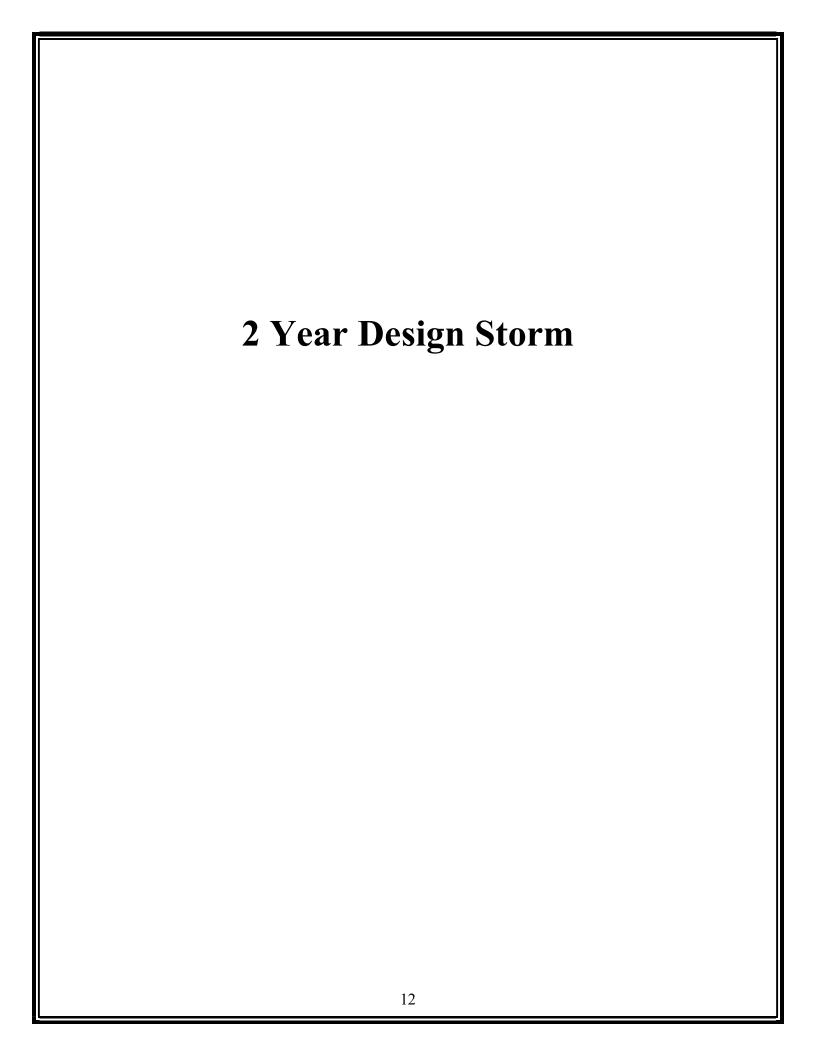












Project Description

File Name Bryant Pharmacy Drainage Analysis 9-5-25.SPF

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	Rational
Time of Concentration (TOC) Method	User-Defined
Link Routing Method	Kinematic Wave
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	NO

Analysis Options

Start Analysis On	Sep 05, 2025	00:00:00
End Analysis On	Sep 06, 2025	00:00:00
Start Reporting On	Sep 05, 2025	00:00:00
Antecedent Dry Days	0	days
Runoff (Dry Weather) Time Step		days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	30	seconds

Number of Elements

Q	≀ty
Rain Gages 0	
Subbasins	
Nodes	
Junctions 0	
Outfalls 1	
Flow Diversions 0	
Inlets 4	
Storage Nodes 0	
Links	
Channels 0	
<i>Pipes</i> 5	
Pumps 0	
Orifices 0	
Weirs 0	
Outlets 0	
Pollutants 0	
Land Uses 0	

Rainfall Details

Return Period...... 2 year(s)

Subbasin Summary

SN Subbasin	Area	Weighted	Total	Total	Total	Peak	Time of
ID		Runoff	Rainfall	Runoff	Runoff	Runoff	Concentration
		Coefficient			Volume		
	(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1 Sub-AD-4	0.35	0.9500	0.51	0.49	0.17	2.02	0 00:05:00
2 Sub-CB-1	0.36	0.9500	0.51	0.49	0.17	2.09	0 00:05:00
3 Sub-CB-2	0.16	0.9500	0.51	0.49	0.08	0.91	0 00:05:00
4 Sub-CB-3	0.30	0.9500	0.51	0.49	0.15	1.77	0 00:05:00

Node Summary

SN Element	Element	Invert	Ground/Rim	Initial	Surcharge	Ponded	Peak	Max HGL	Max	
ID	Type	Elevation	(Max)	Water	Elevation	Area	Inflow	Elevation	Surcharge	F
			Elevation	Elevation				Attained	Depth	
									Attained	
		(ft)	(ft)	(ft)	(ft)	(ft ²)	(cfs)	(ft)	(ft)	
1 Out-1SL - (14)	Outfall	418.00					6.67	418.53		_

Link Summary

SN Eler ID	ment	Element Type		To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Slope	Diameter or Height	Manning's Roughness		•	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth
					(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)
1 L-SI	L - (16)	Pipe	CB-3	CB-2	112.54	433.11	432.25	0.7600	0.000	0.0150	0.07	0.00	0.00	0.00	0.00
2 SL -	- (14)	Pipe	CB-1	Out-1SL - (14)	84.31	422.00	418.00	4.7400	18.000	0.0120	6.67	24.79	0.27	11.93	0.53
3 SL -	- (15)	Pipe	CB-2	CB-1	62.02	426.00	422.00	6.4500	18.000	0.0120	2.65	28.90	0.09	10.23	0.31
4 SL -	- (16)	Pipe	CB-3	CB-2	101.17	429.00	426.00	2.9700	18.000	0.0120	1.69	19.60	0.09	9.23	0.30
5 SL -	- (18)	Pipe	AD-4	CB-1	128.37	427.50	423.00	3.5100	18.000	0.0120	1.99	21.31	0.09	8.25	0.31

Inlet Summary

SN Element	Inlet	Number of	Catchbasin	Max (Rim)	Initial	Ponded	Peak	Peak Flow	Peak Flow	Inlet	Allowable	Max Gutter	Max Gutter
ID	Location	Inlets	Invert	Elevation	Water	Area	Flow	Intercepted	Bypassing	Efficiency	Spread	Spread	Water Elev.
			Elevation		Elevation			by	Inlet	during Peak		during Peak	during Peak
								Inlet		Flow		Flow	Flow
			(ft)	(ft)	(ft)	(ft ²)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
1 AD-4	On Sag	1	427.50	431.80	427.50	10.00	2.02	N/A	N/A	N/A	10.00	8.74	432.16
2 CB-1	On Sag	1	422.00	431.61	422.00	10.00	2.09	N/A	N/A	N/A	10.00	5.00	432.01
3 CB-2	On Sag	1	426.00	432.25	426.00	10.00	0.98	N/A	N/A	N/A	10.00	3.01	432.59
4 CB-3	On Grade	1	429.00	433.11	429.00	N/A	1.77	1.72	0.06	96.77	10.00	6.06	433.28

Subbasin Hydrology

Subbasin : Sub-AD-4

Pipe Input

SN Element	Length	Inlet	Inlet	Outlet	Outlet	Total	Average	Pipe	Pipe	Pipe	Man
ID		Invert	Invert	Invert	Invert	Drop	Slope	Shape	Diameter or	Width	Rougl
		Elevation	Offset	Elevation	Offset				Height		
	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(%)		(in)	(in)	
1 L-SL - (16)	112.54	433.11	4.11	432.25	6.25	0.86	0.7600	Dummy	0.000	0.000	0
2 SL - (14)	84.31	422.00	0.00	418.00	0.00	4.00	4.7400	CIRCULAR	18.000	18.000	0
3 SL - (15)	62.02	426.00	0.00	422.00	0.00	4.00	6.4500	CIRCULAR	18.000	18.000	0
4 SL - (16)	101.17	429.00	0.00	426.00	0.00	3.00	2.9700	CIRCULAR	18.000	18.000	0
5 SL - (18)	128.37	427.50	0.00	423.00	1.00	4.50	3.5100	CIRCULAR	18.000	18.000	0

No. of Barrels

1

1

Pipe Results

SN Element	Peak	Time of	Design Flow	Peak Flow/	Peak Flow	Travel	Peak Flow	Peak Flow	Tota
ID	Flow	Peak Flow	Capacity	Design Flow	Velocity	Time	Depth	Depth/	Surch
		Occurrence		Ratio				Total Depth	
								Ratio	
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		
1 L-SL - (16)	0.07	0 00:05	0.00	0.00	0.00		0.00	0.00	
2 SL - (14)	6.67	0 00:05	24.79	0.27	11.93	0.12	0.53	0.35	
3 SL - (15)	2.65	0 00:05	28.90	0.09	10.23	0.10	0.31	0.20	
4 SL - (16)	1.69	0 00:05	19.60	0.09	9.23	0.18	0.30	0.20	
5 SL - (18)	1.99	0 00:05	21.31	0.09	8.25	0.26	0.31	0.21	

Inlet Input

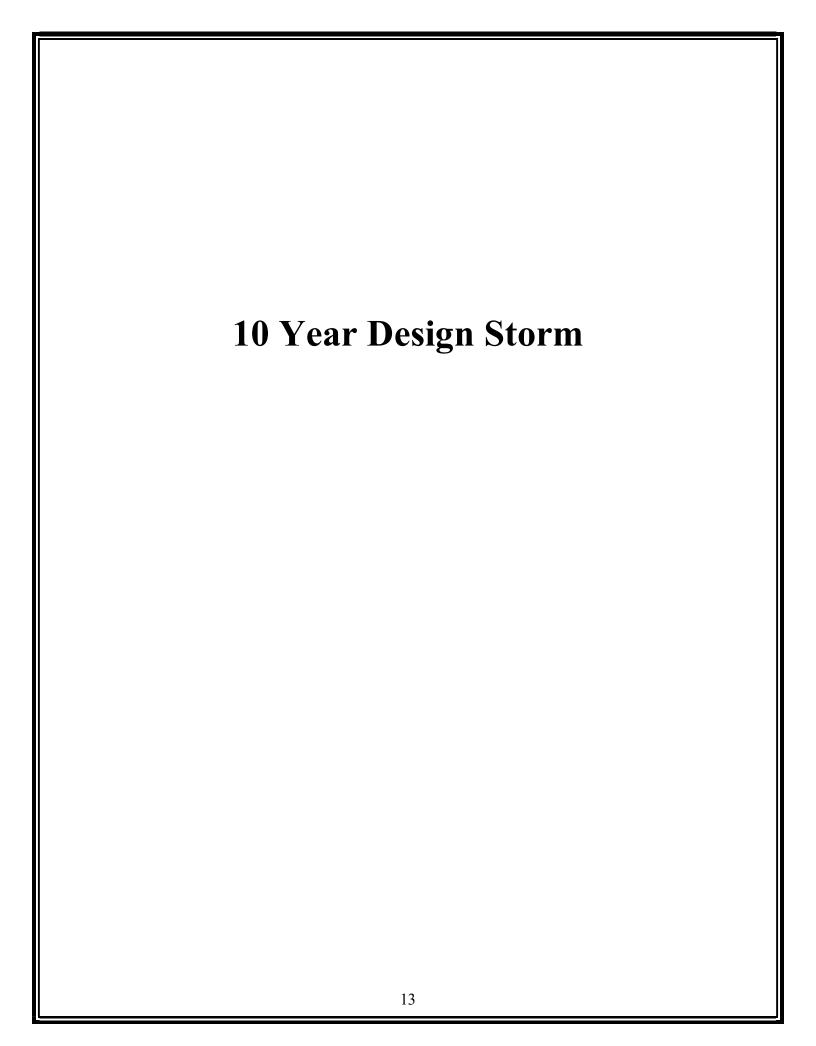
SI	N Element	Inlet	Number of	Catchbasin	Max (Rim)	Inlet	Initial	Initial	Ponded	Grate
	ID	Location	Inlets	Invert	Elevation	Depth	Water	Water	Area	Clogging
				Elevation			Elevation	Depth		Factor
				(ft)	(ft)	(ft)	(ft)	(ft)	(ft ²)	(%)
	1 AD-4	On Sag	1	427.50	431.80	4.30	427.50	0.00	10.00	0.00
	2 CB-1	On Sag	1	422.00	431.61	9.61	422.00	0.00	10.00	0.00
	3 CB-2	On Sag	1	426.00	432.25	6.25	426.00	0.00	10.00	0.00
	4 CB-3	On Grade	1	429.00	433.11	4.11	429.00	0.00	N/A	0.00

Roadway & Gutter Input

SN Element	Roadway	Roadway	Roadway	Gutter	Gutter	Gutter	Allowable
ID	Longitudinal	Cross	Manning's	Cross	Width	Depression	Spread
	Slope	Slope	Roughness	Slope			
	(ft/ft)	(ft/ft)		(ft/ft)	(ft)	(in)	(ft)
1 AD-4	N/A	0.0300	0.0150	0.0300	1.50	0.1312	10.00
2 CB-1	N/A	0.0300	0.0150	0.0200	1.50	0.1312	10.00
3 CB-2	N/A	0.0300	0.0150	0.0200	1.50	0.1312	10.00
4 CB-3	0.0200	0.0300	0.0150	0.0200	1.50	0.1312	10.00

Inlet Results

SN Element	Peak	Peak	Peak Flow	Peak Flow	Inlet	Max Gutter	Max Gutter	Max Gutter	7
ID	Flow	Lateral	Intercepted	Bypassing	Efficiency	Spread	Water Elev.	Water Depth	Max
		Inflow	by	Inlet	during Peak	during Peak	during Peak	during Peak	Occı
			Inlet		Flow	Flow	Flow	Flow	
	(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)	(days h
1 AD-4	2.02	2.02	N/A	N/A	N/A	8.74	432.16	0.36	(
2 CB-1	2.09	2.09	N/A	N/A	N/A	5.00	432.01	0.40	(
3 CB-2	0.98	0.91	N/A	N/A	N/A	3.01	432.59	0.34	(
4 CB-3	1.77	1.77	1.72	0.06	96.77	6.06	433.28	0.17	(



Project Description

File Name Bryant Pharmacy Drainage Analysis 9-5-25.SPF

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	Rational
Time of Concentration (TOC) Method	User-Defined
Link Routing Method	Kinematic Wave
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	NO

Analysis Options

Start Analysis On		00:00:00
End Analysis On	Sep 06, 2025	00:00:00
Start Reporting On	Sep 05, 2025	00:00:00
Antecedent Dry Days		days
Runoff (Dry Weather) Time Step	0 01:00:00	days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	30	seconds

Number of Elements

	Qty
Rain Gages	0
Subbasins	4
Nodes	5
Junctions	0
Outfalls	1
Flow Diversions	0
Inlets	4
Storage Nodes	0
Links	5
Channels	0
Pipes	5
Pumps	0
Orifices	0
Weirs	0
Outlets	0
Pollutants	0
Land Uses	0

Rainfall Details

Subbasin Summary

SN Subbasin	Area	Weighted	Total	Total	Total	Peak	Time of
ID		Runoff	Rainfall	Runoff	Runoff	Runoff	Concentration
		Coefficient			Volume		
	(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1 Sub-AD-4	0.35	0.9500	0.68	0.65	0.22	2.70	0 00:05:00
2 Sub-CB-1	0.36	0.9500	0.68	0.65	0.23	2.80	0 00:05:00
3 Sub-CB-2	0.16	0.9500	0.68	0.65	0.10	1.22	0 00:05:00
4 Sub-CB-3	0.30	0.9500	0.68	0.65	0.20	2.37	0 00:05:00

Node Summary

SN Element	Element	Invert	Ground/Rim	Initial	Surcharge	Ponded	Peak	Max HGL	Max	
ID	Type	Elevation	(Max)	Water	Elevation	Area	Inflow	Elevation	Surcharge	F
			Elevation	Elevation				Attained	Depth	
									Attained	
		(ft)	(ft)	(ft)	(ft)	(ft ²)	(cfs)	(ft)	(ft)	
1 Out-1SL - (14)	Outfall	418.00					8.93	418.62		

Link Summary

_	Element ID	Element Type		To (Outlet) Node	Length	Inlet Invert Elevation	Invert	Slope	Diameter or Height	Manning's Roughness		Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth
					(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)
1	L-SL - (16)	Pipe	CB-3	CB-2	112.54	433.11	432.25	0.7600	0.000	0.0150	0.24	0.00	0.00	0.00	0.00
2	SL - (14)	Pipe	CB-1	Out-1SL - (14)	84.31	422.00	418.00	4.7400	18.000	0.0120	8.93	24.79	0.36	12.91	0.62
3	SL - (15)	Pipe	CB-2	CB-1	62.02	426.00	422.00	6.4500	18.000	0.0120	3.54	28.90	0.12	11.11	0.35
4	SL - (16)	Pipe	CB-3	CB-2	101.17	429.00	426.00	2.9700	18.000	0.0120	2.12	19.60	0.11	9.87	0.33
5	SL - (18)	Pipe	AD-4	CB-1	128.37	427.50	423.00	3.5100	18.000	0.0120	2.66	21.31	0.12	8.42	0.36

Inlet Summary

SN Element	Inlet	Number of	Catchbasin	Max (Rim)	Initial	Ponded	Peak	Peak Flow	Peak Flow	Inlet	Allowable	Max Gutter	Max Gutter
ID	Location	Inlets	Invert	Elevation	Water	Area	Flow	Intercepted	Bypassing	Efficiency	Spread	Spread	Water Elev.
			Elevation		Elevation			by	Inlet	during Peak		during Peak	during Peak
								Inlet		Flow		Flow	Flow
			(ft)	(ft)	(ft)	(ft ²)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
1 AD-4	On Sag	1	427.50	431.80	427.50	10.00	2.70	N/A	N/A	N/A	10.00	10.40	432.21
2 CB-1	On Sag	1	422.00	431.61	422.00	10.00	2.80	N/A	N/A	N/A	10.00	6.07	432.04
3 CB-2	On Sag	1	426.00	432.25	426.00	10.00	1.46	N/A	N/A	N/A	10.00	3.92	432.62
4 CB-3	On Grade	1	429.00	433.11	429.00	N/A	2.37	2.14	0.23	90.23	10.00	6.73	433.30

Subbasin Hydrology

Subbasin : Sub-AD-4

Pipe Input

SN Element	Length	Inlet	Inlet	Outlet	Outlet	Total	Average	Pipe	Pipe	Pipe	Man
ID		Invert	Invert	Invert	Invert	Drop	Slope	Shape	Diameter or	Width	Rougl
		Elevation	Offset	Elevation	Offset				Height		
	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(%)		(in)	(in)	
1 L-SL - (16)	112.54	433.11	4.11	432.25	6.25	0.86	0.7600	Dummy	0.000	0.000	0
2 SL - (14)	84.31	422.00	0.00	418.00	0.00	4.00	4.7400	CIRCULAR	18.000	18.000	0
3 SL - (15)	62.02	426.00	0.00	422.00	0.00	4.00	6.4500	CIRCULAR	18.000	18.000	0
4 SL - (16)	101.17	429.00	0.00	426.00	0.00	3.00	2.9700	CIRCULAR	18.000	18.000	0
5 SL - (18)	128.37	427.50	0.00	423.00	1.00	4.50	3.5100	CIRCULAR	18.000	18.000	0

No. of Barrels

1

1

Pipe Results

SN Element	Peak	Time of	Design Flow	Peak Flow/	Peak Flow	Travel	Peak Flow	Peak Flow	Tota
ID	Flow	Peak Flow	Capacity	Design Flow	Velocity	Time	Depth	Depth/	Surch
		Occurrence		Ratio				Total Depth	
								Ratio	
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		
1 L-SL - (16)	0.24	0 00:05	0.00	0.00	0.00		0.00	0.00	
2 SL - (14)	8.93	0 00:05	24.79	0.36	12.91	0.11	0.62	0.41	
3 SL - (15)	3.54	0 00:05	28.90	0.12	11.11	0.09	0.35	0.24	
4 SL - (16)	2.12	0 00:05	19.60	0.11	9.87	0.17	0.33	0.22	
5 SL - (18)	2.66	0 00:05	21.31	0.12	8.42	0.25	0.36	0.24	

Inlet Input

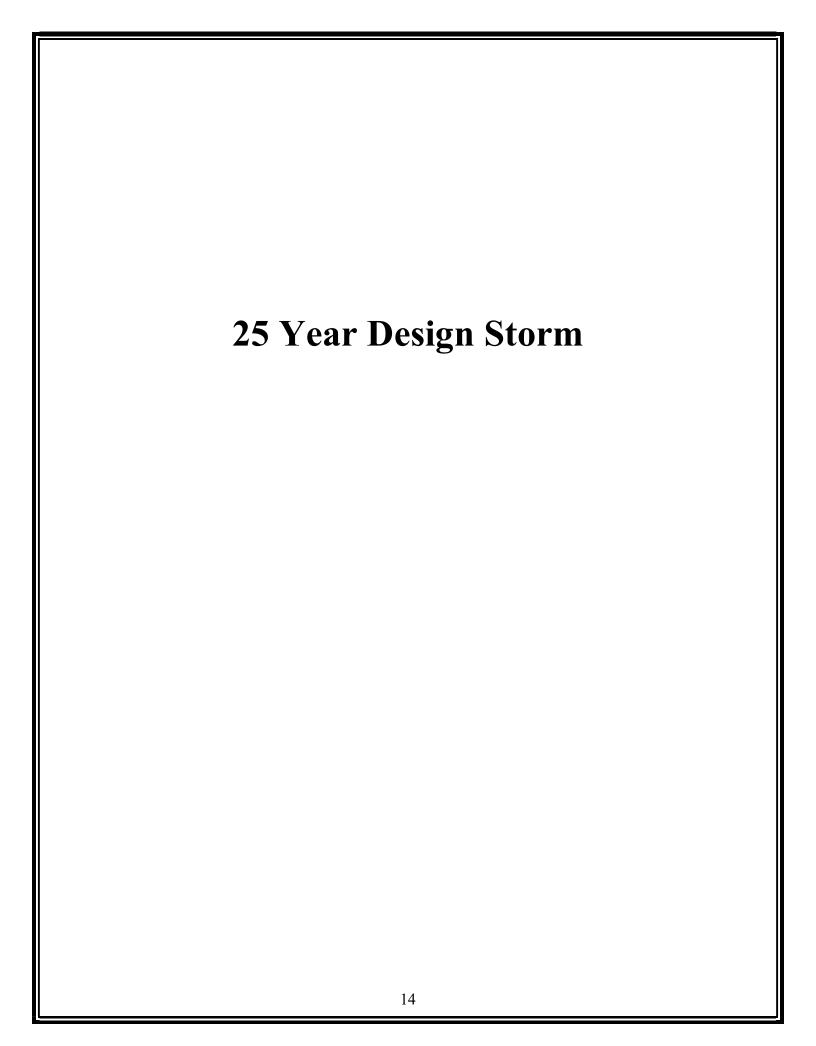
SN Element	Inlet	Number of	Catchbasin	Max (Rim)	Inlet	Initial	Initial	Ponded	Grate
ID	Location	Inlets	Invert	Elevation	Depth	Water	Water	Area	Clogging
			Elevation			Elevation	Depth		Factor
			(ft)	(ft)	(ft)	(ft)	(ft)	(ft ²)	(%)
1 AD-4	On Sag	1	427.50	431.80	4.30	427.50	0.00	10.00	0.00
2 CB-1	On Sag	1	422.00	431.61	9.61	422.00	0.00	10.00	0.00
3 CB-2	On Sag	1	426.00	432.25	6.25	426.00	0.00	10.00	0.00
4 CB-3	On Grade	1	429.00	433.11	4.11	429.00	0.00	N/A	0.00

Roadway & Gutter Input

SN Element	Roadway	Roadway	Roadway	Gutter	Gutter	Gutter	Allowable
ID	Longitudinal	Cross	Manning's	Cross	Width	Depression	Spread
	Slope	Slope	Roughness	Slope			
	(ft/ft)	(ft/ft)		(ft/ft)	(ft)	(in)	(ft)
1 AD-4	N/A	0.0300	0.0150	0.0300	1.50	0.1312	10.00
2 CB-1	N/A	0.0300	0.0150	0.0200	1.50	0.1312	10.00
3 CB-2	N/A	0.0300	0.0150	0.0200	1.50	0.1312	10.00
4 CB-3	0.0200	0.0300	0.0150	0.0200	1.50	0.1312	10.00

Inlet Results

SN Element	Peak	Peak	Peak Flow	Peak Flow	Inlet	Max Gutter	Max Gutter	Max Gutter	٦
ID	Flow	Lateral	Intercepted	Bypassing	Efficiency	Spread	Water Elev.	Water Depth	Max
		Inflow	by	Inlet	during Peak	during Peak	during Peak	during Peak	Occı
			Inlet		Flow	Flow	Flow	Flow	
	(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)	(days h
1 AD-4	2.70	2.70	N/A	N/A	N/A	10.40	432.21	0.41	(
2 CB-1	2.80	2.80	N/A	N/A	N/A	6.07	432.04	0.43	(
3 CB-2	1.46	1.22	N/A	N/A	N/A	3.92	432.62	0.37	(
4 CB-3	2.37	2.37	2.14	0.23	90.23	6.73	433.30	0.19	(



Project Description

File Name Bryant Pharmacy Drainage Analysis 9-5-25.SPF

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	Rational
Time of Concentration (TOC) Method	User-Defined
Link Routing Method	Kinematic Wave
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	NO

Analysis Options

Start Analysis On	Sep 05, 2025	00:00:00
End Analysis On	Sep 06, 2025	00:00:00
Start Reporting On	Sep 05, 2025	00:00:00
Antecedent Dry Days	0	days
Runoff (Dry Weather) Time Step	0 01:00:00	days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	30	seconds

Number of Elements

Q	≀ty
Rain Gages 0	
Subbasins	
Nodes	
Junctions 0	
Outfalls 1	
Flow Diversions 0	
Inlets 4	
Storage Nodes 0	
Links	
Channels 0	
<i>Pipes</i> 5	
Pumps 0	
Orifices 0	
Weirs 0	
Outlets 0	
Pollutants 0	
Land Uses 0	

Rainfall Details

Subbasin Summary

SN Subbasin ID	Area	- 3		Total Runoff	Total Runoff		Time of Concentration
		Coefficient			Volume		
	(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1 Sub-AD-4	0.35	0.9500	0.79	0.75	0.26	3.10	0 00:05:00
2 Sub-CB-1	0.36	0.9500	0.79	0.75	0.27	3.22	0 00:05:00
3 Sub-CB-2	0.16	0.9500	0.79	0.75	0.12	1.40	0 00:05:00
4 Sub-CB-3	0.30	0.9500	0.79	0.75	0.23	2 72	0 00:05:00

Node Summary

SN Element	Element	Invert	Ground/Rim	Initial	Surcharge	Ponded	Peak	Max HGL	Max	
ID	Type	Elevation	(Max)	Water	Elevation	Area	Inflow	Elevation	Surcharge	F
			Elevation	Elevation				Attained	Depth	
									Attained	
		(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	
1 Out-1SL - (14)	Outfall	418.00					10.27	418.67		

Link Summary

SN Element ID	Element Type	From (Inlet) Node	To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Slope	Diameter or Height	Manning's Roughness		Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth
				(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)
1 L-SL - (16)	Pipe	CB-3	CB-2	112.54	433.11	432.25	0.7600	0.000	0.0150	0.37	0.00	0.00	0.00	0.00
2 SL - (14)	Pipe	CB-1	Out-1SL - (14)	84.31	422.00	418.00	4.7400	18.000	0.0120	10.27	24.79	0.41	13.40	0.67
3 SL - (15)	Pipe	CB-2	CB-1	62.02	426.00	422.00	6.4500	18.000	0.0120	4.07	28.90	0.14	11.57	0.38
4 SL - (16)	Pipe	CB-3	CB-2	101.17	429.00	426.00	2.9700	18.000	0.0120	2.34	19.60	0.12	10.19	0.35
5 SL - (18)	Pipe	AD-4	CB-1	128.37	427.50	423.00	3.5100	18.000	0.0120	3.06	21.31	0.14	8.63	0.38

Inlet Summary

SN Element	Inlet	Number of	Catchbasin	Max (Rim)	Initial	Ponded	Peak	Peak Flow	Peak Flow	Inlet	Allowable	Max Gutter	Max Gutter
ID	Location	Inlets	Invert	Elevation	Water	Area	Flow	Intercepted	Bypassing	Efficiency	Spread	Spread	Water Elev.
			Elevation		Elevation			by	Inlet	during Peak		during Peak	during Peak
								Inlet		Flow		Flow	Flow
			(ft)	(ft)	(ft)	(ft ²)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
1 AD-4	On Sag	1	427.50	431.80	427.50	10.00	3.10	N/A	N/A	N/A	10.00	11.31	432.24
2 CB-1	On Sag	1	422.00	431.61	422.00	10.00	3.22	N/A	N/A	N/A	10.00	6.66	432.06
3 CB-2	On Sag	1	426.00	432.25	426.00	10.00	1.77	N/A	N/A	N/A	10.00	4.47	432.64
4 CB-3	On Grade	1	429.00	433.11	429.00	N/A	2.72	2.36	0.37	86.54	10.00	7.08	433.31

Subbasin Hydrology

Subbasin : Sub-AD-4

Pipe Input

SN Element	Length	Inlet	Inlet	Outlet	Outlet	Total	Average	Pipe	Pipe	Pipe	Man
ID		Invert	Invert	Invert	Invert	Drop	Slope	Shape	Diameter or	Width	Rougl
		Elevation	Offset	Elevation	Offset				Height		
	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(%)		(in)	(in)	
1 L-SL - (16)	112.54	433.11	4.11	432.25	6.25	0.86	0.7600	Dummy	0.000	0.000	0
2 SL - (14)	84.31	422.00	0.00	418.00	0.00	4.00	4.7400	CIRCULAR	18.000	18.000	0
3 SL - (15)	62.02	426.00	0.00	422.00	0.00	4.00	6.4500	CIRCULAR	18.000	18.000	0
4 SL - (16)	101.17	429.00	0.00	426.00	0.00	3.00	2.9700	CIRCULAR	18.000	18.000	0
5 SL - (18)	128.37	427.50	0.00	423.00	1.00	4.50	3.5100	CIRCULAR	18.000	18.000	0

No. of Barrels

1

1

Pipe Results

SN Element	Peak	Time of	Design Flow	Peak Flow/	Peak Flow	Travel	Peak Flow	Peak Flow	Tota
ID	Flow	Peak Flow	Capacity	Design Flow	Velocity	Time	Depth	Depth/	Surc
		Occurrence		Ratio				Total Depth	
								Ratio	
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		
1 L-SL - (16)	0.37	0 00:05	0.00	0.00	0.00		0.00	0.00	
2 SL - (14)	10.27	0 00:05	24.79	0.41	13.40	0.10	0.67	0.45	
3 SL - (15)	4.07	0 00:05	28.90	0.14	11.57	0.09	0.38	0.25	
4 SL - (16)	2.34	0 00:05	19.60	0.12	10.19	0.17	0.35	0.23	
5 SL - (18)	3.06	0 00:05	21.31	0.14	8.63	0.25	0.38	0.26	

Inlet Input

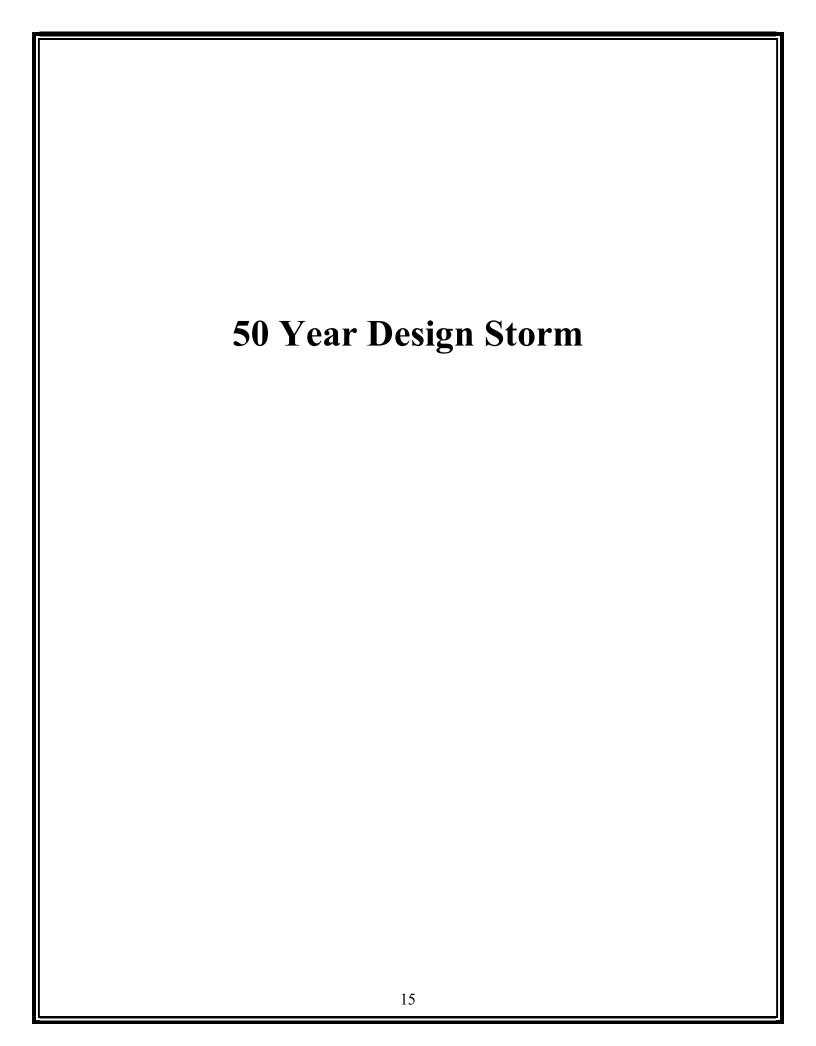
SI	N Element	Inlet	Number of	Catchbasin	Max (Rim)	Inlet	Initial	Initial	Ponded	Grate
	ID	Location	Inlets	Invert	Elevation	Depth	Water	Water	Area	Clogging
				Elevation			Elevation	Depth		Factor
				(ft)	(ft)	(ft)	(ft)	(ft)	(ft ²)	(%)
	1 AD-4	On Sag	1	427.50	431.80	4.30	427.50	0.00	10.00	0.00
	2 CB-1	On Sag	1	422.00	431.61	9.61	422.00	0.00	10.00	0.00
	3 CB-2	On Sag	1	426.00	432.25	6.25	426.00	0.00	10.00	0.00
	4 CB-3	On Grade	1	429.00	433.11	4.11	429.00	0.00	N/A	0.00

Roadway & Gutter Input

SN Element	Roadway	Roadway	Roadway	Gutter	Gutter	Gutter	Allowable
ID	Longitudinal	Cross	Manning's	Cross	Width	Depression	Spread
	Slope	Slope	Roughness	Slope			
	(ft/ft)	(ft/ft)		(ft/ft)	(ft)	(in)	(ft)
1 AD-4	N/A	0.0300	0.0150	0.0300	1.50	0.1312	10.00
2 CB-1	N/A	0.0300	0.0150	0.0200	1.50	0.1312	10.00
3 CB-2	N/A	0.0300	0.0150	0.0200	1.50	0.1312	10.00
4 CB-3	0.0200	0.0300	0.0150	0.0200	1.50	0.1312	10.00

Inlet Results

SN Element	Peak	Peak	Peak Flow	Peak Flow	Inlet	Max Gutter	Max Gutter	Max Gutter	1
ID	Flow	Lateral	Intercepted	Bypassing	Efficiency	Spread	Water Elev.	Water Depth	Max
		Inflow	by	Inlet	during Peak	during Peak	during Peak	during Peak	Occı
			Inlet		Flow	Flow	Flow	Flow	
	(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)	(days h
1 AD-4	3.10	3.10	N/A	N/A	N/A	11.31	432.24	0.44	(
2 CB-1	3.22	3.22	N/A	N/A	N/A	6.66	432.06	0.45	(
3 CB-2	1.77	1.40	N/A	N/A	N/A	4.47	432.64	0.38	(
4 CB-3	2.72	2.72	2.36	0.37	86.54	7.08	433.31	0.20	(



Project Description

File Name Bryant Pharmacy Drainage Analysis 9-5-25.SPF

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	Rational
Time of Concentration (TOC) Method	User-Defined
Link Routing Method	Kinematic Wave
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	NO

Analysis Options

Start Analysis On	Sep 05, 2025	00:00:00
End Analysis On	Sep 06, 2025	00:00:00
Start Reporting On	Sep 05, 2025	00:00:00
Antecedent Dry Days	0	days
Runoff (Dry Weather) Time Step	0 01:00:00	days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	30	seconds

Number of Elements

Rainfall Details

Return Period...... 50 year(s)

Subbasin Summary

SN Subbasin	Area	Weighted	Total	Total	Total	Peak	Time of
ID		Runoff	Rainfall	Runoff	Runoff	Runoff	Concentration
		Coefficient			Volume		
	(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1 Sub-AD-4	0.35	0.9500	0.86	0.82	0.28	3.38	0 00:05:00
2 Sub-CB-1	0.36	0.9500	0.86	0.82	0.29	3.51	0 00:05:00
3 Sub-CB-2	0.16	0.9500	0.86	0.82	0.13	1.53	0 00:05:00
4 Sub-CB-3	0.30	0.9500	0.86	0.82	0.25	2.98	0 00:05:00

Node Summary

SN Element	Element	Invert	Ground/Rim	Initial	Surcharge	Ponded	Peak	Max HGL	Max	
ID	Type	Elevation	(Max)	Water	Elevation	Area	Inflow	Elevation	Surcharge	F
			Elevation	Elevation				Attained	Depth	
									Attained	
		(ft)	(ft)	(ft)	(ft)	(ft²)	(cfs)	(ft)	(ft)	
1 Out-1SL - (14)	Outfall	418.00					11.22	418.71		

Link Summary

SN Element ID	Element Type		To (Outlet) Node	Length	Inlet Invert Elevation	Invert	Slope	Diameter or Height	Manning's Roughness		Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth
				(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)
1 L-SL - (16)	Pipe	CB-3	CB-2	112.54	433.11	432.25	0.7600	0.000	0.0150	0.47	0.00	0.00	0.00	0.00
2 SL - (14)	Pipe	CB-1	Out-1SL - (14)	84.31	422.00	418.00	4.7400	18.000	0.0120	11.22	24.79	0.45	13.71	0.71
3 SL - (15)	Pipe	CB-2	CB-1	62.02	426.00	422.00	6.4500	18.000	0.0120	4.45	28.90	0.15	11.88	0.40
4 SL - (16)	Pipe	CB-3	CB-2	101.17	429.00	426.00	2.9700	18.000	0.0120	2.48	19.60	0.13	10.41	0.36
5 SL - (18)	Pipe	AD-4	CB-1	128.37	427.50	423.00	3.5100	18.000	0.0120	3.34	21.31	0.16	8.85	0.40

Inlet Summary

9	SN Element	Inlet	Number of	Catchbasin	Max (Rim)	Initial	Ponded	Peak	Peak Flow	Peak Flow	Inlet	Allowable	Max Gutter	Max Gutter
	ID	Location	Inlets	Invert	Elevation	Water	Area	Flow	Intercepted	Bypassing	Efficiency	Spread	Spread	Water Elev.
				Elevation		Elevation			by	Inlet	during Peak		during Peak	during Peak
									Inlet		Flow		Flow	Flow
				(ft)	(ft)	(ft)	(ft ²)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
	1 AD-4	On Sag	1	427.50	431.80	427.50	10.00	3.38	N/A	N/A	N/A	10.00	11.93	432.26
	2 CB-1	On Sag	1	422.00	431.61	422.00	10.00	3.51	N/A	N/A	N/A	10.00	7.06	432.07
	3 CB-2	On Sag	1	426.00	432.25	426.00	10.00	2.01	N/A	N/A	N/A	10.00	4.86	432.65
	4 CB-3	On Grade	1	429.00	433.11	429.00	N/A	2.98	2.50	0.47	84.09	10.00	7.31	433.31

Subbasin Hydrology

Subbasin : Sub-AD-4

Pipe Input

SN Element	Length	Inlet	Inlet	Outlet	Outlet	Total	Average	Pipe	Pipe	Pipe	Man
ID		Invert	Invert	Invert	Invert	Drop	Slope	Shape	Diameter or	Width	Rougl
		Elevation	Offset	Elevation	Offset				Height		
	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(%)		(in)	(in)	
1 L-SL - (16)	112.54	433.11	4.11	432.25	6.25	0.86	0.7600	Dummy	0.000	0.000	0
2 SL - (14)	84.31	422.00	0.00	418.00	0.00	4.00	4.7400	CIRCULAR	18.000	18.000	0
3 SL - (15)	62.02	426.00	0.00	422.00	0.00	4.00	6.4500	CIRCULAR	18.000	18.000	0
4 SL - (16)	101.17	429.00	0.00	426.00	0.00	3.00	2.9700	CIRCULAR	18.000	18.000	0
5 SL - (18)	128.37	427.50	0.00	423.00	1.00	4.50	3.5100	CIRCULAR	18.000	18.000	0

No. of Barrels

1

1

Pipe Results

SN Element	Peak	Time of	Design Flow	Peak Flow/	Peak Flow	Travel	Peak Flow	Peak Flow	Tota
ID	Flow	Peak Flow	Capacity	Design Flow	Velocity	Time	Depth	Depth/	Surcl
		Occurrence		Ratio				Total Depth	
								Ratio	
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		
1 L-SL - (16)	0.47	0 00:05	0.00	0.00	0.00		0.00	0.00	
2 SL - (14)	11.22	0 00:05	24.79	0.45	13.71	0.10	0.71	0.47	
3 SL - (15)	4.45	0 00:05	28.90	0.15	11.88	0.09	0.40	0.27	
4 SL - (16)	2.48	0 00:05	19.60	0.13	10.41	0.16	0.36	0.24	
5 SL - (18)	3.34	0 00:05	21.31	0.16	8.85	0.24	0.40	0.27	

Inlet Input

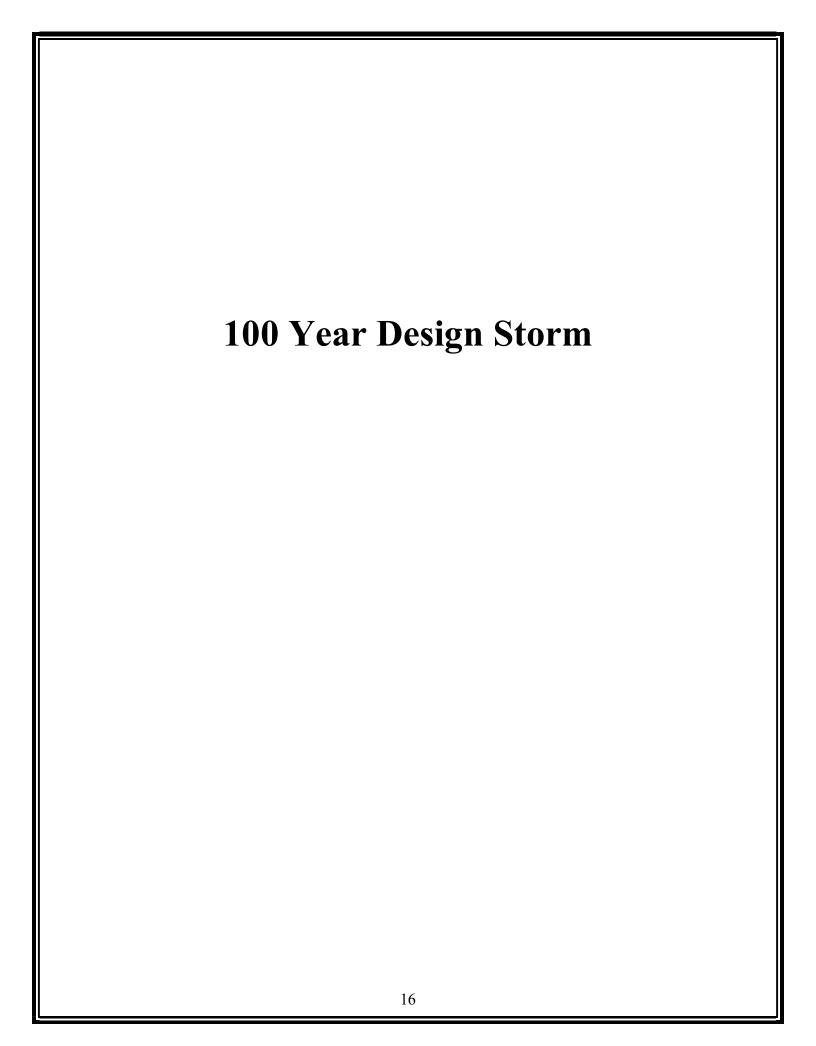
SN Element	Inlet	Number of	Catchbasin	Max (Rim)	Inlet	Initial	Initial	Ponded	Grate
ID	Location	Inlets	Invert	Elevation	Depth	Water	Water	Area	Clogging
			Elevation			Elevation	Depth		Factor
			(ft)	(ft)	(ft)	(ft)	(ft)	(ft ²)	(%)
1 AD-4	On Sag	1	427.50	431.80	4.30	427.50	0.00	10.00	0.00
2 CB-1	On Sag	1	422.00	431.61	9.61	422.00	0.00	10.00	0.00
3 CB-2	On Sag	1	426.00	432.25	6.25	426.00	0.00	10.00	0.00
4 CB-3	On Grade	1	429.00	433.11	4.11	429.00	0.00	N/A	0.00

Roadway & Gutter Input

SN Element	Roadway	Roadway	Roadway	Gutter	Gutter	Gutter	Allowable
ID	Longitudinal	Cross	Manning's	Cross	Width	Depression	Spread
	Slope	Slope	Roughness	Slope			
	(ft/ft)	(ft/ft)		(ft/ft)	(ft)	(in)	(ft)
1 AD-4	N/A	0.0300	0.0150	0.0300	1.50	0.1312	10.00
2 CB-1	N/A	0.0300	0.0150	0.0200	1.50	0.1312	10.00
3 CB-2	N/A	0.0300	0.0150	0.0200	1.50	0.1312	10.00
4 CB-3	0.0200	0.0300	0.0150	0.0200	1.50	0.1312	10.00

Inlet Results

SN Element	Peak	Peak	Peak Flow	Peak Flow	Inlet	Max Gutter	Max Gutter	Max Gutter	٦
ID	Flow	Lateral	Intercepted	Bypassing	Efficiency	Spread	Water Elev.	Water Depth	Max
		Inflow	by	Inlet	during Peak	during Peak	during Peak	during Peak	Occı
			Inlet		Flow	Flow	Flow	Flow	
	(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)	(days h
1 AD-4	3.38	3.38	N/A	N/A	N/A	11.93	432.26	0.46	(
2 CB-1	3.51	3.51	N/A	N/A	N/A	7.06	432.07	0.46	(
3 CB-2	2.01	1.53	N/A	N/A	N/A	4.86	432.65	0.40	(
4 CB-3	2.98	2.98	2.50	0.47	84.09	7.31	433.31	0.20	(



Project Description

File Name Bryant Pharmacy Drainage Analysis 9-5-25.SPF

Project Options

Flow Units	CFS
Elevation Type	Elevation
Hydrology Method	Rational
Time of Concentration (TOC) Method	User-Defined
Link Routing Method	Kinematic Wave
Enable Overflow Ponding at Nodes	YES
Skip Steady State Analysis Time Periods	NO

Analysis Options

Start Analysis On	Sep 05, 2025	00:00:00
End Analysis On	Sep 06, 2025	00:00:00
Start Reporting On	Sep 05, 2025	00:00:00
Antecedent Dry Days	0	days
Runoff (Dry Weather) Time Step	0 01:00:00	days hh:mm:ss
Runoff (Wet Weather) Time Step	0 00:05:00	days hh:mm:ss
Reporting Time Step	0 00:05:00	days hh:mm:ss
Routing Time Step	30	seconds

Number of Elements

Q	ty
Rain Gages 0	
Subbasins	
Nodes	
Junctions 0	
Outfalls 1	
Flow Diversions 0	
Inlets 4	
Storage Nodes 0	
Links 5	
Channels 0	
Pipes 5	
Pumps 0	
Orifices 0	
Weirs 0	
Outlets 0	
Pollutants 0	
Land Uses 0	

Rainfall Details

Subbasin Summary

SN Subbasin	Area	Weighted	Total	Total	Total	Peak	Time of
ID		Runoff	Rainfall	Runoff	Runoff	Runoff	Concentration
		Coefficient			Volume		
	(ac)		(in)	(in)	(ac-in)	(cfs)	(days hh:mm:ss)
1 Sub-AD-4	0.35	0.9500	0.93	0.89	0.31	3.68	0 00:05:00
2 Sub-CB-1	0.36	0.9500	0.93	0.89	0.32	3.82	0 00:05:00
3 Sub-CB-2	0.16	0.9500	0.93	0.89	0.14	1.67	0 00:05:00
4 Sub-CB-3	0.30	0.9500	0.93	0.89	0.27	3.24	0 00:05:00

Node Summary

Max	Max HGL	Peak	Ponded	Surcharge	Initial	Ground/Rim	Invert	Element	SN Element
charge F	Elevation	Inflow	Area	Elevation	Water	(Max)	Elevation	Type	ID
Depth	Attained				Elevation	Elevation			
tained									
(ft)	(ft)	(cfs)	(ft ²)	(ft)	(ft)	(ft)	(ft)		
	418.74	12.20					418.00) Outfall	1 Out-1SL - (14)
chai De tair	Elevation Attained (ft)	Inflow (cfs)	Area	Elevation	Water Elevation	(Max) Elevation	Elevation (ft)	Туре	ID

Link Summary

SN Element ID	Element Type		To (Outlet) Node	Length	Inlet Invert Elevation	Outlet Invert Elevation	Slope	Diameter or Height	Manning's Roughness		Design Flow Capacity	Peak Flow/ Design Flow Ratio	Peak Flow Velocity	Peak Flow Depth
				(ft)	(ft)	(ft)	(%)	(in)		(cfs)	(cfs)		(ft/sec)	(ft)
1 L-SL - (16)	Pipe	CB-3	CB-2	112.54	433.11	432.25	0.7600	0.000	0.0150	0.60	0.00	0.00	0.00	0.00
2 SL - (14)	Pipe	CB-1	Out-1SL - (14)	84.31	422.00	418.00	4.7400	18.000	0.0120	12.20	24.79	0.49	14.00	0.74
3 SL - (15)	Pipe	CB-2	CB-1	62.02	426.00	422.00	6.4500	18.000	0.0120	4.84	28.90	0.17	12.16	0.41
4 SL - (16)	Pipe	CB-3	CB-2	101.17	429.00	426.00	2.9700	18.000	0.0120	2.62	19.60	0.13	10.61	0.37
5 SL - (18)	Pipe	AD-4	CB-1	128.37	427.50	423.00	3.5100	18.000	0.0120	3.64	21.31	0.17	9.06	0.42

Inlet Summary

SN Element	Inlet	Number of	Catchbasin	Max (Rim)	Initial	Ponded	Peak	Peak Flow	Peak Flow	Inlet	Allowable	Max Gutter	Max Gutter
ID	Location	Inlets	Invert	Elevation	Water	Area	Flow	Intercepted	Bypassing	Efficiency	Spread	Spread	Water Elev.
			Elevation		Elevation			by	Inlet	during Peak		during Peak	during Peak
								Inlet		Flow		Flow	Flow
			(ft)	(ft)	(ft)	(ft ²)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)
1 AD-4	On Sag	1	427.50	431.80	427.50	10.00	3.68	N/A	N/A	N/A	10.00	12.56	432.28
2 CB-1	On Sag	1	422.00	431.61	422.00	10.00	3.82	N/A	N/A	N/A	10.00	7.47	432.08
3 CB-2	On Sag	1	426.00	432.25	426.00	10.00	2.26	N/A	N/A	N/A	10.00	5.26	432.66
4 CB-3	On Grade	1	429.00	433.11	429.00	N/A	3.24	2.64	0.59	81.70	10.00	7.54	433.32

Subbasin Hydrology

Subbasin : Sub-AD-4

Pipe Input

SN Element	Length	Inlet	Inlet	Outlet	Outlet	Total	Average	Pipe	Pipe	Pipe	Man
ID		Invert	Invert	Invert	Invert	Drop	Slope	Shape	Diameter or	Width	Rougl
		Elevation	Offset	Elevation	Offset				Height		
	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(%)		(in)	(in)	
1 L-SL - (16)	112.54	433.11	4.11	432.25	6.25	0.86	0.7600	Dummy	0.000	0.000	0
2 SL - (14)	84.31	422.00	0.00	418.00	0.00	4.00	4.7400	CIRCULAR	18.000	18.000	0
3 SL - (15)	62.02	426.00	0.00	422.00	0.00	4.00	6.4500	CIRCULAR	18.000	18.000	0
4 SL - (16)	101.17	429.00	0.00	426.00	0.00	3.00	2.9700	CIRCULAR	18.000	18.000	0
5 SL - (18)	128.37	427.50	0.00	423.00	1.00	4.50	3.5100	CIRCULAR	18.000	18.000	0

No. of Barrels

1

1

Pipe Results

SN Element	Peak	Time of	Design Flow	Peak Flow/	Peak Flow	Travel	Peak Flow	Peak Flow	Tota
ID	Flow	Peak Flow	Capacity	Design Flow	Velocity	Time	Depth	Depth/	Surc
		Occurrence		Ratio				Total Depth	
								Ratio	
	(cfs)	(days hh:mm)	(cfs)		(ft/sec)	(min)	(ft)		
1 L-SL - (16)	0.60	0 00:05	0.00	0.00	0.00		0.00	0.00	
2 SL - (14)	12.20	0 00:05	24.79	0.49	14.00	0.10	0.74	0.50	
3 SL - (15)	4.84	0 00:05	28.90	0.17	12.16	0.09	0.41	0.28	
4 SL - (16)	2.62	0 00:05	19.60	0.13	10.61	0.16	0.37	0.25	
5 SL - (18)	3.64	0 00:05	21.31	0.17	9.06	0.24	0.42	0.28	

Inlet Input

SI	N Element	Inlet	Number of	Catchbasin	Max (Rim)	Inlet	Initial	Initial	Ponded	Grate
	ID	Location	Inlets	Invert	Elevation	Depth	Water	Water	Area	Clogging
				Elevation			Elevation	Depth		Factor
				(ft)	(ft)	(ft)	(ft)	(ft)	(ft ²)	(%)
	1 AD-4	On Sag	1	427.50	431.80	4.30	427.50	0.00	10.00	0.00
	2 CB-1	On Sag	1	422.00	431.61	9.61	422.00	0.00	10.00	0.00
	3 CB-2	On Sag	1	426.00	432.25	6.25	426.00	0.00	10.00	0.00
	4 CB-3	On Grade	1	429.00	433.11	4.11	429.00	0.00	N/A	0.00

Roadway & Gutter Input

SN Element	Roadway	Roadway	Roadway	Gutter	Gutter	Gutter	Allowable
ID	Longitudinal	Cross	Manning's	Cross	Width	Depression	Spread
	Slope	Slope	Roughness	Slope			
	(ft/ft)	(ft/ft)		(ft/ft)	(ft)	(in)	(ft)
1 AD-4	N/A	0.0300	0.0150	0.0300	1.50	0.1312	10.00
2 CB-1	N/A	0.0300	0.0150	0.0200	1.50	0.1312	10.00
3 CB-2	N/A	0.0300	0.0150	0.0200	1.50	0.1312	10.00
4 CB-3	0.0200	0.0300	0.0150	0.0200	1.50	0.1312	10.00

Inlet Results

SN Element	Peak	Peak	Peak Flow	Peak Flow	Inlet	Max Gutter	Max Gutter	Max Gutter	٦
ID	Flow	Lateral	Intercepted	Bypassing	Efficiency	Spread	Water Elev.	Water Depth	Max
		Inflow	by	Inlet	during Peak	during Peak	during Peak	during Peak	Occı
			Inlet		Flow	Flow	Flow	Flow	
	(cfs)	(cfs)	(cfs)	(cfs)	(%)	(ft)	(ft)	(ft)	(days h
1 AD-4	3.68	3.68	N/A	N/A	N/A	12.56	432.28	0.48	(
2 CB-1	3.82	3.82	N/A	N/A	N/A	7.47	432.08	0.47	(
3 CB-2	2.26	1.67	N/A	N/A	N/A	5.26	432.66	0.41	(
4 CB-3	3.24	3.24	2.64	0.59	81.70	7.54	433.32	0.21	(

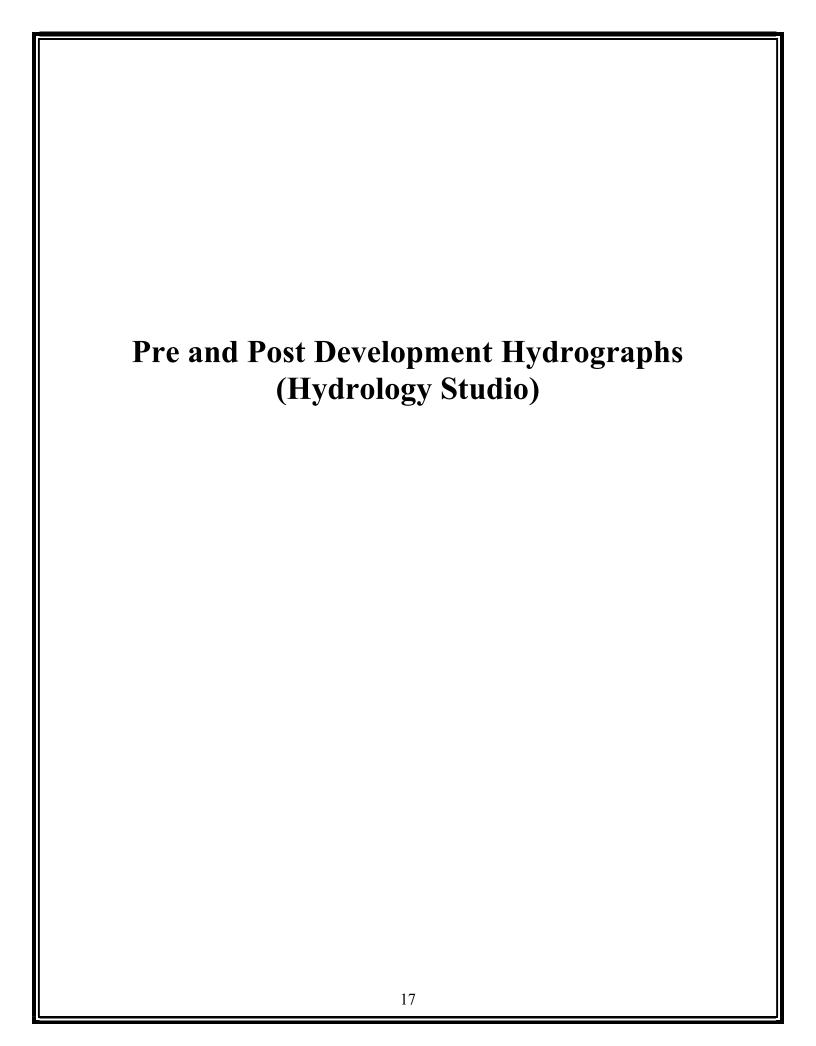


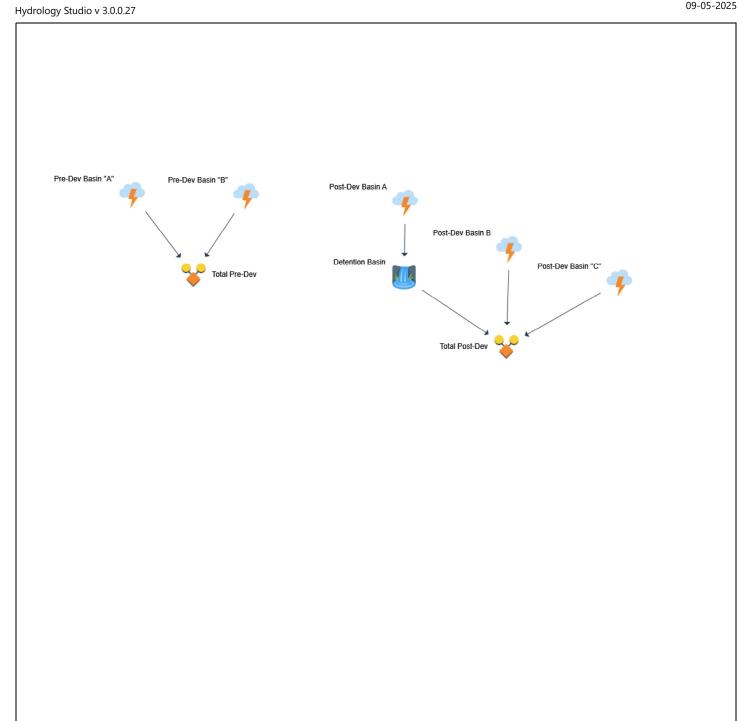
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	Hydrograph No. 6, Rational, Post-Dev Basin B	49
	Hydrograph No. 7, Rational, Post-Dev Basin "C"	50
	Hydrograph No. 8, Junction, Total Post-Dev	51



Hydrograph by Return Period

Project Name: Bryant Pharmacy 09-05-2025

Hydrograph No. Type		Hydrograph				Peak Out	flow (cfs)			
•	Туре	Name	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-y
-	Rational	Pre-Dev Basin "A"		3.084			4.131	4.746	5.188	5.636
ı	Rational	Pre-Dev Basin "B"		1.736			2.327	2.674	2.924	3.175
,	Junction	Total Pre-Dev		4.385			5.876	6.751	7.381	8.017
ı	Mod Rational	Post-Dev Basin A		3.206			4.316	4.964	5.437	5.893
ı	Pond Route	Detention Basin		3.206			4.316	4.964	5.437	5.89
ı	Rational	Post-Dev Basin B		0.573			0.767	0.882	0.964	1.04
ı	Rational	Post-Dev Basin "C"		0.532			0.712	0.818	0.893	0.97
	Junction	Total Post-Dev		3.810			5.127	5.623	6.097	6.62

Project Name: Bryant Pharmacy

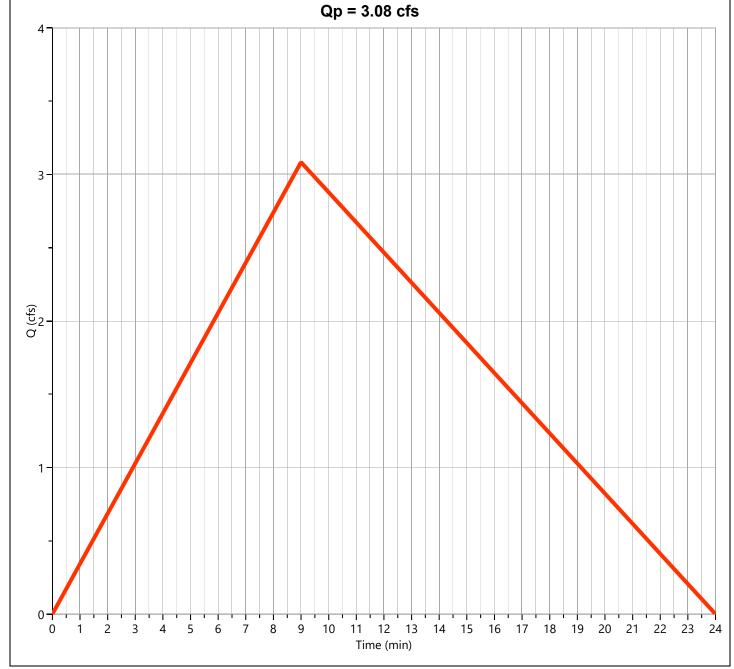
Hydrograph 2-yr Summary

09-05-2025 Hydrology Studio v 3.0.0.27 Peak Time to Hydrograph Inflow Maximum Maximum Hyd. Hydrograph Hydrograph Flow Peak Volume Hyd(s) Elevation Storage No. Type Name (hrs) (cuft) (cuft) (cfs) (ft) Rational Pre-Dev Basin "A" 3.084 0.15 2,223 1 2 Rational Pre-Dev Basin "B" 1.736 0.20 1,668 3 Junction Total Pre-Dev 4.385 0.15 3,886 1, 2 Mod Rational 3.206 0.08 8,946 4 Post-Dev Basin A 5 Pond Route **Detention Basin** 3.206 0.63 8,847 418.73 558 4 Rational Post-Dev Basin B 0.573 0.17 459 6 7 Rational Post-Dev Basin "C" 0.532 0.08 213 8 Junction Total Post-Dev 3.810 0.17 9,501 5, 6, 7

Hydrology Studio v 3.0.0.27 09-05-2025

Pre-Dev Basin "A"

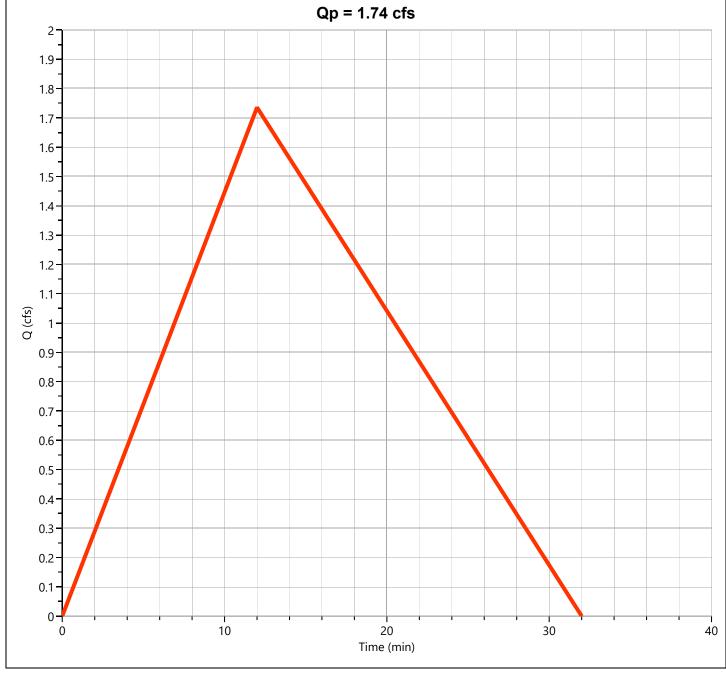
Hydrograph Type	= Rational	Peak Flow	= 3.084 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.15 hrs
Time Interval	= 1 min	Runoff Volume	= 2,223 cuft
Drainage Area	= 1.17 ac	Runoff Coeff.	= 0.56
Tc Method	= TR55	Time of Conc. (Tc)	= 9.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 4.71 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Pre-Dev Basin "B"

Hydrograph Type	= Rational	Peak Flow	= 1.736 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.20 hrs
Time Interval	= 1 min	Runoff Volume	= 1,668 cuft
Drainage Area	= 0.75 ac	Runoff Coeff.	= 0.56
Tc Method	= TR55	Time of Conc. (Tc)	= 12.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 4.13 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factor	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Total Pre-Dev Hyd. No. 3

Hydrograph Type	= Junction	Peak Flow	= 4.385 cfs			
Storm Frequency	= 2-yr	Time to Peak	= 0.15 hrs			
Time Interval	= 1 min	Hydrograph Volume	= 0.15 hrs ne = 3,886 cuft			
Inflow Hydrographs	= 1, 2	Total Contrib. Area	= 1.92 ac			
	Qp = 4.39	cfs				
5 - 4 - 3 - (st) O 2 -	Qp = 4.39	cfs				
	10 20 Time (m — Pre-Dev Basin "A" — Pre-Dev B		40			

Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin A

Storm Frequency = 2-yr	Hydrograph Type	= Mod Rational	Peak Flow	= 3.206 cfs
Drainage Area	Storm Frequency	= 2-yr	Time to Peak	= 0.08 hrs
To Method = User Time of Conc. (Tc) = 5.0 min IDF Curve = City of Bryant IDF Curve.idf Intensity = 2.25 in/hr Freq. Corr. Factor = 1.00 Storm Duration = 9.3 x Tc Target Q = 3.080 cfs Required Storage = 4,233 cuft Qp = 3.21 cfs	Time Interval	= 1 min	Runoff Volume	= 8,946 cuft
IDF Curve = City of Bryant IDF Curve.idf Intensity = 2.25 in/hr Freq. Corr. Factor = 1.00 Storm Duration = 9.3 x Tc Target Q = 3.080 cfs Required Storage = 4,233 cuft Qp = 3.21 cfs	Drainage Area	= 1.5 ac	Runoff Coeff.	= 0.95
Freq. Corr. Factor = 1.00	Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Target Q = 3.080 cfs Required Storage = 4,233 cuft Qp = 3.21 cfs 4 3	IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 2.25 in/hr
Qp = 3.21 cfs	Freq. Corr. Factor	= 1.00	Storm Duration	= 9.3 x Tc
3- - - - - - - - - -	Target Q	= 3.080 cfs	Required Storage	= 4,233 cuft
		Qp = 3.21 cfs		
(\$\frac{1}{2}\)	-			
	_			
0 10 20 30 40 50 6 Time (min)	1-		40	60 60

Hydrology Studio v 3.0.0.27 09-05-2025

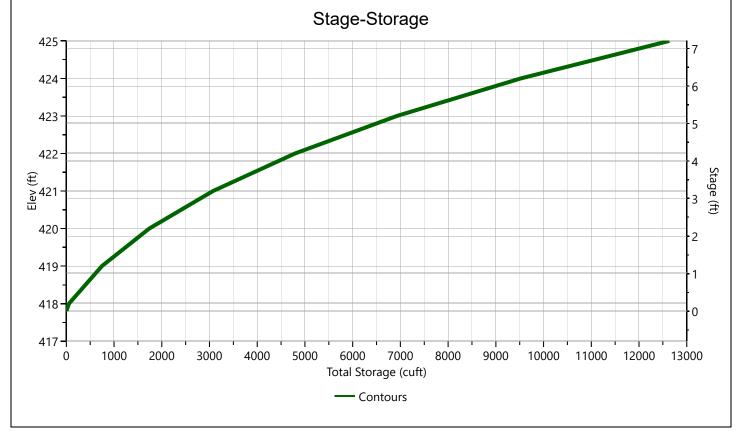
Detention Basin

Hydrograph Type	= Pond Route		Peak Flow	= 3.206 cfs
Storm Frequency	= 2-yr		Time to Peak	= 0.63 hrs
Time Interval	= 1 min		Hydrograph Volume	= 8,847 cuft
Inflow Hydrograph	= 4 - Post-Dev Basin A		Max. Elevation	= 418.73 ft
Pond Name	= Bryant Pharmacy De	tention Pond	Max. Storage	= 558 cuft
Pond Routing by Storage In	dication Method		Center of ma	ass detention time = 3 min
		Qp = 3.21 cfs		
3- (St) O				
	20 3	0 40 Time (min) Post-Dev Basin A — Det	50 60	70 80

Bryant Pharmacy Detention Pond

Stage-Storage

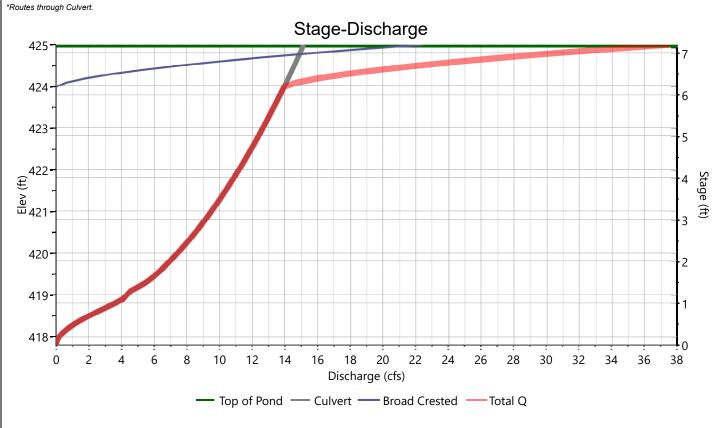
User Defined Conto			Stage / Stora	ge Table		
Description	Input	Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
Bottom Elevation, ft	417.80					
Voids (%)	100.00	0.00	417.80	4	0.000	0.000
Volus (78)	100.00	0.20	418.00	548	55.2	55.2
Volume Calc	Ave End Area	1.20	419.00	833	691	746
		2.20	420.00	1,155	994	1,740
		3.20	421.00	1,516	1,336	3,075
		4.20	422.00	1,916	1,716	4,791
		5.20	423.00	2,356	2,136	6,927
		6.20	424.00	2,835	2,596	9,523
		7.20	425.00	3,390	3,113	12,635



Bryant Pharmacy Detention Pond

Stage-Discharge

Culvert / Ouifices	Culvent		Orifices		Doufounted Discu
Culvert / Orifices	Culvert	1	2	3	Perforated Riser
Rise, in	15				Hole Diameter, in
Span, in	15				No. holes
No. Barrels	1				Invert Elevation, ft
Invert Elevation, ft	417.80				Height, ft
Orifice Coefficient, Co	0.60				Orifice Coefficient, Co
Length, ft	30				
Barrel Slope, %	1.32				
N-Value, n	0.012				
Maine	Riser*		Weirs		Amaillam
Weirs	Kiser	1	2	3	Ancillary
Shape / Type	Circular	Broad Crested			Exfiltration, in/hr
Crest Elevation, ft		424			
Crest Length, ft		6			
Angle, deg		45 (1:1)			
Weir Coefficient, Cw		3.3			



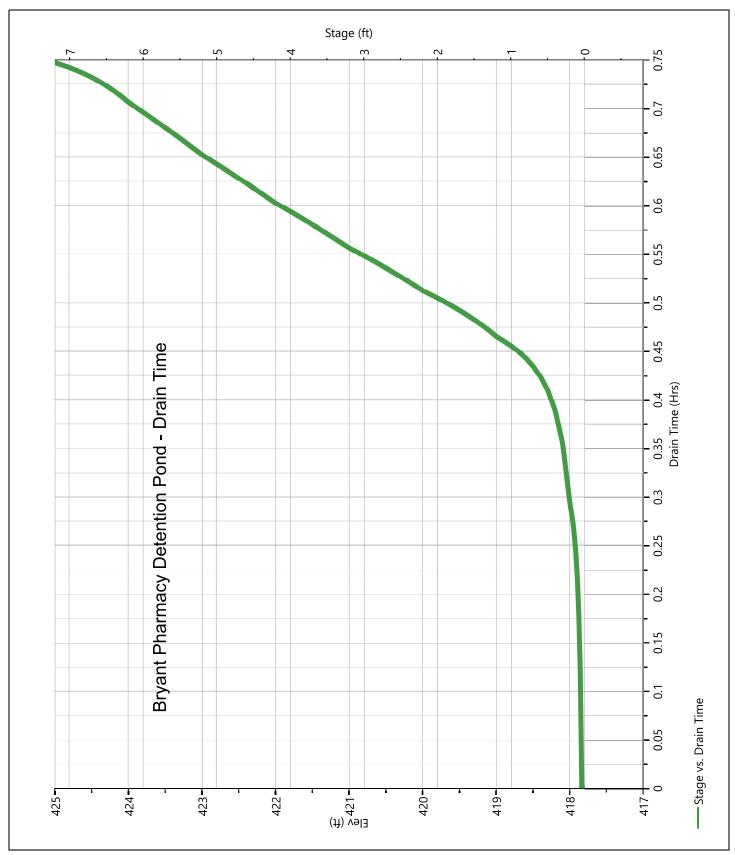
Bryant Pharmacy Detention Pond

Stage-Storage-Discharge Summary

Stage	Elev.	Storage	Culvert	C	Orifices, cf	s	Riser		Weirs, cfs	i	Pf Riser	Exfil	User	Total
(ft)	(ft)	(cuft)	(cfs)	1	2	3	(cfs)	1	2	3	(cfs)	(cfs)	(cfs)	(cfs)
0.00	417.80	0.000	0.000					0.000						0.000
0.20	418.00	55.2	0.193 ic					0.000						0.193
1.20	419.00	746	4.324 oc					0.000						4.324
2.20	420.00	1,740	7.415 ic					0.000						7.415
3.20	421.00	3,075	9.481 ic					0.000						9.481
4.20	422.00	4,791	11.17 ic					0.000						11.17
5.20	423.00	6,927	12.64 ic					0.000						12.64
6.20	424.00	9,523	13.95 ic					0.000						13.95
7.20	425.00	12,635	15.15 ic					22.44						37.59

Bryant Pharmacy Detention Pond

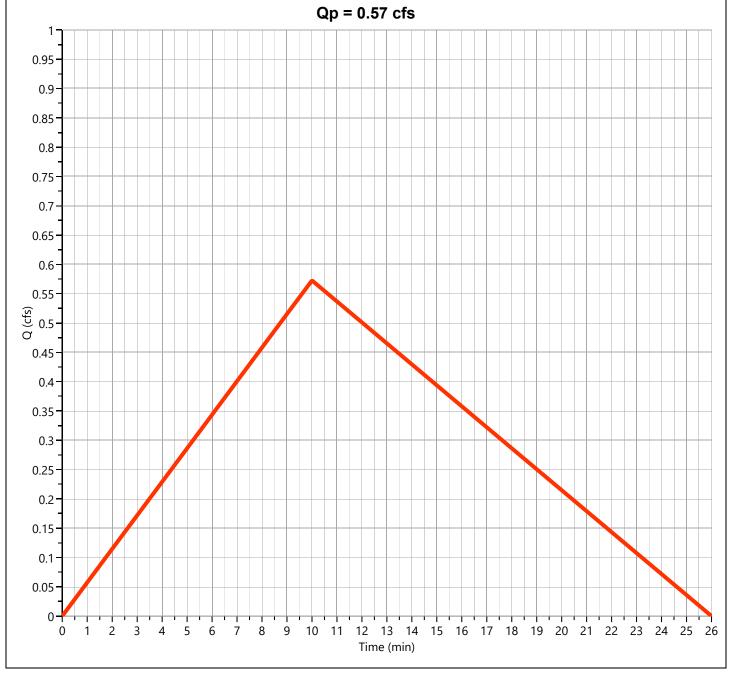
Pond Drawdown



Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin B Hyd. No. 6

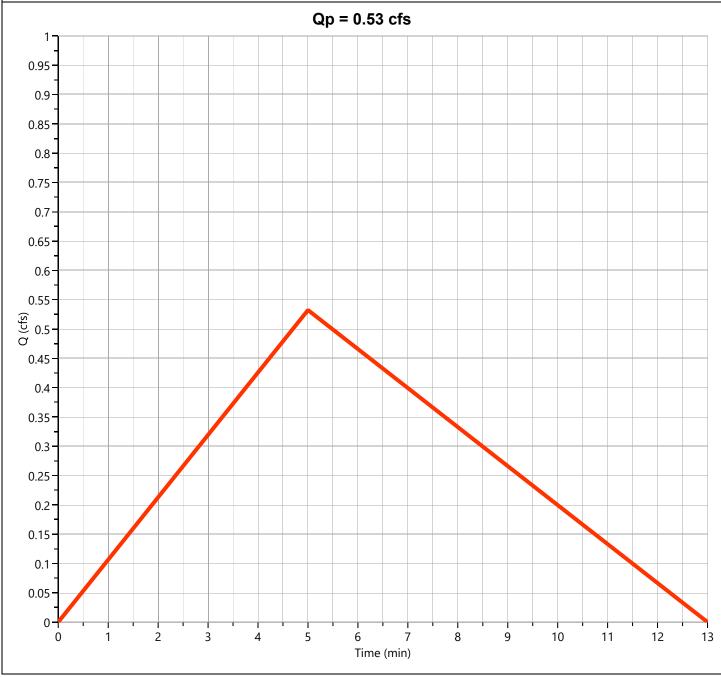
Hydrograph Type	= Rational	Peak Flow	= 0.573 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.17 hrs
Time Interval	= 1 min	Runoff Volume	= 459 cuft
Drainage Area	= 0.22 ac	Runoff Coeff.	= 0.58
Tc Method	= TR55	Time of Conc. (Tc)	= 10.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 4.49 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin "C"

Hydrograph Type	= Rational	Peak Flow	= 0.532 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 213 cuft
Drainage Area	= 0.17 ac	Runoff Coeff.	= 0.51
Tc Method	= TR55	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 6.14 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Total Post-Dev Hyd. No. 8

Hydrograph Type	= Junction	Peak Flow	= 3.810 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.17 hrs
Time Interval	= 1 min	Hydrograph Volume	= 9,501 cuft
Inflow Hydrographs	= 5, 6, 7	Total Contrib. Area	= 0.39 ac
	Qp = 3.81 cfs		
3-			
(Sb) 2 - 0 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	0 15 20 25 30 35 40	45 50 55	60 65 70
0 5 10	0 15 20 25 30 35 40 Time (min)	45 50 55	60 65 /0
-	— Detention Basin — Post-Dev Basin B — Post-Dev Ba	sin "C" — Total Post-Dev	

Project Name: Bryant Pharmacy

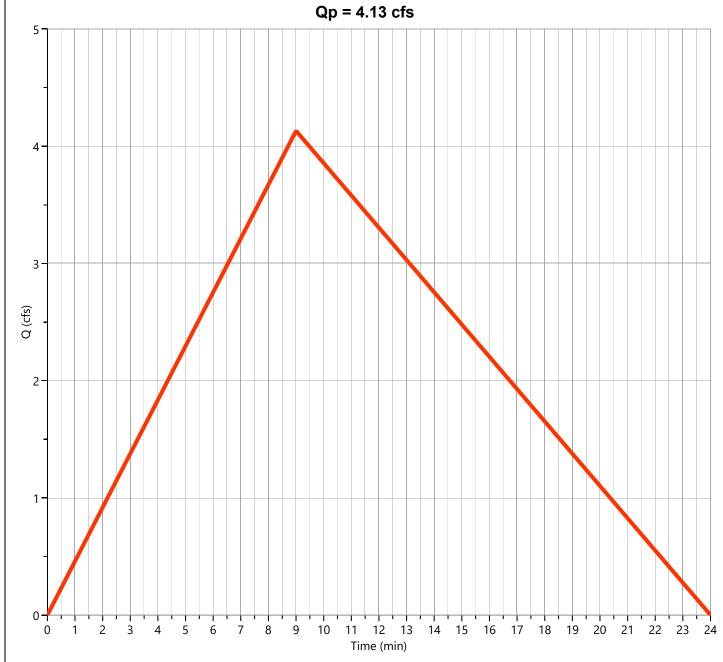
Hydrograph 10-yr Summary

09-05-2025 Hydrology Studio v 3.0.0.27 Peak Time to Hydrograph Inflow Maximum Maximum Hyd. Hydrograph Hydrograph Flow Peak Volume Hyd(s) Elevation Storage No. Type Name (hrs) (cuft) (cuft) (cfs) (ft) Rational Pre-Dev Basin "A" 4.131 0.15 2,978 1 2 Rational Pre-Dev Basin "B" 2.327 0.20 2,236 0.15 3 Junction Total Pre-Dev 5.876 5,208 1, 2 0.08 Mod Rational 12,041 4 Post-Dev Basin A 4.316 0.77 419.00 5 Pond Route **Detention Basin** 4.316 11,909 743 4 Rational Post-Dev Basin B 0.767 0.17 615 6 7 Rational Post-Dev Basin "C" 0.712 0.08 285 8 Junction Total Post-Dev 5.127 0.17 12,785 5, 6, 7

Hydrology Studio v 3.0.0.27 09-05-2025

Pre-Dev Basin "A"

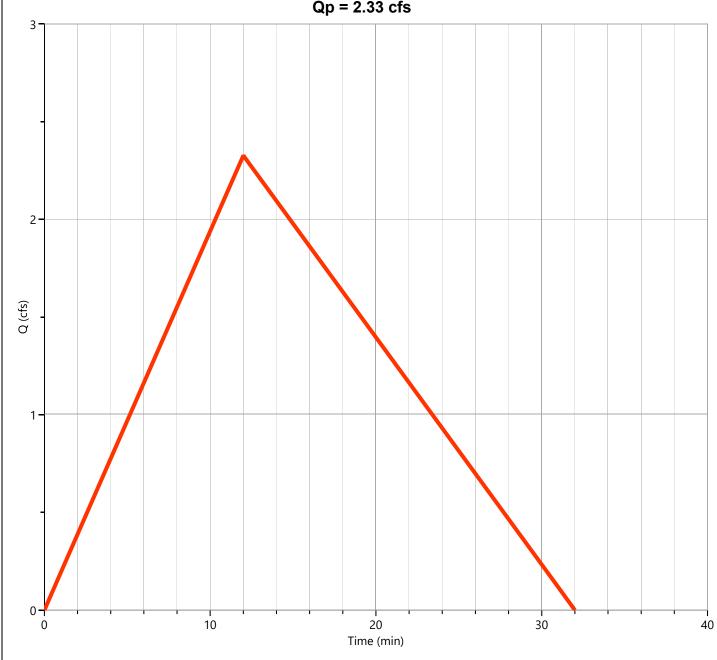
= Rational	Peak Flow	= 4.131 cfs
= 10-yr	Time to Peak	= 0.15 hrs
= 1 min	Runoff Volume	= 2,978 cuft
= 1.17 ac	Runoff Coeff.	= 0.56
= TR55	Time of Conc. (Tc)	= 9.0 min
= City of Bryant IDF Curve.idf	Intensity	= 6.30 in/hr
= 1.00	Asc/Rec Limb Factors	s = 1/1.67
	= 10-yr = 1 min = 1.17 ac = TR55 = City of Bryant IDF Curve.idf	= 10-yr Time to Peak = 1 min Runoff Volume = 1.17 ac Runoff Coeff. = TR55 Time of Conc. (Tc) = City of Bryant IDF Curve.idf Intensity



Hydrology Studio v 3.0.0.27 09-05-2025

Pre-Dev Basin "B"

		D 1 5			
Hydrograph Type	= Rational	Peak Flow = 2.327 cfs			
Storm Frequency	= 10-yr	Time to Peak = 0.20 hrs			
Time Interval	= 1 min	Runoff Volume = 2,236 cuft			
Drainage Area	= 0.75 ac	Runoff Coeff. = 0.56			
Tc Method	= TR55	Time of Conc. (Tc) = 12.0 min			
IDF Curve	= City of Bryant IDF Curve.idf	Intensity = 5.54 in/hr			
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors = 1/1.67			
Qp = 2.33 cfs					
3 7	·•				



Hydrology Studio v 3.0.0.27 09-05-2025

Total Pre-Dev Hyd. No. 3

Storm Frequency = 10-yr Time to Peak = 0.15 hrs Time Interval = 1 min Hydrograph Volume = 5,208 cuft Inflow Hydrographs = 1, 2 Qp = 5.88 cfs G G G G G G G G G G G G G	Hydrograph Type	= Junction	Peak Flow	= 5.876 cfs
Inflow Hydrographs = 1, 2 Qp = 5.88 cfs G 4 Qp = 5.88 cfs	Storm Frequency	= 10-yr	Time to Peak	= 0.15 hrs
Qp = 5.88 cfs 6 4 4 2 2	Time Interval	= 1 min	Hydrograph Volume	= 5,208 cuft
	Inflow Hydrographs	= 1, 2	Total Contrib. Area	= 1.92 ac
		Qp = 5.88 cfs		
	4-	Qp = 5.88 cfs		
0 10 20 30 Time (min) —— Pre-Dev Basin "A" —— Pre-Dev Basin "B" —— Total Pre-Dev	1	Time (min)		40

50

60

Hydrograph Report

Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin A

Hyd. No. 4

Hydrograph Type	= Mod Rational	Peak Flow	= 4.316 cfs
Storm Frequency	= 10-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 12,041 cuft
Drainage Area	= 1.5 ac	Runoff Coeff.	= 0.95
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 3.03 in/hr
Freq. Corr. Factor	= 1.00	Storm Duration	= 9.3 x Tc
Target Q	= 4.130 cfs	Required Storage	= 5,722 cuft
	Qp = 4.32 cfs		
4-			
(s) - (ct) O			

30

Time (min)

40

20

10

Hydrology Studio v 3.0.0.27 09-05-2025

Detention Basin

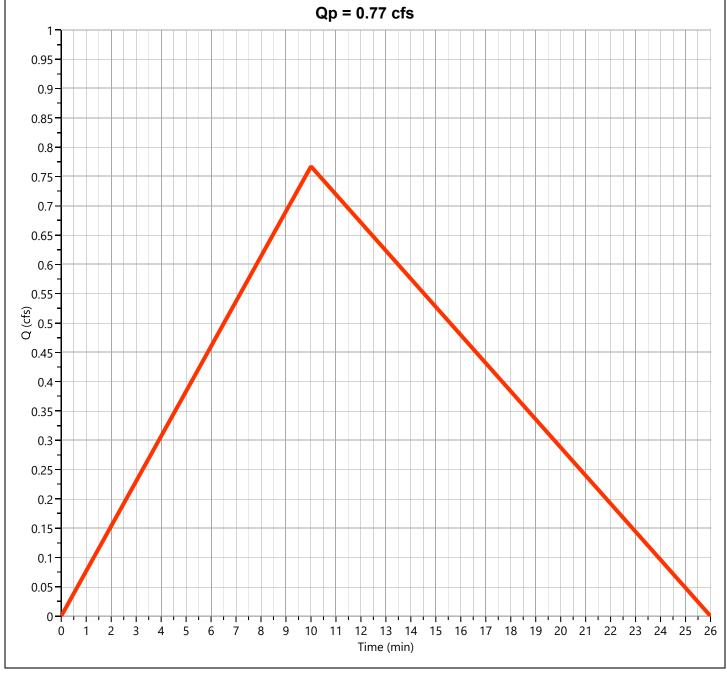
Hydrograph Type	= Pond Route			Peak Flow	= 4.316 cfs
Storm Frequency	= 10-yr			Time to Peak	= 0.77 hrs
Time Interval	= 1 min			Hydrograph Volume	= 11,909 cuft
Inflow Hydrograph	= 4 - Post-Dev Basin	Α		Max. Elevation	= 419.00 ft
Pond Name	= Bryant Pharmacy [Detention Pond		Max. Storage	= 743 cuft
Pond Routing by Storage I	ndication Method			Center of n	nass detention time = 3 min
		Qp = 4.32	cfs		
5		<u> </u>			
1					
4					
-					
3					
Q (cfs)					
2					
_					
1					
1-					
1 /					
0					
0	10 20	30 Time (r	40 nin)	50	60 70
		or — Post-Dev Basi		ntion Basin	
	neq a ste		Deter		
		04			

Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin B

Hyd.	No.	6
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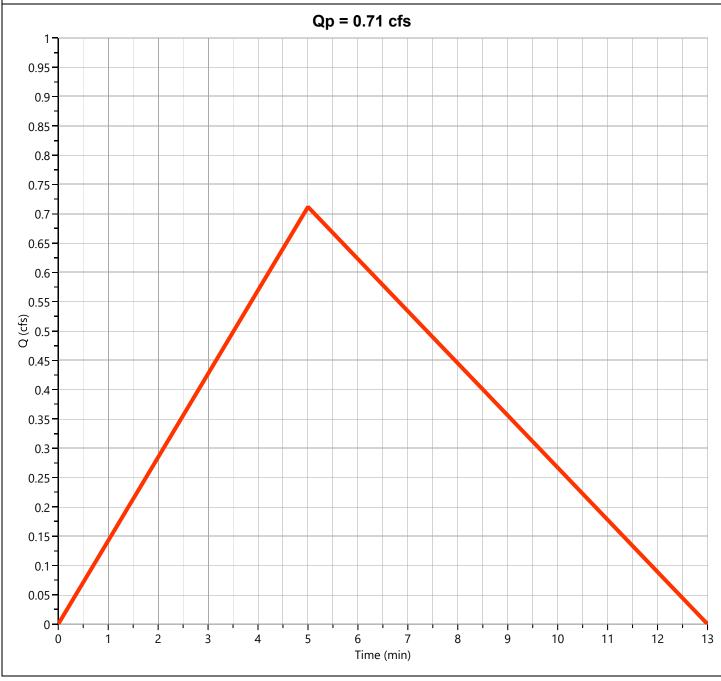
Hydrograph Type	= Rational	Peak Flow	= 0.767 cfs
Storm Frequency	= 10-yr	Time to Peak	= 0.17 hrs
Time Interval	= 1 min	Runoff Volume	= 615 cuft
Drainage Area	= 0.22 ac	Runoff Coeff.	= 0.58
Tc Method	= TR55	Time of Conc. (Tc)	= 10.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 6.01 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin "C"

Hydrograph Type	= Rational	Peak Flow	= 0.712 cfs
Storm Frequency	= 10-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 285 cuft
Drainage Area	= 0.17 ac	Runoff Coeff.	= 0.51
Tc Method	= TR55	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 8.21 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Total Post-Dev Hyd. No. 8

Hydrograph Type	= Junction	Peak Flow	= 5.127 cfs
Storm Frequency	ncy = 10-yr Time to Peak		= 0.17 hrs
Time Interval	= 1 min	Hydrograph Volume	
Inflow Hydrographs	= 5, 6, 7	Total Contrib. Area	= 0.39 ac
	Qp = 5.13 cfs		
5-4-	ψp = 5.13 CIS		
(SS) 3			
	0 15 20 25 30 35 40 Time (min)	45 50 55	60 65
_	— Detention Basin — Post-Dev Basin B — Post-Dev B	asin "C" — Total Post-Dev	
	24		

Project Name: Bryant Pharmacy

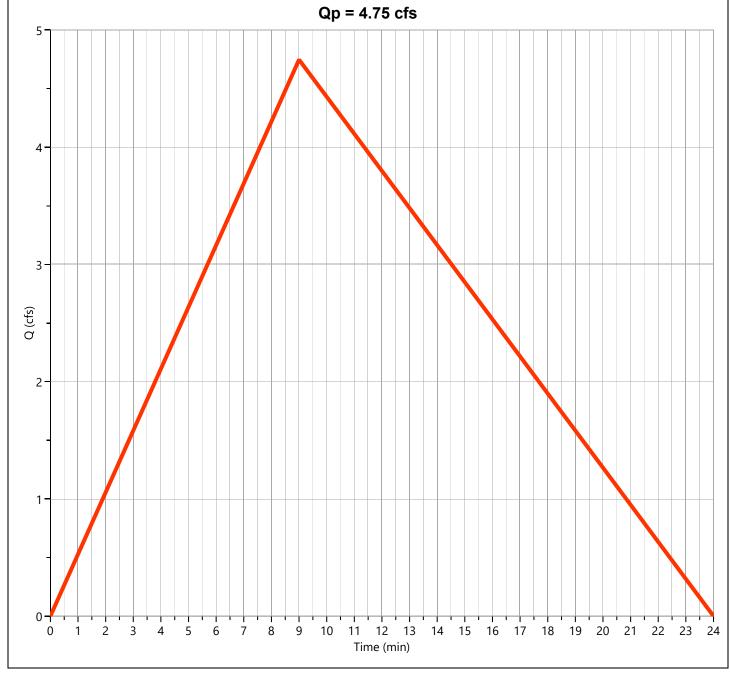
Hydrograph 25-yr Summary Hydrology Studio v 3.0.0.27

09-05-2025

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	Rational	Pre-Dev Basin "A"	4.746	0.15	3,421			
2	Rational	Pre-Dev Basin "B"	2.674	0.20	2,570			
3	Junction	Total Pre-Dev	6.751	0.15	5,984	1, 2		
4	Mod Rational	Post-Dev Basin A	4.964	0.08	13,849			
5	Pond Route	Detention Basin	4.964	0.77	13,698	4	419.19	930
6	Rational	Post-Dev Basin B	0.882	0.17	706			
7	Rational	Post-Dev Basin "C"	0.818	0.08	327			
8	Junction	Total Post-Dev	5.623	0.17	14,704	5, 6, 7		

Pre-Dev Basin "A"

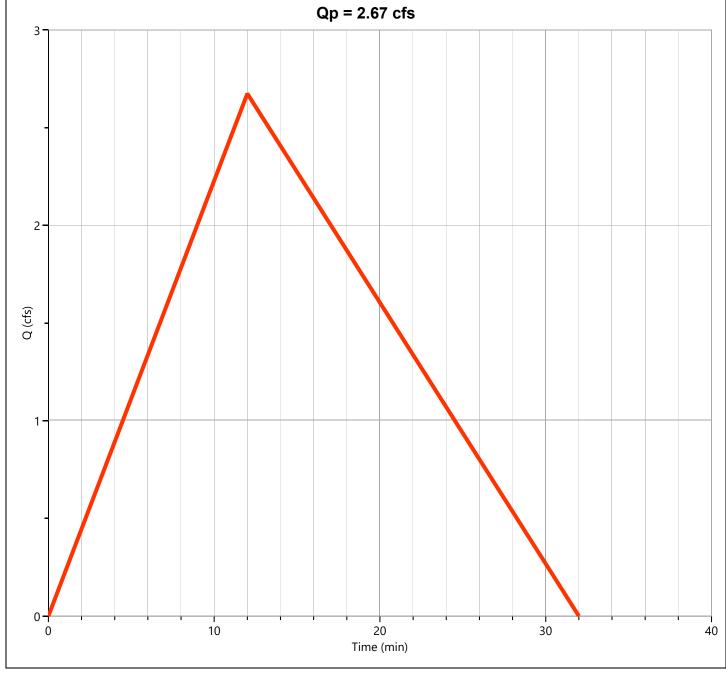
Hydrograph Type	= Rational	Peak Flow	= 4.746 cfs
Storm Frequency	= 25-yr	Time to Peak	= 0.15 hrs
Time Interval	= 1 min	Runoff Volume	= 3,421 cuft
Drainage Area	= 1.17 ac	Runoff Coeff.	= 0.56
Tc Method	= TR55	Time of Conc. (Tc)	= 9.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 7.24 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Pre-Dev Basin "B"

Hydrograph Type	= Rational	Peak Flow	= 2.674 cfs
Storm Frequency	= 25-yr	Time to Peak	= 0.20 hrs
Time Interval	= 1 min	Runoff Volume	= 2,570 cuft
Drainage Area	= 0.75 ac	Runoff Coeff.	= 0.56
Tc Method	= TR55	Time of Conc. (Tc)	= 12.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 6.37 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Total Pre-Dev Hyd. No. 3

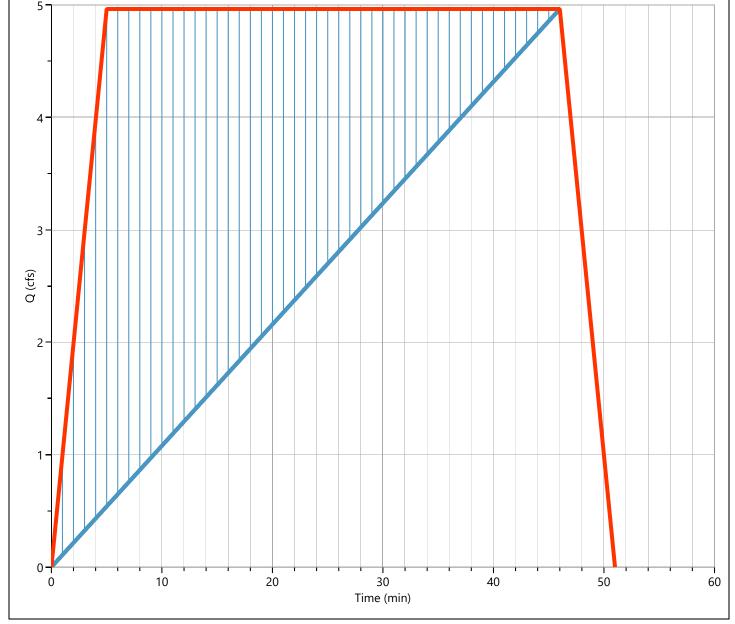
Hydrograph Type	= Junction		Peak Flow	= 6.751 cfs
Storm Frequency	= 25-yr		Time to Peak	= 0.15 hrs
Time Interval	= 1 min		Hydrograph Volume	= 5,984 cuft
Inflow Hydrographs	= 1, 2		Total Contrib. Area	= 1.92 ac
	Qp =	6.75 cfs		
7 - 6 - 5 - 4 - (c) O				
2				
0 0	10 — Pre-Dev Basin "A" — P	20 Time (min) re-Dev Basin "B"	30 Total Pre-Dev	40

Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin A

Hydrograph Type	= Mod Rational	Peak Flow	= 4.964 cfs
Storm Frequency	= 25-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 13,849 cuft
Drainage Area	= 1.5 ac	Runoff Coeff.	= 0.95
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 3.48 in/hr
Freq. Corr. Factor	= 1.00	Storm Duration	= 9.3 x Tc
Target Q	= 4.750 cfs	Required Storage	= 6,582 cuft





Hydrology Studio v 3.0.0.27 09-05-2025

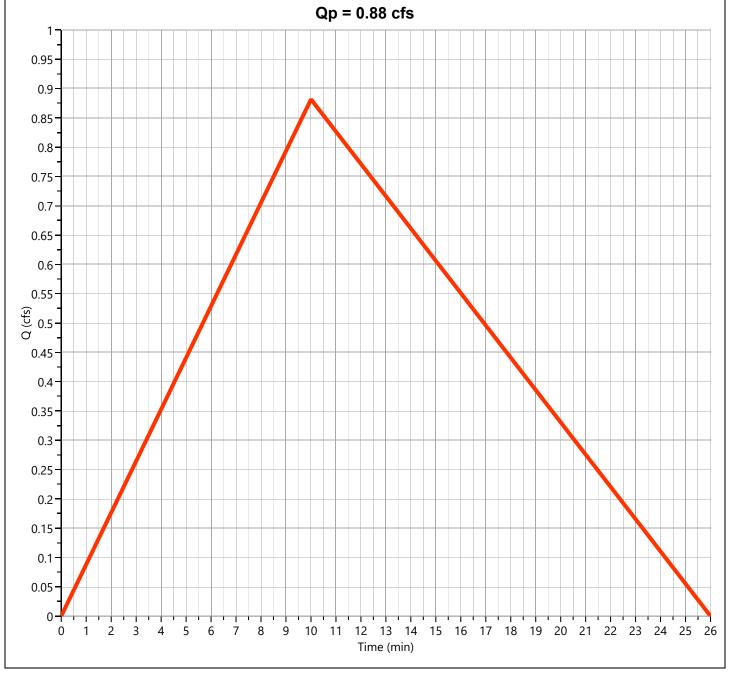
Detention Basin

Time Interval =	= 25-yr = 1 min = 4 - Post-Dev Basin A		Time to Peak Hydrograph Volume	= 0.77 hrs
			Hydrograph Volume	
Inflow Hydrograph =	= 4 - Post-Dev Basin A		riyurograpir volume	= 13,698 cuft
illiow riyurograpii –			Max. Elevation	= 419.19 ft
Pond Name =	= Bryant Pharmacy Deten	tion Pond	Max. Storage	= 930 cuft
Pond Routing by Storage Indicat	tion Method		Center of ma	ss detention time = 3 min
57		Qp = 4.96 cfs		
5 - 4 - 3 - (5t) O - 2 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1				
0 10	20	30 40	50	60 70
		Time (min)		
	Req'd Stor	Post-Dev Basin A — Dete	ntion Basin	

Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin B Hyd. No. 6

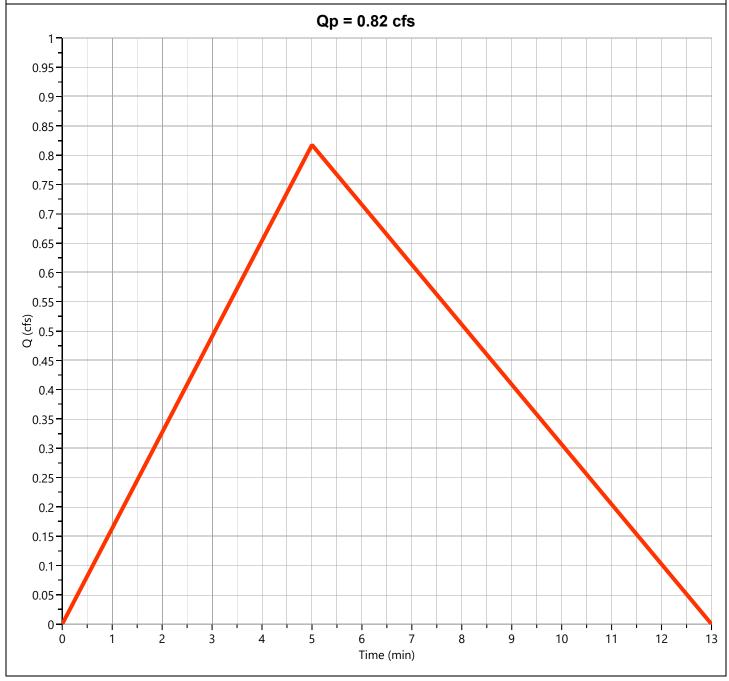
Hydrograph Type	= Rational	Peak Flow	= 0.882 cfs
Storm Frequency	= 25-yr	Time to Peak	= 0.17 hrs
Time Interval	= 1 min	Runoff Volume	= 706 cuft
Drainage Area	= 0.22 ac	Runoff Coeff.	= 0.58
Tc Method	= TR55	Time of Conc. (Tc)	= 10.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 6.91 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin "C"

Hydrograph Type	= Rational	Peak Flow	= 0.818 cfs
Storm Frequency	= 25-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 327 cuft
Drainage Area	= 0.17 ac	Runoff Coeff.	= 0.51
Tc Method	= TR55	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 9.43 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Total Post-Dev Hyd. No. 8

Hydrograph Type	= Junction		Peak Flow	= 5.623 cfs
Storm Frequency	= 25-yr		Time to Peak	= 0.17 hrs
Time Interval	= 1 min		Hydrograph Volume	= 14,704 cuft
Inflow Hydrographs	= 5, 6, 7		Total Contrib. Area	= 0.39 ac
		Qp = 5.62 cfs		
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	— Detention basin — Post-D	ev dasili d —— Post-Dev Ba	Siii C — iotal Post-Dev	
_	— Detention Basin — Post-D	ev Basin B — Post-Dev Ba	sin "C" — Total Post-Dev	

Project Name: Bryant Pharmacy

Hydrograph 50-yr Summary Hydrology Studio v 3.0.0.27

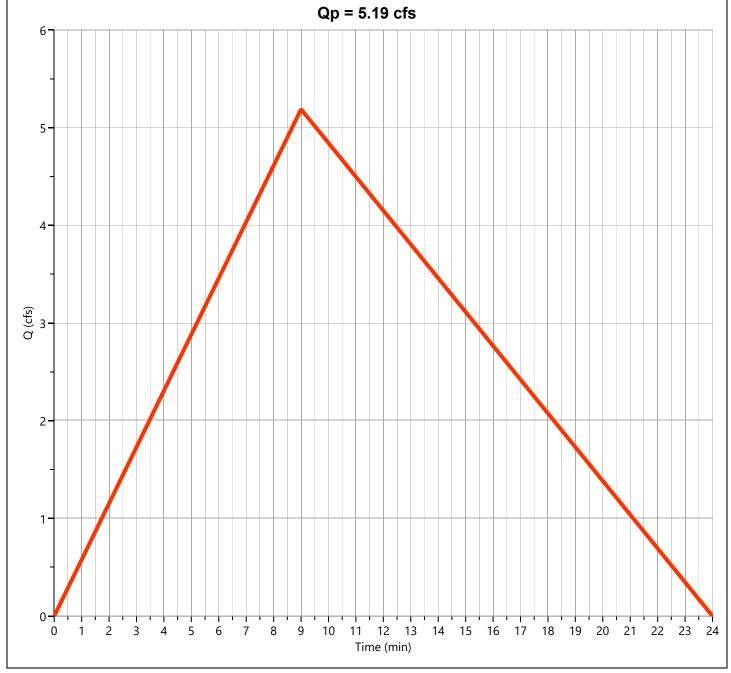
09-05-2025

Hydrology St	udio v 3.0.0.27							09-05-2025
Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	Rational	Pre-Dev Basin "A"	5.188	0.15	3,740			
2	Rational	Pre-Dev Basin "B"	2.924	0.20	2,810			
3	Junction	Total Pre-Dev	7.381	0.15	6,542	1, 2		
4	Mod Rational	Post-Dev Basin A	5.437	0.08	15,170			
5	Pond Route	Detention Basin	5.437	0.77	15,005	4	419.29	1,035
6	Rational	Post-Dev Basin B	0.964	0.17	772			
7	Rational	Post-Dev Basin "C"	0.893	0.08	358			
8	Junction	Total Post-Dev	6.097	0.17	16,105	5, 6, 7		

Hydrology Studio v 3.0.0.27 09-05-2025

Pre-Dev Basin "A"

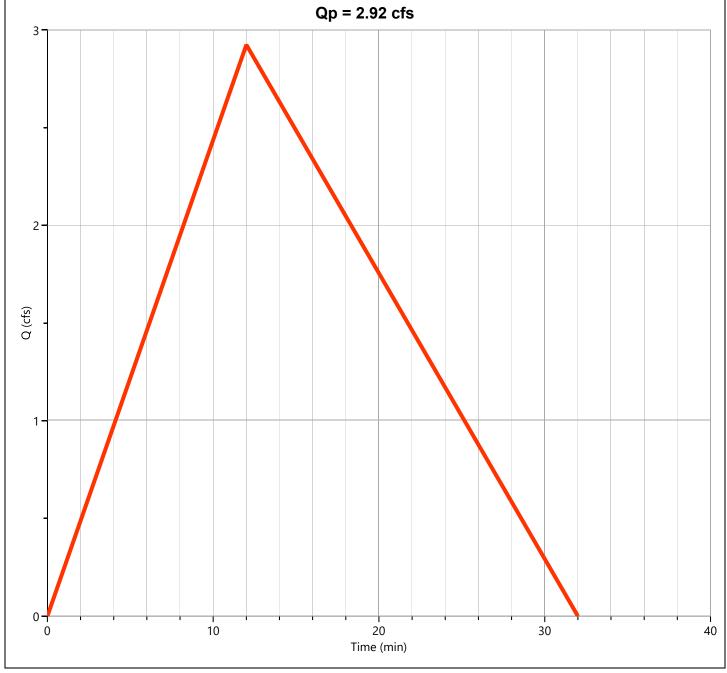
Hydrograph Type	= Rational	Peak Flow	= 5.188 cfs
Storm Frequency	= 50-yr	Time to Peak	= 0.15 hrs
Time Interval	= 1 min	Runoff Volume	= 3,740 cuft
Drainage Area	= 1.17 ac	Runoff Coeff.	= 0.56
Tc Method	= TR55	Time of Conc. (Tc)	= 9.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 7.92 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Pre-Dev Basin "B"

Hydrograph Type	= Rational	Peak Flow	= 2.924 cfs
Storm Frequency	= 50-yr	Time to Peak	= 0.20 hrs
Time Interval	= 1 min	Runoff Volume	= 2,810 cuft
Drainage Area	= 0.75 ac	Runoff Coeff.	= 0.56
Tc Method	= TR55	Time of Conc. (Tc)	= 12.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 6.96 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factor	rs = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

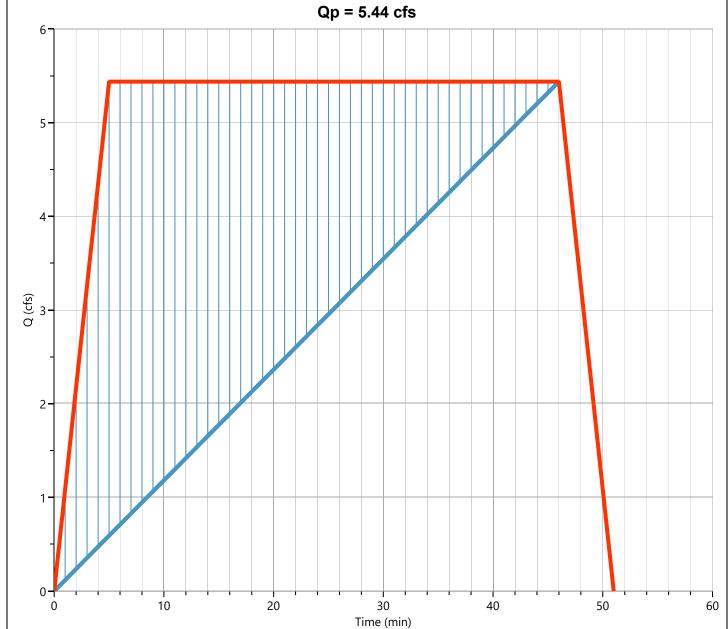
Total Pre-Dev Hyd. No. 3

Hydrograph Type	= Junction		Peak Flow	= 7.381 cfs
Storm Frequency	= 50-yr		Time to Peak	= 0.15 hrs
Time Interval	= 1 min		Hydrograph Volume	= 6,542 cuft
Inflow Hydrographs	= 1, 2		Total Contrib. Area	= 1.92 ac
Qp = 7.38 cfs				
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-		Time (min)	50	.0
	— Pre-Dev Basin "A'	' — Pre-Dev Basin "B" —	■ Total Pre-Dev	
		37		

Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin A

Target Q	= 5.190 cfs	Required Storage	= 7,230 cuft
Freq. Corr. Factor	= 1.00	Storm Duration	= 9.3 x Tc
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 3.82 in/hr
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
Drainage Area	= 1.5 ac	Runoff Coeff.	= 0.95
Time Interval	= 1 min	Runoff Volume	= 15,170 cuft
Storm Frequency	= 50-yr	Time to Peak	= 0.08 hrs
Hydrograph Type	= Mod Rational	Peak Flow	= 5.437 cfs



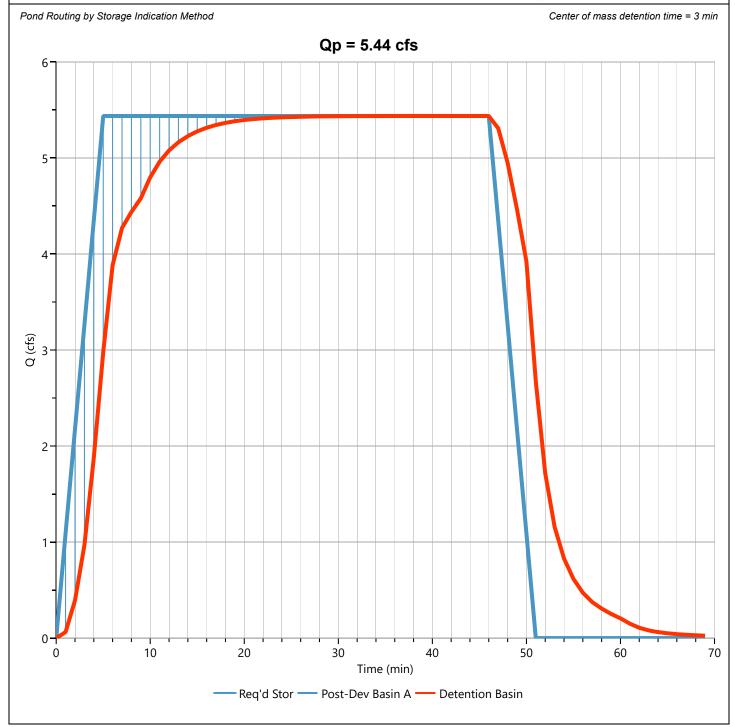
Hyd. No. 5

Hydrograph Report

Hydrology Studio v 3.0.0.27 09-05-2025

Detention Basin

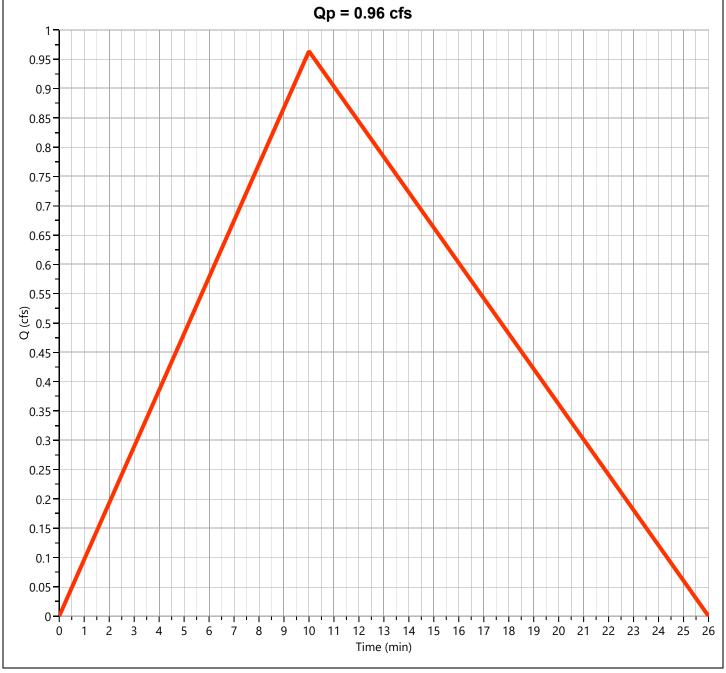
Hydrograph Type	= Pond Route	Peak Flow	= 5.437 cfs
Storm Frequency	= 50-yr	Time to Peak	= 0.77 hrs
Time Interval	= 1 min	Hydrograph Volume	= 15,005 cuft
Inflow Hydrograph	= 4 - Post-Dev Basin A	Max. Elevation	= 419.29 ft
Pond Name	= Bryant Pharmacy Detention Pond	Max. Storage	= 1,035 cuft



Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin B Hyd. No. 6

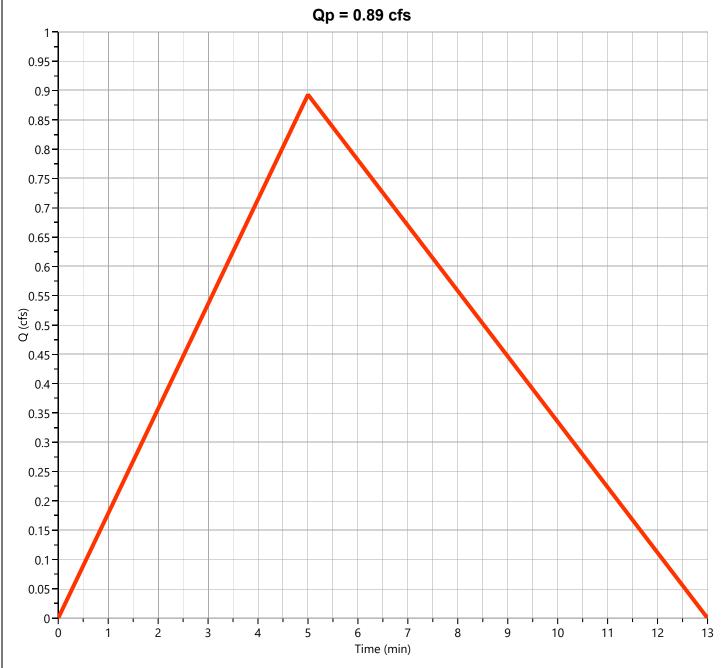
Hydrograph Type	= Rational	Peak Flow	= 0.964 cfs
Storm Frequency	= 50-yr	Time to Peak	= 0.17 hrs
Time Interval	= 1 min	Runoff Volume	= 772 cuft
Drainage Area	= 0.22 ac	Runoff Coeff.	= 0.58
Tc Method	= TR55	Time of Conc. (Tc)	= 10.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 7.55 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factor	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin "C"

Hydrograph Type	= Rational	Peak Flow	= 0.893 cfs
Storm Frequency	= 50-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 358 cuft
Drainage Area	= 0.17 ac	Runoff Coeff.	= 0.51
Tc Method	= TR55	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 10.30 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factor	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Total Post-Dev Hyd. No. 8

Hydrograph Type	= Junction	Peak Flow	= 6.097 cfs
Storm Frequency	= 50-yr	Time to Peak	= 0.17 hrs
Time Interval	= 1 min	Hydrograph Volume	= 16,105 cuft
Inflow Hydrographs	= 5, 6, 7	Total Contrib. Area	= 0.39 ac
	Qp = 6.10 cfs		
7 - 6 - 5 - 4 - (sty) O	Qρ = 6.10 cτs		
	10 15 20 25 30 35 40 Time (min) Detention Basin — Post-Dev Basin B — Post-Dev Basin B	45 50 55 asin "C" — Total Post-Dev	60 65

Project Name: Bryant Pharmacy

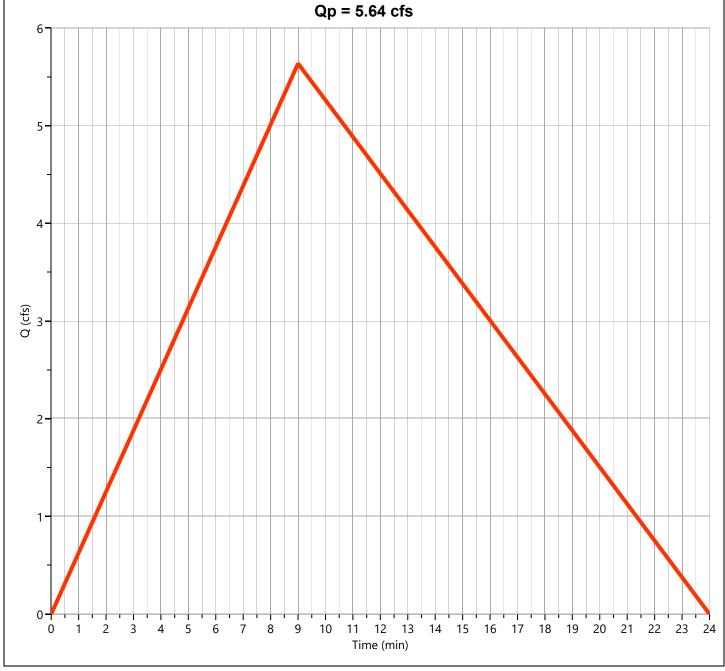
Hydrograph 100-yr Summary

09-05-2025 Hydrology Studio v 3.0.0.27 Peak Time to Hydrograph Inflow Maximum Maximum Hyd. Hydrograph Hydrograph Volume Flow Peak Hyd(s) Elevation Storage No. Type Name (hrs) (cuft) (cuft) (cfs) (ft) Rational Pre-Dev Basin "A" 5.636 0.15 4,063 1 2 Rational Pre-Dev Basin "B" 3.175 0.20 3,052 8.017 3 Junction Total Pre-Dev 0.15 7,106 1, 2 Mod Rational 0.08 4 Post-Dev Basin A 5.893 16,440 5 Pond Route **Detention Basin** 5.892 0.77 16,261 419.42 1,163 4 Rational Post-Dev Basin B 1.047 0.17 839 6 7 Rational Post-Dev Basin "C" 0.971 0.08 389 8 Junction Total Post-Dev 6.620 0.17 17,456 5, 6, 7

Hydrology Studio v 3.0.0.27 09-05-2025

Pre-Dev Basin "A"

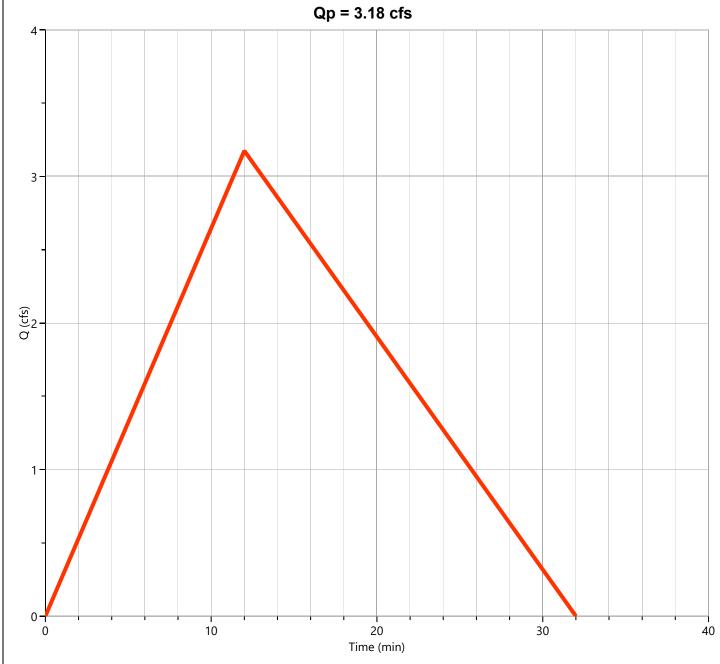
Hydrograph Type	= Rational	Peak Flow	= 5.636 cfs
Storm Frequency	= 100-yr	Time to Peak	= 0.15 hrs
Time Interval	= 1 min	Runoff Volume	= 4,063 cuft
Drainage Area	= 1.17 ac	Runoff Coeff.	= 0.56
Tc Method	= TR55	Time of Conc. (Tc)	= 9.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 8.60 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Pre-Dev Basin "B"

Qp = 3.18 cfs			
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factor	rs = 1/1.67
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 7.56 in/hr
Tc Method	= TR55	Time of Conc. (Tc)	= 12.0 min
Drainage Area	= 0.75 ac	Runoff Coeff.	= 0.56
Time Interval	= 1 min	Runoff Volume	= 3,052 cuft
Storm Frequency	= 100-yr	Time to Peak	= 0.20 hrs
Hydrograph Type	= Rational	Peak Flow	= 3.175 cfs



Hydrology Studio v 3.0.0.27 09-05-2025

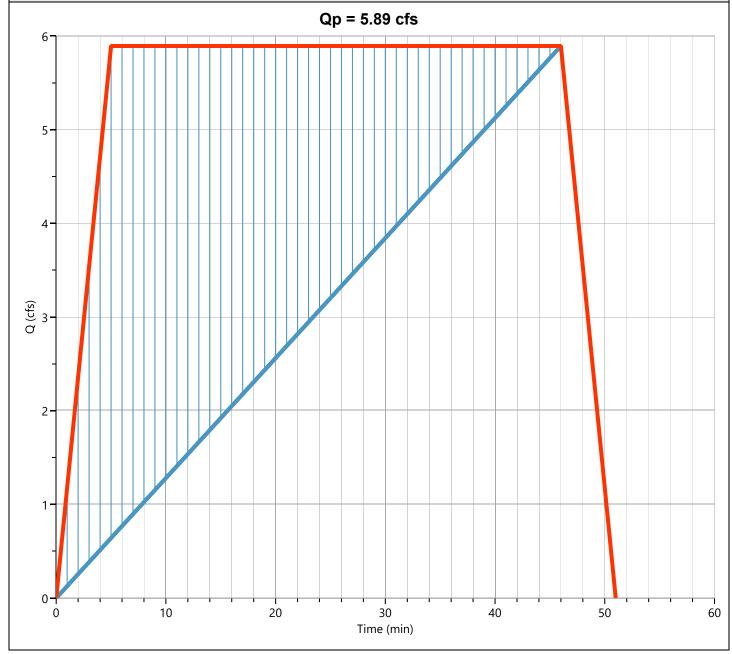
Total Pre-Dev Hyd. No. 3

Hydrograph Type	= Junction	Peak Flow	= 8.017 cfs
Storm Frequency	= 100-yr	Time to Peak	= 0.15 hrs
Time Interval	= 1 min	Hydrograph Volu	ıme = 7,106 cuft
Inflow Hydrographs	= 1, 2	Total Contrib. Are	ea = 1.92 ac
	Qp =	: 8.02 cfs	
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0	10	20 30 Time (min)	40
	— Pre-Dev Basin "A" — P	re-Dev Basin "B" — Total Pre-Dev	

Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin A

Hydrograph Type	= Mod Rational	Peak Flow	= 5.893 cfs
Storm Frequency	= 100-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 16,440 cuft
Drainage Area	= 1.5 ac	Runoff Coeff.	= 0.95
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 4.14 in/hr
Freq. Corr. Factor	= 1.00	Storm Duration	= 9.3 x Tc
Target Q	= 5.640 cfs	Required Storage	= 7,811 cuft



Hyd. No. 5

Hydrograph Report

10

20

Hydrology Studio v 3.0.0.27 09-05-2025

Detention Basin

Storm Frequency = 100-yr Time Interval = 1 min Inflow Hydrograph = 4 - Post-Dev Basin A Pond Name = Bryant Pharmacy Detention Pond Max. Storage = 1,163 cuft Pond Routing by Storage Indication Method Center of mass detention time = 3 min Qp = 5.89 cfs
Inflow Hydrograph = 4 - Post-Dev Basin A Max. Elevation = 419.42 ft Pond Name = Bryant Pharmacy Detention Pond Max. Storage = 1,163 cuft Pond Routing by Storage Indication Method Center of mass detention time = 3 min Qp = 5.89 cfs
Pond Name = Bryant Pharmacy Detention Pond Max. Storage = 1,163 cuft Pond Routing by Storage Indication Method Center of mass detention time = 3 min Qp = 5.89 cfs
Pond Routing by Storage Indication Method Qp = 5.89 cfs
Qp = 5.89 cfs
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(Fig. 1)
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Time (min)

Req'd Stor — Post-Dev Basin A — Detention Basin

40

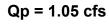
30

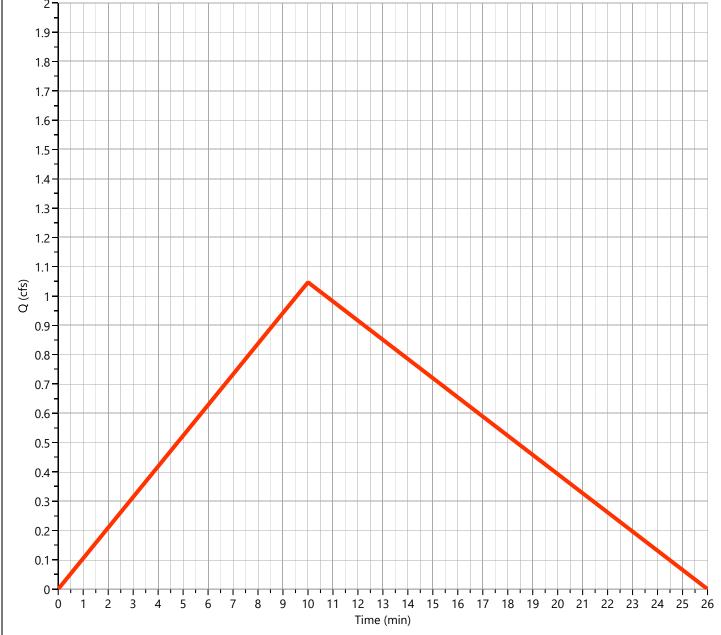
50

Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin B

Hydrograph Type	= Rational	Peak Flow	= 1.047 cfs
Storm Frequency	= 100-yr	Time to Peak	= 0.17 hrs
Time Interval	= 1 min	Runoff Volume	= 839 cuft
Drainage Area	= 0.22 ac	Runoff Coeff.	= 0.58
Tc Method	= TR55	Time of Conc. (Tc)	= 10.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 8.20 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67

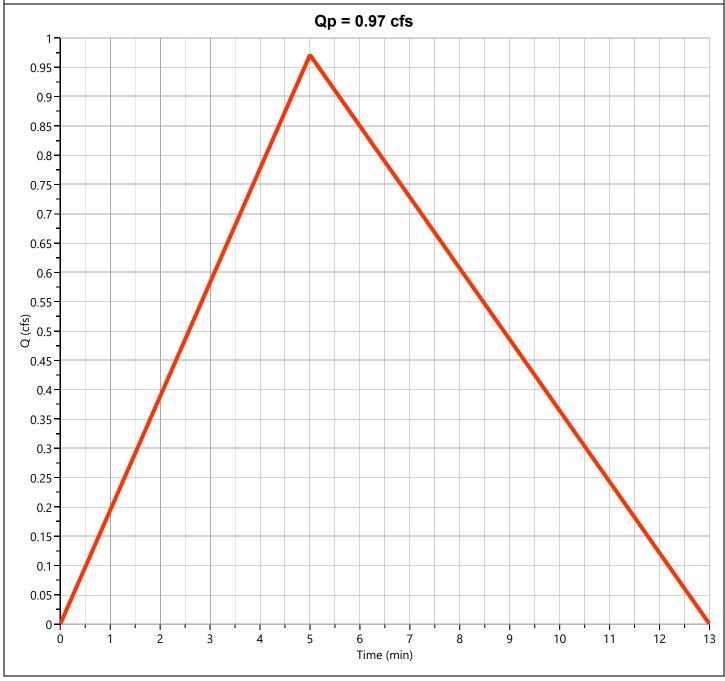




Hydrology Studio v 3.0.0.27 09-05-2025

Post-Dev Basin "C"

Hydrograph Type	= Rational	Peak Flow	= 0.971 cfs
Storm Frequency	= 100-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 389 cuft
Drainage Area	= 0.17 ac	Runoff Coeff.	= 0.51
Tc Method	= TR55	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= City of Bryant IDF Curve.idf	Intensity	= 11.20 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	s = 1/1.67



Hydrology Studio v 3.0.0.27 09-05-2025

Total Post-Dev Hyd. No. 8



CERT

V0174,1

129 North Main Street Benton, Arkansas 72015 office: (501) 315-2626 fax: (501) 315-0024 www.HopeConsulting.com

plat. This drawing does not represent an actual survey. This Plot Plan depicts the lot as it appears on the subdivision 6.0 No1°08'16"E 84.00 PROPOSED DWELLING PROPOSED DWELLING S88°51'44"E 126.00' 96.00 RECORD DESCRIPTION LOT 4,BLOCK 2 OF MIDTOWN BRYANT SUBDIVISION IN THE CITY OF BRYANT,ARKANSAS. PROPOSED DWELLING CONCRETE-SIDEWALK A STREET (46' R/W) PROPOSED DWELLING LOT 4 - 8 BLOCK 2 0.27 AC 11562.6 SF No1008'16"E 32 N88°51'44"W 30.00' PROPOSED DWELLING 3' B.S.L S01°08'16"W 116.62 30.2 -CONCRETE SIDEWALK COMMONWEALTH DRIVE (60' R/W) PLOT PLAN LEGEND Found Monument Set 1/2" Rebar #1664

to the Flood Insurance Rate Map, panel # <u>05125C0225E</u>, dated:

112 "A" Street

HD Homes

Address

For the Exclusive Use and Benefit of:

Bryant, Arkansas, 72022

No portion of the property described hereon lies within the 100 year flood plain, according

06/05/2020

(M)

(R/D/P) B.S.L.

U.E./D.E

08/12/2025

Date

Computed Point Location Measured by Surveyor Record/Deed/Plat Measurements

Building Setback Line Restriction Utility/Drainage Easement Property Boundary Line Fence Lines

Centerlines Parcel Lines/Misc Lines

 $\overline{\Omega}$

WCS



129 North Main Street Benton, Arkansas 72015 office: (501) 315-2626 fax: (501) 315-0024 www. Hope Consulting. com

Utility/Drainage Easement Property Boundary Line Fence Lines

Centerlines Parcel Lines/Misc Lines

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WCS

U.E./D.E.

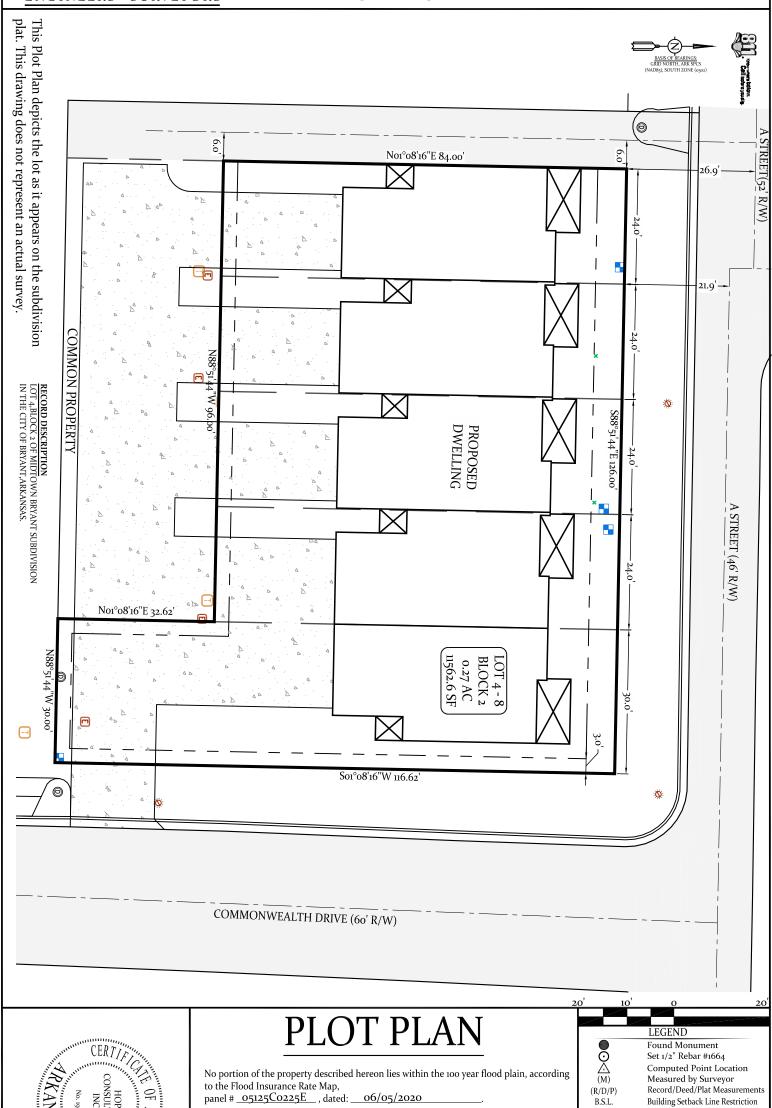
08/12/2025

1664

62

Date

210



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For the Exclusive Use and Benefit of:

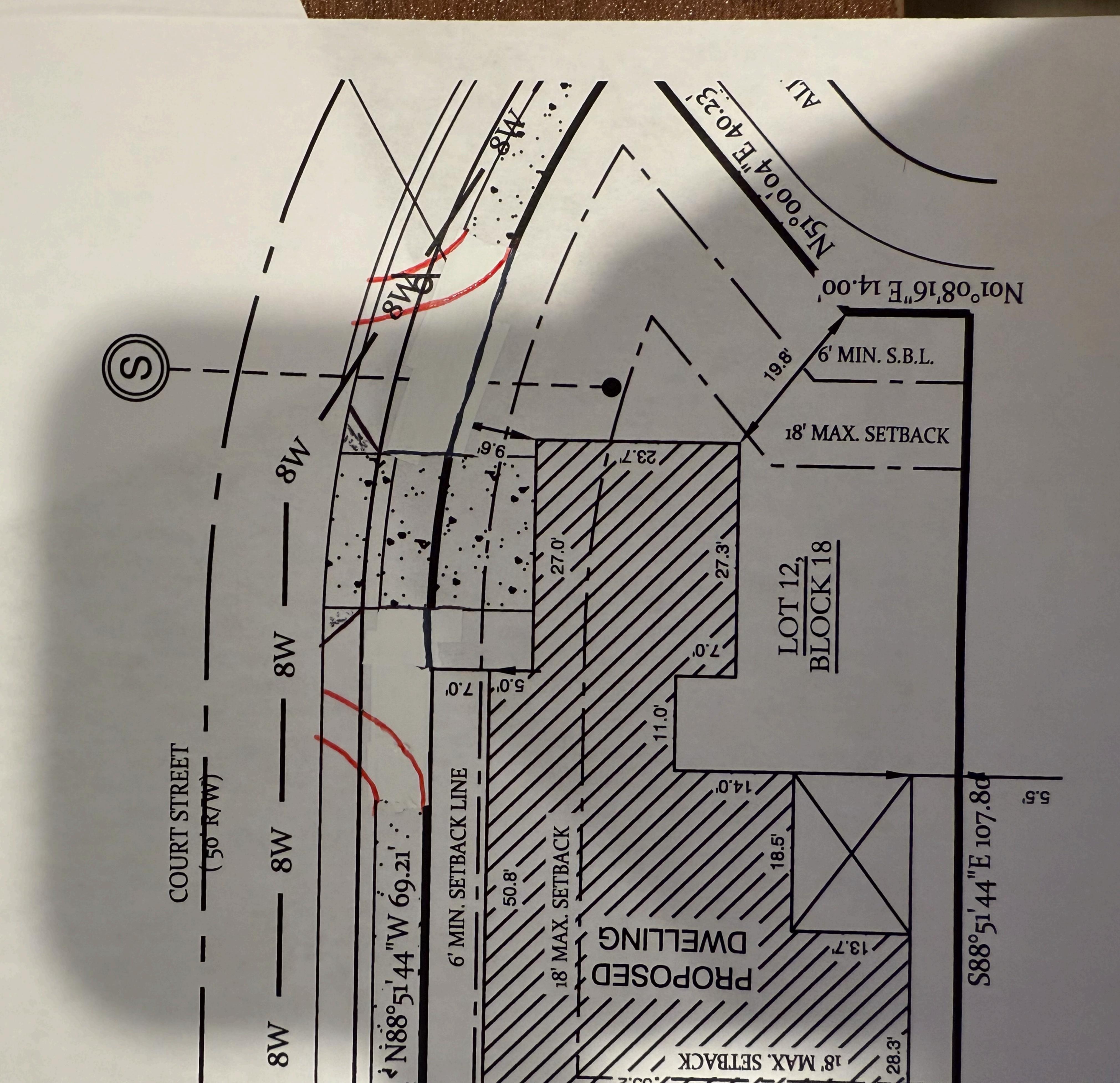
Bryant, Arkansas, 72022

112 "A" Street

HD Homes

Address

V0174.





Conditional Use Permit Application

Applicants are advised to read the Conditional Use Permit section of Bryant Zoning Code prior to completing and signing this form. The Zoning Code is available at www.cityofbryant.com under the Planning and Community Development tab.

Date: 10-03-2025	
Applicant or Designee:	Project Location:
Name AAVID & NENETSE MOULTN	Property Address 40/5 RORTAINODA CTRCE
Address 4015 ROBDINGO CORCLE	BRYANT, AR. TZONZ
BRYPAIT, BL. 72022	
Phone <u>501-951-2032</u>	Parcel Number <u>840-11500-092</u>
Email: <u>AMOULTALO YAHDO COM</u>	Zoning Classification <u>[FST] ANTIAL</u>
Property Owner (If different from Applicant)	:
Name	
Phone	
Address	
Email Address	
Additional Information:	
Subdivision Lot and Block Number or Legal Descr	iption (Attach Legal Description to Application)
	72/ 2012-90103
V 67 Carteman I 27 Bendenhell V A Bende Al M Bell Peril I I A M A A subsending A	
Current Use of Property PRIMARY RES	LALACE
Description of Conditional Use Request / Propos	ed Use of Property (Attach any necessary drawings or images)
	BIMINING FOR USE AS A POOL
HOUSE.	

Application Checklist

Requirements for Submission

- Letter stating request of Conditional Use and reasoning for requestCompleted Conditional Use Permit Application
- ☑ Submit Conditional Use Permit Application Fee (\$125)
- ☑ Submit Copy of completed Public Notice [Attachment 1]
 - Submit one (1) copy of the Development Plan (Site Plan) showing:
 - Location, size, and use of buildings/signs/land or improvements
 - Location, size, and arrangement of driveways and parking. Ingress/Egress
 - Existing topography and proposed grading
 - Proposed and existing lighting
 - Proposed landscaping and screening
 - Use of adjacent properties
 - Scale, North Arrow, Vicinity Map
 - Additional information that may be requested by the administrative official due to unique conditions of the site.
- □ Public Notice Requirements: **NOTE: Failure to provide notice in the following manners** shall require delay of the public hearing until notice has been properly made.
 - Publication: Public Notice shall be published at least one (1) time fifteen (15)
 days prior to the public hearing at which the variance will be heard. A copy of
 the public notice is provided on last page of application. [Attachment 1] Once
 published, the proof of publication must be provided to the Planning and
 Development office.
 - Posting of Property: The city shall provide a sign to post on the property involved for the fifteen (15) consecutive days leading up to the public hearing.
 One (1) sign is required for every two hundred (200) feet of street frontage.

Once the application is received, the material will be reviewed to make sure all the required information is provided. The applicant will be notified if additional information is required. The application will then go before the Development and Review Committee (DRC) for a recommendation to the Planning Commission. A public hearing will be held at this meeting for comments on the Conditional Use. After the public hearing, the Planning Commission will make a decision on the use.

D	EA	D	CA	DEC	3 4 4 9	V	DELODE	SIGNING
13	1	VD.	LA	KFF	1 1 1 1	Y	REFURE	

I do hereby certify that all information contained within this application is true and correct. I further certify that the owner of the property authorizes this proposed application. I understand that I must comply with all City Codes and that it is my responsibility to obtain all necessary permits required.

David and Deneise Moulin 4015 Robinwood Circle Bryant, Arkansas 72022

September 30, 2025

City of Bryant, Arkansas Planning and Development 210 SW 3rd Street Bryant, Arkansas 72022

RE: Conditional Use Permit Application

Good Day,

We are respectfully requesting approval of a Conditional Use Permit for a replacement covered building on our primary residence property. The previous covered deck has been completely removed, and the replacement structure for the most part, is in place. We have approximately \$23k in purchased building materials, an additional \$5k in roof structure that was built off site, and \$1800 in specialized labor (plumbing rough-in). I genuinely believed that replacing an existing structure would not be an issue...however, I am now painfully aware that a permit was needed.

The intended use will be as a pool house, and the location is contained within a fenced corner area of our backyard. There is no egress into any of the neighboring properties and access into our backyard is through a lockable front gate. There was an existing covered deck in the same location. This project started out being a repair to the existing structure by replacing rotten boards. Then this turned into a rebuild due to deterioration of the wood structure. The frame is $2'' \times 8''$ treated lumber on $6'' \times 6''$ treated posted cemented into ground at original post locations. The roof will be metal and match the house color for aesthetics. The walls will be non-structural in nature and would be installed between the $6'' \times 6''$ posts, outside painted to match house. This request includes provisions for a small half bathroom and electrical. There is existing electrical service to the building.

Thank You in Advance,

David L. Moulin

[Attachment 1]

SAMPLE NEWSPAPER NOTICE - This notice is to be run in the legal notices section of the Saline Courier no less than fifteen (15) days prior to the public hearing.

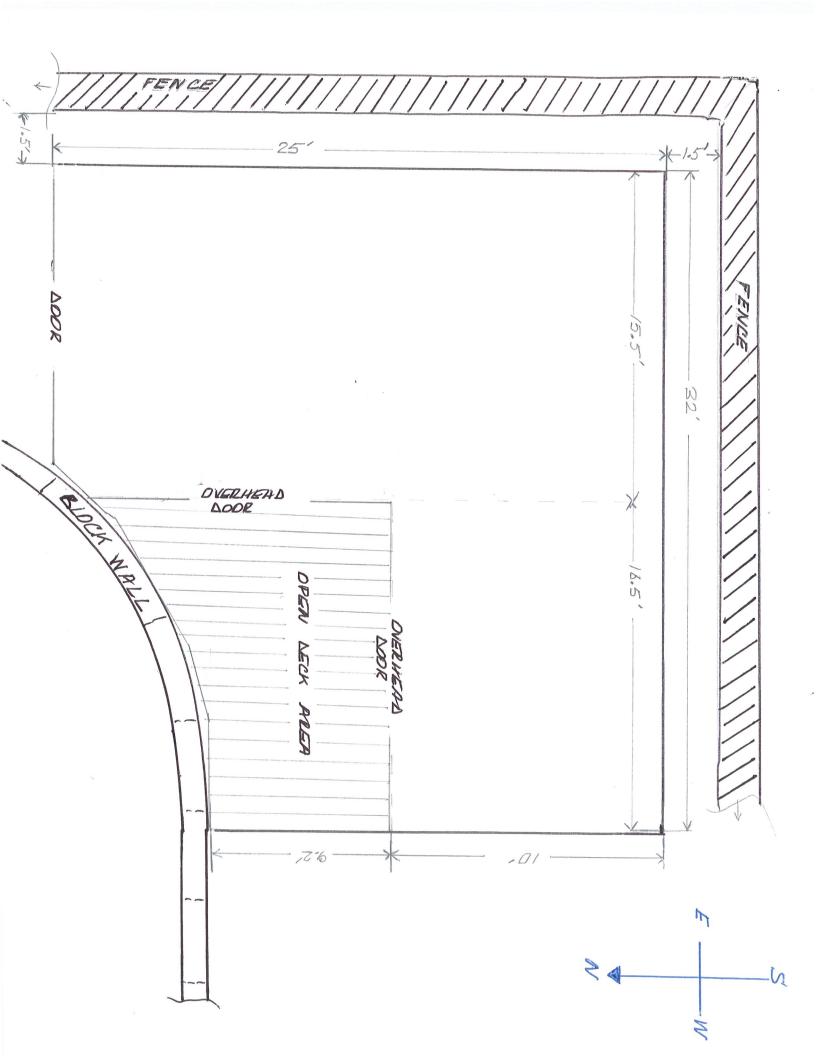
NOTICE OF PUBLIC HEARING

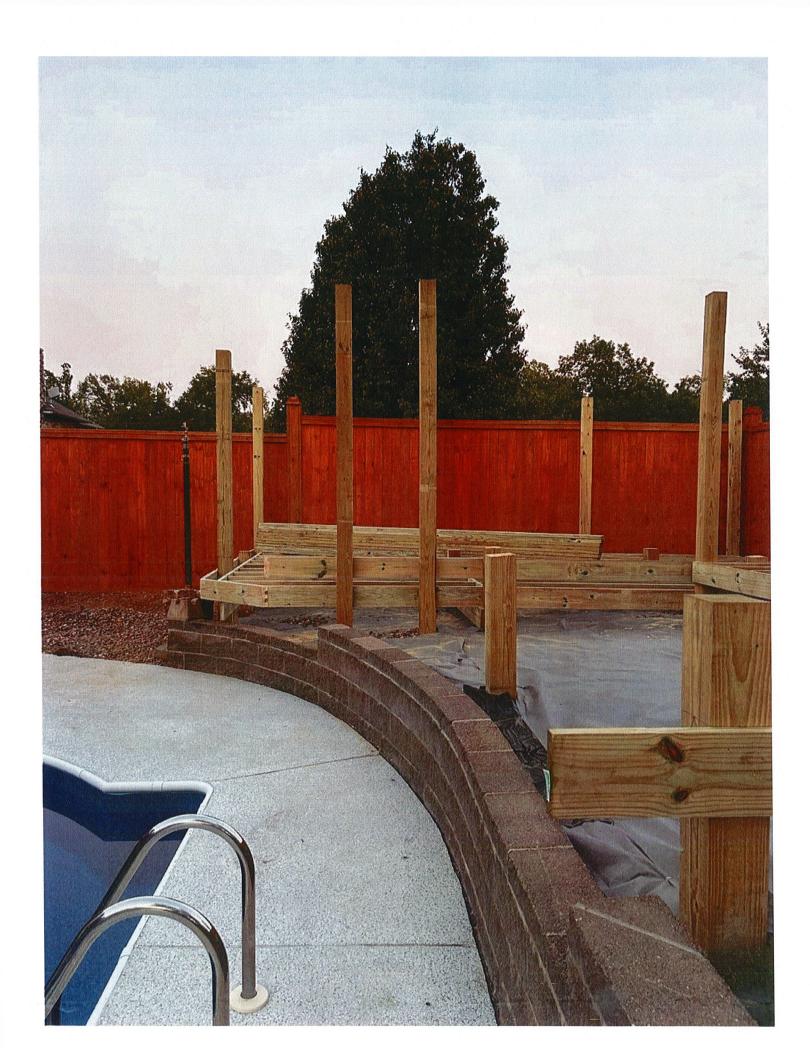
A public hearing will be held by the City of Bryant, AR Planning (Commission on
Monday, at 6:00 P.M. at	the Bryant City Office
Complex, 210 Southwest 3rd Street, for the purpose of public co	omment on the
application for AAVII) L. MOULIA	(your name) to obtain a
Conditional Use for the purpose of COMPLETION OF A FR	DOL HOUSE
IN BACKYARD-CORNER OF PRODUCTY (SE)	(use requested)
within a(current zoning) zone at the site of 40/5 ROB	INVUDDA CIRCLE
BRYANT ARKANSAS 72022 (address)	. A legal description of
this property can be obtained by contacting the Bryant Planning	and Development
Department at 501-943-0488.	

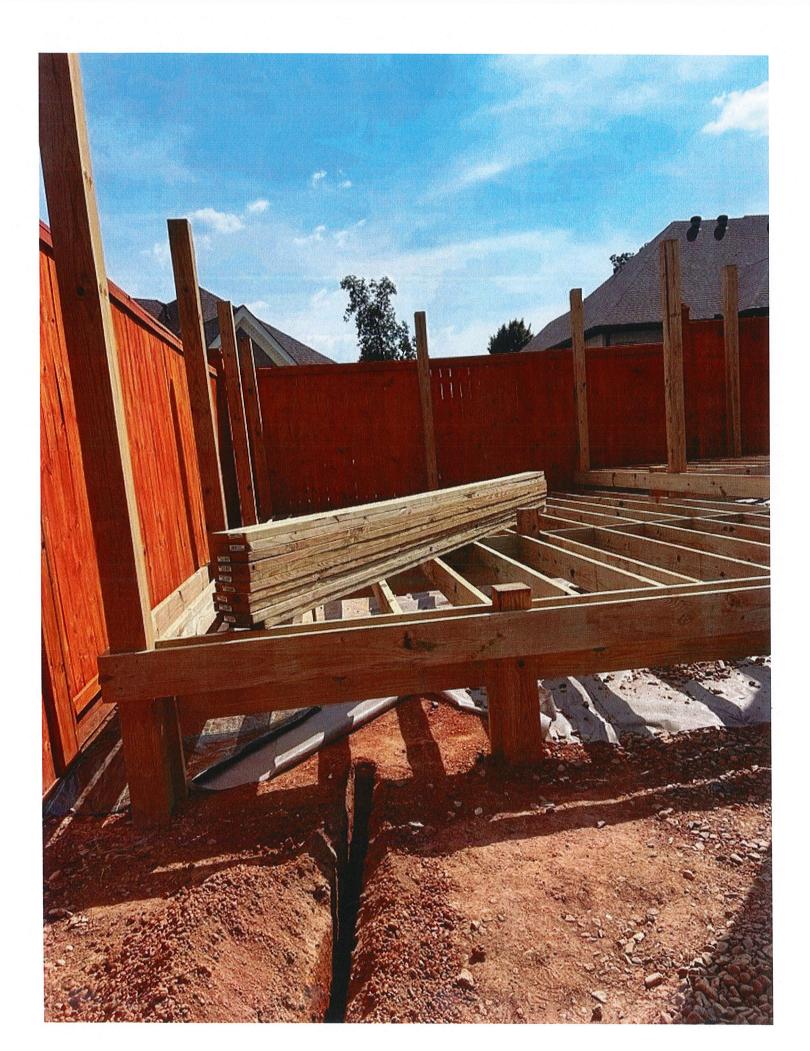
Parcel Number: Property Address: County Name: Mailing Address: Subdivision: Collector's Mailing Address @: Total Acres: Lot/Block: Sec-Twp-Rng: Timber Acres: School District: Legal Description: Homestead Parcel?: mprovement Districts: 840-11500-092 0.00 Saline County 0.00 **BRYANT AR 72022** MOULIN DAVID L JR & DENEISE M Map This Address 4015 ROBINWOOD CIR MOULIN DAVID L JR & DENEISE M ATTN: REFUNDS DEPT - CL CORELOGIC *MTG* 4015 ROBINWOOD CIR BRYANT, AR WESTPOINTE NORTH WESTPOINTE NORTH PHII 16-01S-14W **IRVING, TX 75063** 3001 HACKBERRY RD Taxable 253 BRYANT/BRYANT 2012-90103

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SIGN PERMIT APPLICATION

Applicants are advised to read the Sign Ordinance prior to completing and signing this form.

The Sign Ordinance is available at www.cityofbryant.com under the Planning and Community

Development tab.

09/19/2025 Date:	Note: Electrical Permits may be Required, Please contact the Community Development Office for more information.
Sign Co. or Sign Owner	Property Owner
NameARKANSAS SIGN & NEON	Name GASSY'S PHILLIPS 66
Address 8525 DISTRIBUTION DR	Address 6101 HWY 5
City, State, ZIFTLE ROCK AR 72209	City, State, Zip
Phone 501.562.3942 lora@arkansassign.com Email Address	Phone 501-399-9910 Email Address BRYANTONESTOPLLC@GMAIL.COM
GENERAL INFORMATION	
Name of Business GASSY'S PHILLIPS 66	
Address/Location of sign 6101 HWY 5	
Zoning Classification	
Please use following page to provide details on	the signs requesting approval. Along with information

Please use following page to provide details on the signs requesting approval. Along with information provided on this application, a Site Plan showing placement of sign(s) and any existing sign(s) on the property is required to be submitted. Renderings of the sign(s) showing the correct dimensions is also required to be submitted with the application. A thirty-five dollar (\$35) per sign payment will be collected at the time of permit issuance. According to the Sign Ordinance a fee for and sign variance or special sign permit request shall be one hundred dollars (\$100). Additional documentation may be required by Sign Administrator.

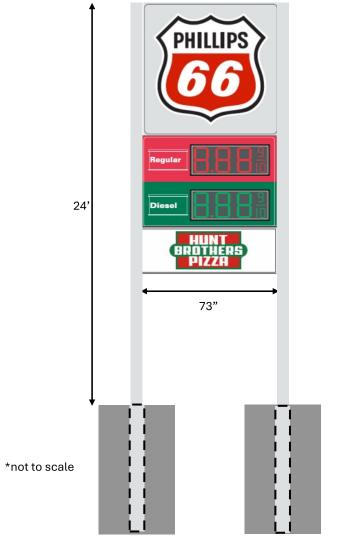
and correct. I fully understand that the terms of the Sign Ordinance supersede the Sign Administrator's approval and that all signs must fully comply with all terms of the Sign Ordinance regardless of approval. I further certify that the proposed sign is authorized by the owner of the property and that I am authorized by the property owner to make this application. I understand

READ REFULLY BEFORE SIGNING

that no sign may be placed in public right of way. I understand that I must comply with all Building and Electrical Codes and that it is my responsibility to obtain all necessary permits.

Use table below to enter information regarding each sign for approval. Please use each letter to reference each sign rendering.

SIGN	Type (Façade, Pole, Monument, other)	Dimensions (Height, Length, Width)	Sqft (Measured in whole as rectangle)		t of Sign om lot surface)	Column for Admin Certifying Approval
				Top of Sign	Bottom of Sign	
Α	POLE	7' X 9'	63	24'	15'	
В						
С						
E						
F						
G						



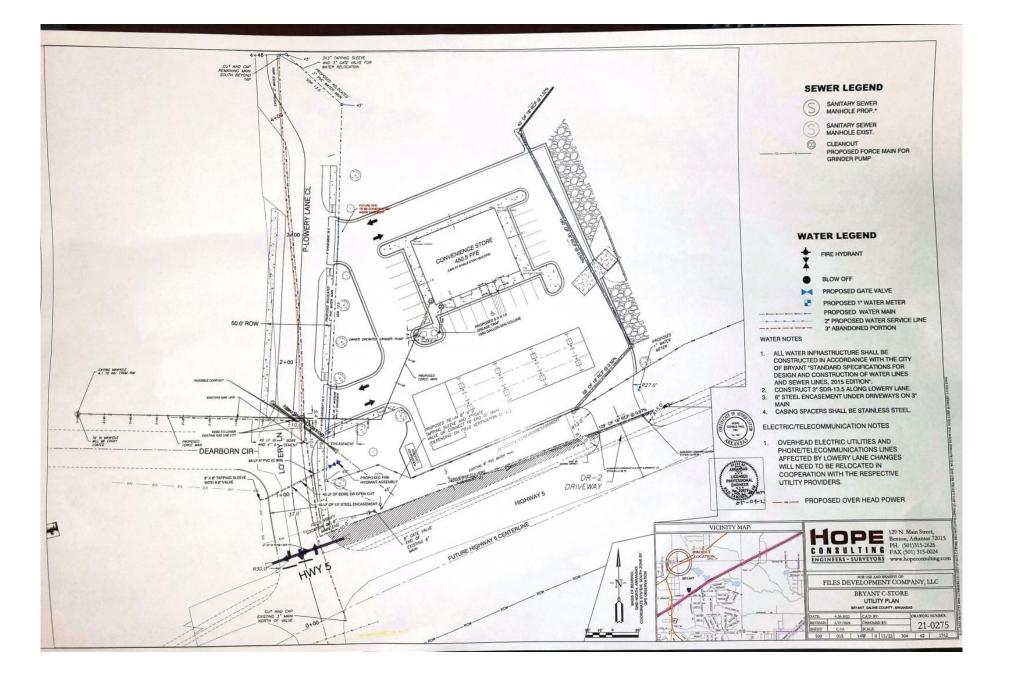
6x6 Dual Pole Mount Rivet ID 2-Product LED Price Sign (Regular, Diesel) Paint Poles Titanium Gray Poles = 8" x 8" – DIRECT BURY 24' above grade Gassy's #2 ST#922842 6101 Highway 5 Bryant, AR 72022

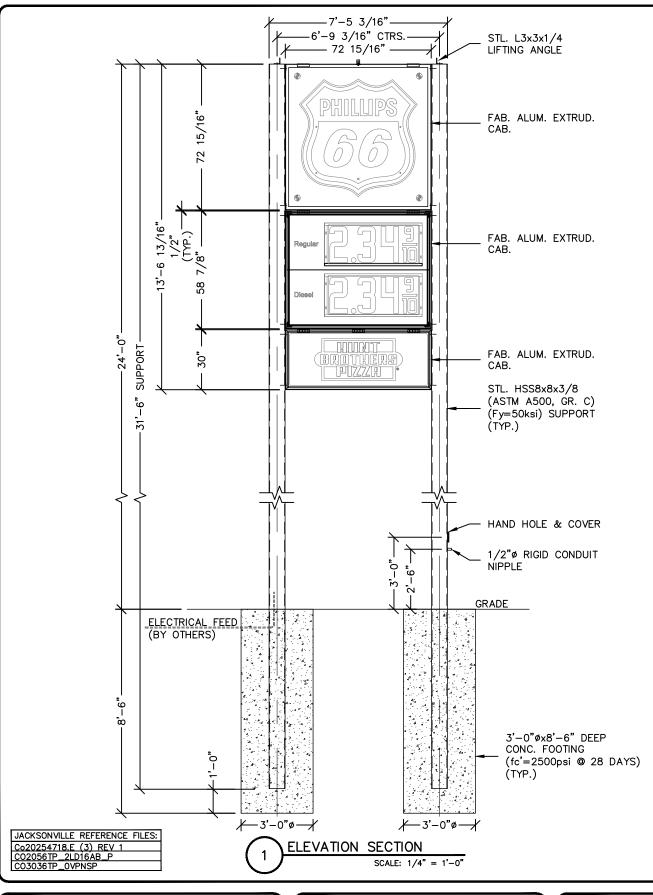


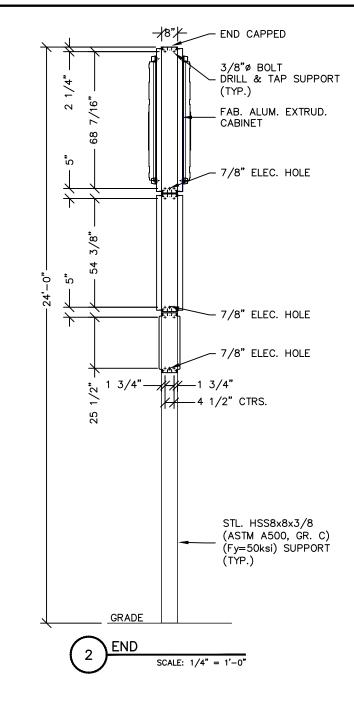


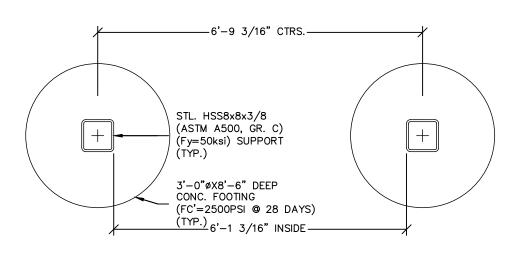












PLAN VIEW

SCALE: 1/2" = 1'-0"

GENERAL NOTES:

BRYANT, AR 72002

- 1. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS ON JOB SITE.
 2. STRUCTURAL STEEL RECTANGULAR AND ROUND TUBE SHALL CONFORM TO ASTM A500, GR. C.
 3. STRUCTURAL STEEL ANGLE SHALL CONFORM TO ASTM A36.
 4. STRUCTURAL STEEL PLATES SHALL CONFORM TO ASTM A36.
 5. WELDING SHALL CONFORM TO AWS D 1.1 & AISC SPECS.
 6. ALL WELDING TO BE PERFORMED BY CERTIFIED WELDER.
 7. ISOLATE ALLIMITURE FORM STEEL

- ISOLATE ALUMINUM FROM STEEL.
- 9. ALL BOLT HOLES TO BE DRILLED OR PUNCHED.

 9. ALL ELECTRICAL WORK TO CONFORM TO THE REQUIREMENTS OF UL48 AND SECTION 600 OF NEC.

 10. THE LOCATION OF THE DISCONNECT SWITCH AFTER INSTALLATION SHALL COMPLY WITH
- ARTICLE 600.6 OF THE NEC.

 11. UL AND DATA LABELS REQUIRED.
- 12. SIGNS TO BE 6-FT HORIZONTAL & 12-FT VERTICAL FROM HIGH VOLTAGE WIRES.

DESIGN NO:

13. CONCRETE TO HAVE f'c=2500 PSI MIN. © 28 DAYS.

14. DESIGN IS BASED ON 105 MPH WIND, 3-SEC GUST, EXPOSURE C, ASCE 7-16.

15. FOUNDATION DESIGN IS BASED ON PRESUMPTIVE SOIL BEARING CAPACITY PER IBC TABLE 1806-2 ASSUMING SOIL TYPE CLASS 4. IF FIELD CONDITIONS ARE DIFFERENT, PLEASE CONTACT THE ENGINEER FOR GUIDANCE.

PROJECT MGR.:



SIGN COMPANY WWW.FEDERALHEATH.COM

1845 PRECINCT LINE ROAD, SUITE 100, HURST, TEXAS 76054 T:817.685.9075 F:817.685.9103

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ART DESIGN REFERENCE #22-79098-20-R1

NO.	REVISIONS	DATE	BY
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S-1001825 D. SAMMONS DRAWN BY: DATE: PAT HODGKINS 15OCT25 JOB NO: 22-79098-20 6101 AR-5

SHEET NO:



Pediatric Therapy Clinic



PROPERTY BRAND/EXTENSION: TheraPeds

PROPERTY LOCATION: 2208 N Reynolds Rd Bryant, AR 72022 PROPERTY CODE:

DATE: 09/30/2025

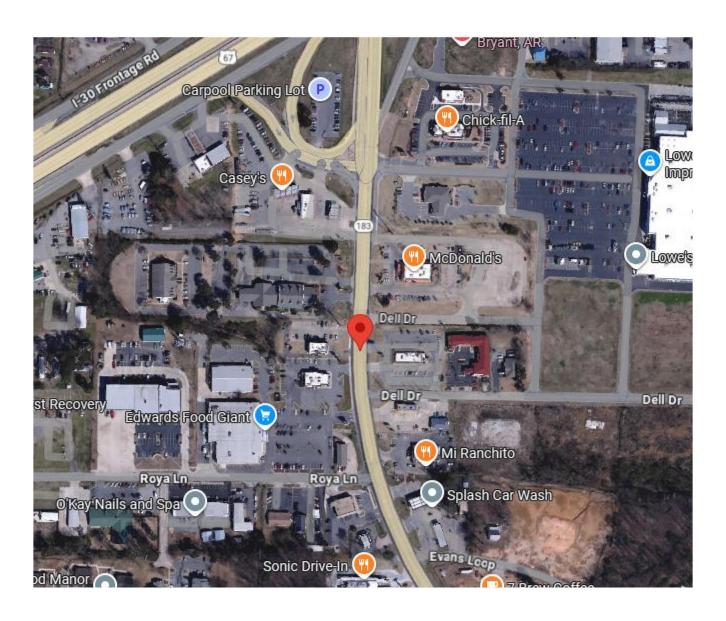
SALES REP: Jason McDonald PREPARED BY:

Victoria Phan

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INITIALS: ____

LOCATION MAP





PROPERTY BRAND/EXTENSION: TheraPeds

PROPERTY LOCATION: 2208 N Reynolds Rd Bryant, AR 72022 PROPERTY CODE:

DATE: 09/30/2025

SALES REP: Jason McDonald

PREPARED BY: Victoria Phan

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INITIALS: ___

SITE PLAN

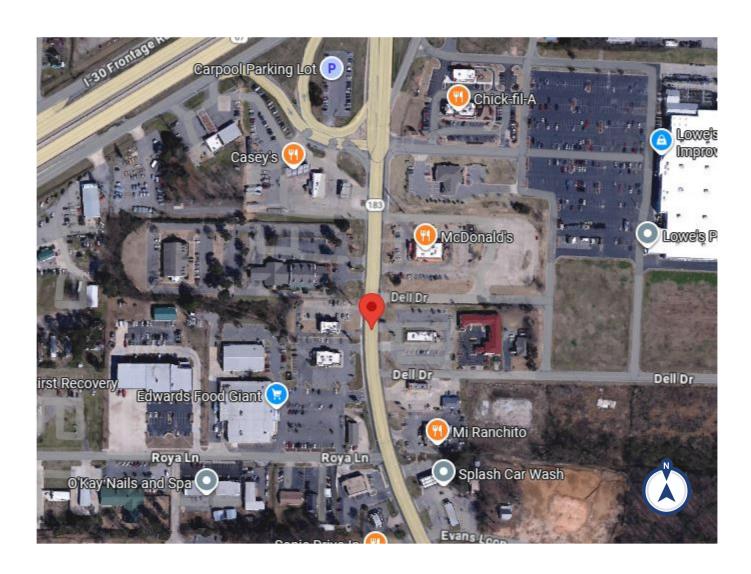
PROPOSED SIGNS:

CHANNEL LETTERS
 CHANNEL LETTERS

EXISTING SIGNS:

1 NO SIGN

2 NO SIGN





PROPERTY BRAND/EXTENSION: TheraPeds

PROPERTY LOCATION:

2208 N Reynolds Rd Bryant, AR 72022

PROPERTY CODE: TBD

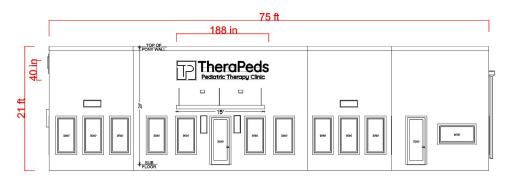
DATE: 09/30/2025

SALES REP: Jason McDonald PREPARED BY: Victoria Phan

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INITIALS: __

PROPOSED



FRONT ELEVATION RENDERINGS NOT TO SCALE



H40" Logo cabinet

H24" x 12' THERAPEDS Channel letter set on raceway

H12" x W10'-7" PEDIATRIC THERAPY CENTER Channel letter set on raceway

Overall Dimension: H40" x W15'-8"

Dual color day/night vinyl on letters

\bigcirc	PANTONE WHITE
	PANTONE BLACK



09/30/2025

PROPERTY BRAND/EXTENSION:
TheraPeds
PROPERTY LOCATION:
2208 N Reynolds Rd Bryant, AR 72022
TBD

DATE:
SALES REP:
PREPARED BY:

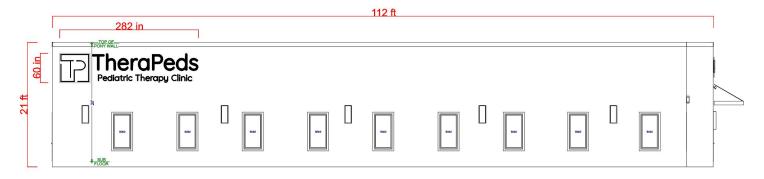
Victoria Phan

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Jason McDonald

INITIALS: ____

PROPOSED



LEFT ELEVATION RENDERINGS NOT TO SCALE



H5' Logo cabinet
H36" x 18' THERAPEDS Channel letter set
H18" x W16' PEDIATRIC THERAPY CENTER Channel letter set
Overall Dimension: H5' x W23'-6"
Dual color day/night vinyl on letters

\bigcirc	PANTONE WHITE
	PANTONE BLACK



PROPERTY BRAND/EXTENSION:
TheraPeds

PROPERTY LOCATION:
2208 N Reynolds Rd Bryant, AR 72022

DATE:
09/30/2025

SALES REP:
Jason McDonald

PREPARED BY:
Victoria Phan

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INITIALS: __